

REPORT Water Report Level 2

Lafarge Goodwood Pit Extension

Submitted to:

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1.0 INTRODUCTION

Golder Associates Ltd. (Golder) was retained by Lafarge Canada Inc. (Lafarge) to prepare a Water Report Level 2 in support of Class A Pit Below Water aggregate licensing application for the proposed Goodwood Pit Extension (the Site). The Site is located at 4900 4th Concession Road in the Township of Uxbridge, Regional Municipality of Durham (Figure 1). The Site is located immediately north of the existing Lafarge Goodwood Pit and is intended to be an extension of that active operation.

The objectives of this study are to 1) characterize the existing hydrogeological and hydrological conditions in the vicinity of the Site; and 2) assess the potential impacts, if any, of the proposed below water extraction on groundwater and surface water resources.

The Study considers three Site conditions:

- 1) Existing Scenario;
- 2) Operational Scenario; and
- 3) Rehabilitated Scenario.

1.1 Policy Considerations

This Study has been prepared to address the requirements of:

- Aggregate Resources of Ontario Standards: A compilation of the four standards adopted by Ontario Regulation 244/97 under the Aggregate Resources Act (August 2020). Pertinent considerations for this report include:
 - A description of the physical setting including local geology, hydrogeology, and surface water systems;
 - "Maximum predicted water table";
 - Potential effects of the operation on any ground water and surface water features located within the zone of influence, including but not limited to: water wells; springs; aquifers; and surface water; and
 - Monitoring plans.
- The Growth Plan for the Greater Gold Horseshoe (2017);
- Lake Simcoe Protection Plan (2009);
- South Georgian Bay Lake Simcoe Source Protection Plan (Approved 2015, Amended 2018);
- The Provincial Policy Statement (2014);
- Township of Uxbridge Official Plan (Office Consolidation January 2014);
- Durham Regional Official Plan (Consolidation May 2017); and
- Oak Ridges Moraine Conservation Plan (2002).

Key hydrogeologic/hydrologic considerations set out by the above policies include:

- Identify an appropriate monitoring program to protect water resources;
- Proposed contingency and mitigative measures that will be implemented if unforeseen impacts occur;
- Minimize potential negative impacts, including cross-subwatershed impacts, and identify surface water and groundwater features;
- Ensure municipal drinking water supply and designated vulnerable areas are protected;
- Protect vulnerable surface water and groundwater sensitive features and their hydrogeologic/hydrologic functions;
- Maintain linkages and related functions between surface water features and groundwater features;
- Promote efficient and sustainable use of water resources, including practices for water conservation sustaining water quality;
- Describe how the connectivity between key hydrogeologic/hydrologic features will be maintained before, during and after extraction;
- Describe how private and agricultural water supplies will be protected;
- Demonstrate no negative impact to groundwater recharge and discharge;
- Describe measures to protect water resources from contamination from on-Site equipment; and
- Ensure there are no adverse thermal impacts to sensitive nearby water features.

2.0 PROPOSED DEVELOPMENT

A brief overview of the Operations and Rehabilitated Scenarios is provided below. are included in Appendix A.

2.1 Operational Scenario

The proposed licence area is approximately 17.9 hectares (ha) with an extraction area limit of 15.4 ha (Figure 2). Approximately 5.7 ha will be below water. Setbacks are as follows: 15 m along the north boundary, a 30 m setback along the eastern boundary, and no setback along the south and west boundary where the Site is adjacent to the existing Goodwood Pit. The pit lake depth will reach a minimum elevation of approximately 310 masl, corresponding to roughly 38 m to 32 m below current ground surface.

Annual tonnage from the Goodwood Extension will not exceed the current limit of 1,177,000 tonnes per year at the existing Goodwood Pit. Extraction will begin in the Phase 1 area to the west and progress easterly to Phase 2. Operations will be serviced by loaders, a screening plant, and a portable crushing plant. Future extraction of aggregate below the water table may utilize a dragline method. Extracted material would be temporarily stockpiled near the face of the extraction with pore water allowed to drain back into the subsurface.

The Site Operations will not require any pumping or active dewatering.

No fuel will be stored on-Site.

2.2 Rehabilitated Scenario

The proposed Rehabilitated Scenario will consist of a 5.7 ha centrally located pond with 3:1 re-vegetated side slopes reaching the Site setbacks. The Site will be separated from the original (i.e. existing) Goodwood Pit via a small topographic divide along its southern perimeter. The future pond water elevation is estimated to be approximately 321 to 322 metres above sea level (masl), as described further below.

3.0 PHYSICAL SETTING

The following subsections provide a general overview of the Site and surrounds physical setting under the Existing Scenario.

3.1 Climate

The Site is located approximately 22 kilometres (km) west of the Environment Canada (EC) Port Perry climate station. The Port Perry period of record spans 21 years (1984 – 2005) and is a reasonably proximal dataset to characterize average climatological conditions in the vicinity of the Site.

Based on the Port Perry station data, average annual precipitation is 874 millimetres per year (mm/yr) and the average annual temperature is 6 degrees Celsius. Based on Site land use, the evapotranspiration per EC data is 558 mm/yr with a resulting surplus of 316 mm/yr.

3.2 Existing Land Use

Site land use is largely cropland with the approximate eastern third being used as an equestrian facility. Several barn structures are located centrally within the Site. The Site perimeter and a minor portion of its interior is tree plantation.

North of the Site lies the Canadian National Railway (CNR) corridor; beyond that are large estate properties or farmland on Wagg Road (Figure 1). South and west of the Site is the existing Lafarge Goodwood Pit. Several privately owned residential properties are located beyond the southeast corner of the Site on the west side of Concession Road 4 and immediately east of Concession Road 4. Part Lot 20, at the southeast corner of Wagg Road and Concession Road 4, is licensed by S.A.S.E. Aggregates for a Class A Pit Above Water aggregate extraction.

3.3 Topography and Drainage

The Site lies just north of the topographic crest of the Oak Ridges Moraine (ORM), a major physiographic landform in the region (Figure 1). A ground elevation high of approximately 348 masl exists in the southwest corner of the Site whereas a low of approximately 342 masl occurs in the northeast portion of the Site (Figure 2).

There are no permanent surface water features on-Site. According to the property tenant, who has occupied the land since 2001, flowing surface water (i.e. runoff) is typically observed only during the spring melt. During this period, the runoff either: 1) ponds within localized depressions and infiltrates; or 2) exits the Site via topographic lows at the north, east and south of the Site (Figure 2). Additional details regarding local drainage patterns are described Section 5 Water Budget.

Regionally, the Site is located within the Pefferlaw River subwatershed, which itself lies within the Lake Simcoe watershed. The only naturally occurring surface water features mapped within 1 km of the Site are two small

wetland areas located to the northeast along the rail corridor (described herein as "Wetland 1" and Wetland 2") and a third wetland to the northwest ("Wetland 3") (Figure 1). None of these wetlands have been evaluated as Provincially Significant. Previous studies have indicated that Wetlands 1 and 2 are perched above the water table (Harden, 2014), a conclusion that aligns with our assessment of aquifer groundwater levels in the area. The same is likely true for Wetland 3, given that proximal well records (MECP, 20211) list water table depths greater than 20 m below ground surface in this area. Based on our review of local drainage patterns, Wetlands 1 and 2 have no hydrologic relationship to the Site. However, Wetland 3 has some relevance to the Study as described later in Section 5 Water Budget.

3.4 Geology and Hydrostratigraphy

The Site aggregate resource is located within the Oak Ridges Moraine Aquifer Complex (ORAC). The ORAC is generally comprised of sands and gravels with localized deposits of finer grained materials. Regional studies suggest that the ORAC is over 50 m thick in this area (Earthfx, 2006). The ORAC is underlain by the Newmarket Till, a thick and regionally extensive aquitard. Figure 3 provides a geologic / hydrostratigraphic cross-section based on Site borehole logs and Ministry of Environment, Conservation and Parks (MECP) Water Well Information System (WWIS) database well records (MECP, 2021¹).

3.5 Regional Recharge and Groundwater Flow

Regional groundwater modelling undertaken for the Regional Municipality of Durham suggests that recharge rates in the vicinity of the Site are on the order of 200 mm/yr (Earthfx, 2010).

Based on the same study, the Site is inferred to be just downgradient of a regional groundwater divide within the ORAC, with groundwater flowing in a generally northwards direction through the Site at groundwater elevations in the +/- 320 masl range (Earthfx, 2010).

3.6 Water Users

The MECP WWIS indicates that there are 31 water wells within 500 m of the Site (Figure 4). The wells are drilled to depths ranging from 28 mbgs to 57 mbgs, with the exception of one observation well which has a depth of 9 mbgs. The majority of the wells are completed within sand/gravel units which are presumed to be part of the ORAC. The primary use for most wells is classed as "Domestic" (27 wells); the remaining wells are "Livestock" (1 well), "Observation Wells" (1 well), and "Unknown" (2 wells). A domestic supply well, Well ID 1909612, is located on the property.

According the MECP Permit To Take Water database (MECP, 2021²), the closest large water user in the area is Wyndance Golf Club (ClubLink Corporation), with the water source mapped about 1.6 km southeast of the Site. The Wyndance Golf Club has a permit (2872-A8YKPF) to withdraw up to 4,900 m³/d of groundwater for irrigation and communal supply purposes.

3.7 Source Water Protection and Regulated Area Considerations

The Site's location within the South Georgian Bay Lake Simcoe Source Protection Region is examined as part of this Study. Using the Lake Simcoe Region Conservation Authority (LSRCA) interactive mapping tool (accessed online August 2019), the Site's address was input into the Vulnerable Area Search query with the following result:

"This property is not in a vulnerable area. Policies found in the South Georgian Bay Lake Simcoe (SGBLS) Source Protection Plan do not apply." The closest source water protection vulnerable area is a community of Uxbridge municipal Wellhead Protection Area (WHPA), which lies over 6 km northeast of the Site and is thus not a concern in this Study.

In addition, the Site lies external to any LSRCA Regulated Area drainages as governed under O. Reg. 179/06.

4.0 FIELD PROGRAM

A Site field program was initiated in 2018 with the objectives of characterizing hydrogeologic conditions at the Site, including: geologic units, water levels, hydraulic conductivity and water quality. The following subsections describe the methodology and results of the field program in detail.

4.1 Borehole Drilling

The Site monitoring network includes three monitoring wells (MW18-01, MW18-02, and MW18-03) as shown on Figure 2. Note that the on-Site domestic well cannot be readily monitored as the well is shut-in with live electrical. A monitoring well summary is provided in Table 1; detailed borehole logs are provided in Appendix B. The following is noted:

Well Location. The monitoring wells are strategically placed near the Site corners to establish Site-wide water level patterns. The well locations and elevations were surveyed by a professional land surveyor.

Construction. The wells were constructed in April 2018. Boreholes were advanced by Choice Sonic Drilling using a SDC 550 track mounted Sonic drill rig. Core barrel dimensions were 165 mm outer diameter / 114 mm inner diameter. Borehole depths ranged from 36.9 metres below ground surface (mbgs) to 38.1 mbgs. The boreholes were completed as monitoring wells using 50.8 mm diameter Schedule 40 PVC pipe with approximately 6.1 m lengths of 10-slot screens positioned within the ORAC / saturated target resource. The pipe stick ups were enclosed within a protective steel casing cemented into the ground and locked. The wells were developed upon installation.

Geology and Hydrostratigraphy. A cross-section illustrating Site geology is provided in Figure 3. In general, the subsurface conditions from hole to hole primarily consists of sand to sand and gravel with the exception of shallow (<3 mbgs) topsoil samples, some of which are observed to contain silty or clayey components. The borehole logs support the conceptual hydrostratigraphy of an unconfined sand and gravel aquifer (ORAC).

4.2 Water Level Measurements

Groundwater level monitoring at the Site began in May 2018 with monthly monitoring events occurring thereafter. All water level monitoring is conducted manually using a water level probe. Water levels are listed in Table 1 whereas groundwater hydrographs are illustrated on Figure 5. Lastly, an inferred high water table map for the unconfined sand and gravel aquifer is provided in Figure 6. The following observations are noted:

- Depth to water ranges from 23.65 mbgs to 26.77 mbgs depending on the well and the time of year. (Table 1). Correspondingly, groundwater elevations range from 319.96 masl to 322.25 masl with a Site-wide average of approximately 321 masl. The groundwater elevations are approximately 11 m to 12 m above the proposed final extraction elevation of 310 masl.
- Groundwater levels at each well fluctuate between approximately +/- 0.4 m during the period of record (Figure 5). Water levels remain fairly stable during May 2018 to March 2019 whereupon they somewhat steadily rise into the summer of 2020 with the highest recorded water levels occurring in July August 2020. Thereafter water levels being to decline for the remainder of 2020 and into 2021 and 2022. This water table rise/fall

trend, spanning the course of several years, is perhaps less common than the shorter annual rise and fall often observed in other areas of Ontario. It is speculated that this behaviour is, at least in part, attributable to the relatively thick unsaturated zone providing a moderated response to surficial infiltration.

An inferred high-water table map is developed using water levels measured during the July 31, 2020 event (Figure 6). Consistent with other monitoring events, the on-Site flow pattern during this period is from roughly south to north / northeast. The highest water level occurs to the southeast at MW18-02 (322.25 masl) and the lowest water level occurs to the northeast at MW18-01 (320.97 masl). Under these circumstances the flow gradient across the Site may reach up to 4E-3 m/m, or 0.4%.

Well ID	ell ID MW18-01		MW1	18-02	MW18-03		
East83/North83	645,890	4,879,758	645,868	4,879,445	645,315	4,879,385	
Ground Elev (masl)	34	13.74	346.43		346	5.63	
Pipe Elev (masl)	nasl) 344.62		347	7.34	347	' .47	
Well Depth (mbgs)	3	6.88	38	.10	36	.88	
Date	Water Level (mbgs)	Groundwater Elev (masl)	Water Level (mbgs)	Groundwater Elev (masl)	Water Level (mbgs)	Groundwater Elev (masl)	
22-May-18	24.44	320.18	25.89	321.45	26.12	321.36	
27-Jun-18	24.39	320.23	25.85	321.49	26.10	321.38	
17-Jul-18	24.41	320.21	25.86	321.48	26.13	321.34	
10-Aug-18	24.42	320.20	25.85	321.49	26.12	321.35	
17-Sep-18	24.38	320.24	25.79	321.55	26.07	321.41	
22-Oct-18	24.44	320.19	25.84	321.50	26.14	321.33	
29-Nov-18	24.48	320.14	25.88	25.88 321.46		321.31	
20-Dec-18	24.41	320.22	25.83	321.51	26.07	321.40	
07-Feb-19	24.43	320.20	25.84	321.50	26.12	321.36	
25-Mar-19	24.40	320.22	25.86	321.48	26.17	321.31	
23-Apr-19	24.26	320.36	25.73	321.61	25.99	321.49	
14-May-19	24.28	320.34	25.73	321.61	25.98	321.50	
10-Jun-19	24.15	320.47	25.61	321.73	25.85	321.62	
18-Jul-19	24.14	320.48	25.57	321.77	25.86	321.61	
16-Aug-19	24.03	320.59	25.46	321.89	25.76	321.72	
13-Sep-19	24.02	320.6	25.43	321.91	25.73	321.74	

Table 1: Monitoring Well Summary and Groundwater Levels

Well ID	MV	V18-01	MW1	18-02	MW18-03		
16-Oct-19	23.95	320.67	25.38	321.96	25.62	321.85	
28-Nov-19	24.10	320.524	25.48	321.86	25.78	321.694	
23-Dec-19	23.99	320.635	25.39	321.96	25.65	321.82	
10-Jan-20	24.02	320.6	25.42	321.92	25.69	321.78	
26-Feb-20	23.88	320.74	25.28	322.06	25.58	321.89	
17-Mar-20	23.87	320.75	25.26	322.08	25.52	321.95	
31-Jul-20	23.65	320.97	25.09	322.25	25.42	322.05	
26-Aug-20	23.66	320.96	25.09	322.25	25.42	322.06	
29-Sep-20	23.69	320.94	25.12	322.22	25.42	322.05	
29-Oct-20	23.75	320.88	25.19	322.15	25.47	322.00	
26-Nov-20	23.74	320.88	25.19	322.15	25.45	322.03	
23-Dec-20	23.81	320.81	25.24	322.10	25.56	321.91	
26-Feb-21	23.94	320.68	25.38	321.97	25.73	321.74	
31-Mar-21	23.96	320.66	25.40	321.94	25.68	321.79	
30-Apr-21	24.00	320.624	25.45	321.90	25.71	321.757	
17-May-21	24.036	320.584	25.53	321.81	25.79	321.68	
25-Jun-21	24.09	320.53	25.53	321.81	25.91	321.56	
31-Aug-21	24.18	320.44	25.63	321.71	26.1	321.37	
27-Sep-21	24.2	320.42	25.6	321.74	26.1	321.37	
28-Oct-21	24.22	320.4	25.66	321.68	26.16	321.31	
29-Nov-21	24.36	320.26	25.795	321.55	26.16	321.31	
22-Dec-21	24.25	320.37	25.8	321.54	26.1	321.37	
29-Sep-22	24.59	320.03	26.03	321.31	26.77	320.7	
26-Oct-22	24.57	320.05	26.05	321.29	26.38	321.09	
29-Nov-22	24.67	319.96	26.13	321.22	26.52	320.95	
16-Dec-22	24.64	319.98	26.12	321.22	26.45	321.02	

4.3 Hydraulic Conductivity

Hydraulic conductivity is a parameter that quantifies the ease with which water may travel through soil. The hydraulic conductivity of coarser-grained material, such as that found on-Site, may be estimated from laboratory derived grain size distribution curves using the commonly employed Hazen Method as follows (Fetter, 2001):

$$K = C (d_{10})^2$$

Where:

- K is hydraulic conductivity in m/s;
- C is an empirical coefficient, which takes a value between 0.8 and 1.2 for medium to coarse sands (1.0 is used herein); and
- d₁₀ is the diameter of the 10th percentile grain size of the material (effective grain size) in cm.

Grain size data for Site soils are obtained from a preceding resource evaluation study (Lafarge, 2017) and are supplied in Appendix C. A total of 25 below-water samples are assessed to provide an understanding of the saturated hydraulic conductivity of the ORAC at the Site.

In summary, the Hazen Method calculations yielded aquifer material hydraulic conductivities ranging from 6E-5 m/s to 1E-3 m/s with a geometric mean of 2E-4 m/s.

Assuming a flow gradient of 4E-3 m/m (see prior section), an effective porosity of 0.25, and a hydraulic conductivity range of 6E-5 m/s to 1E-3 m/s, the groundwater velocity at the Site is estimated to range from 9.6E-7 m/s (0.08 m/day) to 1.6E-5 m/s (1.4 m/day).

4.4 Water Quality

Baseline groundwater quality conditions were evaluated by taking water samples from the three monitoring wells and the on-Site domestic well ("PW") during September and October 2018. Follow up sampling events occurred during April 2019 and October 2020. The samples were collected using dedicated Waterra Model D-25 inertial pumps and 16-millimetre (5/8 inch) inside diameter polyethylene tubing. Prior to sampling, the wells were purged of a minimum of three well volumes and allowed to recover to their approximate static water level at the time of sampling. The samples were collected in pre-supplied laboratory bottles, placed in coolers and delivered within twenty-four hours to an accredited laboratory.

The groundwater samples were analysed for the following parameters:

- Inorganic water quality parameters including metals;
- Microbiology / bacteria;
- Polycyclic Aromatic Hydrocarbons (PAHs);
- Volatile Organic Compounds (VOCs); and
- Petroleum hydrocarbons (PHC) (F1 F4).

The water quality analysis results are provided in Appendix D. Parameter concentrations were compared to "Table 2: Full Depth Generic Site Condition Standards [SCS] in a Potable Ground Water Condition" from the

- None of the inorganic parameters including metals were detected at concentrations greater than the Table 2 SCS criteria.
- Chlorides were found in all wells, suggesting impacts from road salting.
- Nitrate was found in MW18-03, suggesting impacts from fertilizer application to farm fields.
- Hardness as (CaC0₃) was greater than 200 mg/L, indicating hard water.
- Sodium was found to be above the Ontario Drinking Water Standard (ODWS) guideline of 20 mg/L for people on sodium restricted diets at MW18-01; however, sodium at the domestic well was less than 20 mg/L.
- Total dissolved solids (TDS) was found to be above the ODWS aesthetic objective of 500 mg/L at MW18-01.
- E. Coli bacteria was present in MW18-01 and total coliforms were found in MW18-03. The presence of bacteria in these wells may be as a result of fertilizer application or faecal waste from farm animals.
- PAHs were not detected in the two wells tested (MW18-02 and MW18-03).
- VOCs were not detected in the four wells.
- PHCs (F3 fraction) were found at MW18-01 in September 2018 at a concentration of 2,400 ug/L, notably above the Table 2 SCS criteria of 500 ug/L. Additionally, PHCs (F2 and F3) were detected at MW18-03 in October 2018 but at concentrations below Table 2 SCS criteria. The location of the PHC detects could suggest that the railway corridor is a potential source. We further note that the on-Site domestic well and upgradient well MW18-02 did not have PHCs. The results of subsequent PHC-focused sampling events are summarized as follows:
 - April 2019: Sampling at all wells did not detect any PHCs except for MW18-03 which had PHC F3 at a concentration below Table 2 SCS.
 - October 2020: Well MW18-03 could not be sampled as a result of the waterra tubing snapping in the well. Sampling at all the remaining wells did not detect any PHCs except for MW18-01 which had a PHC F3 concentration at the Table 2 SCS limit of 500 ug/L.
 - May 2021: Sampling at all wells did not detect any PHCs.

The sampling regime to-date generally suggests that the presence of PHC has been declining at tested wells and, as of the most recent sampling event, is no longer detected. As such, the historic presence of PHC is not considered a factor in this licence application. Nonetheless, we recommend continued confirmatory water quality sampling during operations (Section 8).

5.0 WATER BUDGET

A Site water budget was conducted to estimate the average annual water balance for Existing and Rehabilitated Scenarios. The Operational Scenario was not explicitly considered because the range of its potential effects are captured between the Existing and Rehabilitated Scenario results.

5.1 Approach

The water budget employs Environment Canada procedures (Johnstone and Louie, 1983) and is governed by the following generalized formula:

Rainfall + Snowmelt – Evapotranspiration – Change in Soil Storage = Surplus

The Environment Canada Port Perry station data (1984 – 2005) provides monthly water budget summaries used to infer average annual climatic conditions at the Site. These water budget summaries contain monthly average precipitation, evapotranspiration and surplus values (in mm) for a range of water holding capacities (WHC). For temperate regions, the change in soil storage is relatively stable year-round and represents a minor component of the annualized water budget; as such, it is ignored in this analysis.

The Site's average annual precipitation (rainfall + snowmelt) totals approximately 874 mm/yr. Whereas precipitation values are independent of the Site's physical characteristics, evapotranspiration – and thus surplus – depends on the selected WHC for a given catchment. WHCs are specific to the soil type and land use and may be applied using Table 3.1 of the *Ministry of Environment (MOE) Stormwater Management Planning and Design Manual* (MOE, 2003). The following WHCs are implemented at the Site:

- 150 mm for moderately rooted crops and pasture areas;
- 250 mm for forested areas;
- 100 mm for the pit side;
- Limited evaporation for impermeable surfaces (Surplus = 90% Precipitation); and
- 0 mm for pond water (Potential ET = Actual ET).

Our approach further separates surplus into infiltration and runoff. Infiltration estimates for each land use may be obtained using the infiltration factors shown in Table 3.1 of MOE, 2003. Current and future land use at the Site is identified as either Crop Land (the farm fields), Pasture (sloped and flat), Forest (natural and rehabilitated), Impermeable (rooftops) and Pond Area (flooded extraction area below groundwater table). The infiltration factors for each land use is estimated as the sum of the cover, soil type, and topography factors. Infiltration factors represent the proportion of surplus becoming infiltration with the remainder of the surplus going to runoff. Since the Rehabilitated Site is internally draining (i.e. no discharge from pit bottom to surface), any runoff within the berm area will eventually become infiltration as it reports to the pit. Any infiltration factors applied in this analysis are shown in Table 2.

VEGETATION COVER	Water Holding	Infiltration Factor						
	Capacity (mm)	COVER FACTOR	SOIL TYPE COVER	TOPOGRAPHY FACTOR	FINAL INFILTRATION FACTOR			
Crop Land	150	0.1	0.4	0.2	0.7			
Pasture	150	0.15	0.4	0.2	0.75			

Table 2	2: La	nd Use	Water	Budget	Input Data
	<u>u</u>	114 030	T utor	Duugot	Input Dutu

VEGETATION COVER	Water Holding	Infiltration Factor						
	Capacity (mm)	COVER FACTOR	SOIL TYPE COVER	TOPOGRAPHY FACTOR	FINAL INFILTRATION FACTOR			
Forest and Rehab, Vegetation	250	0.2	0.4	0.2	0.8			
Rooftops/ Impermeable surfaces	3	0	0	0	0			
Rehab Pit Sidewall	100	0.1	0.4	0.1	0.6			
Pit Pond	AET = PET	-	-	-	1			

5.2 Catchment Areas

Site catchment (i.e. drainage) areas are delineated for Existing and Rehabilitated Scenarios based on topographic mapping provided by MHBC (Appendix A).

5.2.1 Existing Scenario

Under the Existing Scenario the Site is divided into three catchments based on the direction of natural drainage (Figure 7):

- Catchment 101 (6.3 ha) draining south towards the existing Lafarge Goodwood pit. The existing pit floor appears to have no natural outlet, suggesting it drains internally to infiltration with no external runoff;
- Catchment 102 (2.8 ha) draining north across a low point along the rail line, and from there draining northwest to Wetland 3 approximately 600 m northwest of the Site; and
- Catchment 103 (8.8 ha) draining to the east via sheet flow to a roadside ditch, across Concession Road 4 via culvert and then infiltrating within adjacent farm field.

The Existing scenario includes a combination of Pasture, Cropland, and Impermeable areas (i.e. rooftops, roads). Pertinent characteristics of the Existing Scenario catchment are summarized in Table 3.

Table 3: Existing Scenario Land Use

CATCHMENT	Cropland (ha)	Pasture (ha)	Forest and Rehab. Vegetation (ha)	Rooftops and Impermeable Surfaces (ha)	Rehab Pit Sidewall (ha)	Pond, Wetland, and Flooded Quarry (ha)	Total (ha)
101 - Existing South	5.4	0.0	0.8	0.0	0.0	0.0	6.3
102 - Existing North	0.8	1.7	0.2	0.1	0.0	0.0	2.8
103 - Existing East	0.0	7.2	1.1	0.5	0.0	0.0	8.8
Total	6.2	8.9	2.1	0.6	0.0	0.0	17.9

5.2.2 Rehabilitated Scenario

The Rehabilitated Scenario contemplates a 5.7 ha pit pond with surrounding rehabilitated pit slopes (9.7 ha), assumed to be pasture-type land with long grasses, draining towards the pond (Figure 8). Setback areas outside of the extraction boundary which used to drain to the north and east (2.1 ha) would continue to drain externally to the north and east. The southerly setback area which used to drain south to the adjacent pit (0.5 ha) would be cut off from the adjacent pit and would instead drain to the pond. In the Rehabilitated Scenario all surplus within the former extraction area is considered infiltration – whether that occurs within the rehabilitated sloping or into the pond itself (the pond being a manifestation of the groundwater table at surface). Pertinent characteristics of the Rehabilitated Scenario catchment are summarized in Table 4.

CATCHMENT	Cropland (ha)	Pasture (ha)	Forest and Rehab. Vegetation (ha)	Rooftops and Impermeable Surfaces (ha)	Rehab Pit Sidewall (ha)	Pond, Wetland, and Flooded Quarry (ha)	Total (ha)
201 - Buffer South	0.0	0.4	0.1	0.0	0.0	0.0	0.5
202 - Buffer North	0.0	0.4	0.1	0.0	0.0	0.0	0.5
203 - Buffer East	0.0	1.5	0.1	0.0	0.0	0.0	1.6
204 - Rehab Pit	0.0	0.0	0.0	0.0	9.7	5.7	15.4
Total	0.0	2.2	0.3	0.0	9.7	5.7	17.9

Table 4: Rehabilitated Scenario Land Use

5.3 Results

5.3.1 Existing Scenario

The water budget results for the Existing Scenario catchments are listed in Table 5 in both mm/yr and m³/yr. As mentioned previously, the Port Perry climate station records an average annual precipitation of 874 mm/yr (156,300 m³/yr). Under the Existing Scenario, the overall Site water budget results in an area-weighted average evapotranspiration rate of 558 mm/yr (99,800 m³/yr) and corresponding surplus of 316 mm/yr (56,600 m³/yr) as determined based on data obtained from the Environment Canada dataset and land use information. Based on the catchment infiltration factors, an infiltration rate of 214 mm/yr (38,300 m³/yr) is estimated with a corresponding runoff of 102 mm/yr (18,300 m³/yr).

On an individual catchment scale, the runoff for Catchment 101 (5,400 m³/yr), Catchment 102 (2,600 m³/yr) and Catchment 103 (10,300 m³/yr) are notable in that these are flow exiting the Site. Catchment 101 contributes runoff to the exiting Lafarge Goodwood pit, Catchment 102 contributes runoff to (ultimately) Wetland 3, and Catchment 103 contributes runoff to the agricultural field east of the Site adjacent to Concession Road 4.

Table 5: Existing Scenario Results

CATCHMENT	TOTAL AREA			EVAPO- TRANSPIRATION		SURPLUS		INFILTRATION		RUNOFF	
	(HA)	MM/YR	M ³ /YR	MM/YR	M³/YR	MM/YR	M³/YR	MM/YR	M³/YR	MM/YR	M³/YR
101 - Existing South	6.3	874	54,800	574	36,000	300	18,800	214	13,400	86	5,400
102 - Existing North	2.8	874	24,300	562	15,600	312	8,700	218	6,100	94	2,600
103 - Existing East	8.8	874	77,300	545	48,200	329	29,100	213	18,800	116	10,300
TOTAL	17.9	874	156300	558	99,800	316	56,600	214	38,300	102	18,300

5.3.2 Rehabilitated Scenario

The water budget results for the Rehabilitated Scenario catchments are listed in Table 6. Site rainfall remains unchanged (874 mm/yr or 156,300 m³/yr), but there is a slight increase in overall evapotranspiration (567 mm/yr or 101,400 m³/yr), reflecting the increased evaporation from the surface of the pond. It follows that total surplus under the Rehabilitated Conditions (55,000 m³/yr) remains similar to the Existing Scenario (56,6000 m³/yr). Total infiltration under Rehabilitated Conditions (53,500 m³/yr) is significantly greater than the Existing Conditions infiltration (38,300 m³/s). Conversely, there is a reduced runoff in the Rehabilitated Conditions (1,500 m³/yr) as opposed to the Existing Conditions (18,300 m³/yr), as the proposed extraction area would drain internally under Rehabilitated Conditions.

On an individual catchment scale, the runoff exiting the Site to the north (300 m³/yr), south (0 m³/yr), and east (1,200 m³/yr) are reduced relative to the Existing Scenario. The implications of the runoff reductions are discussed in Section 6 Impact Assessment.

Table 6: Rehabilitated Scenario Results

CATCHMENT	TOTAL AREA	PRECIPITATION		EVAPO- TRANSPIRATION		SURPLUS		INFILTRATION		RUNOFF	
	(HA)	MM/YR	M³/YR	MM/YR	M³/YR	MM/YR	M³/YR	MM/YR	M³/YR	MM/YR	M³/YR
201 - Buffer South	0.5	874	4,000	571	2,600	303	1,400	303	1,400	0	0
202 - Buffer North	0.5	874	3,900	576	2,600	298	1,300	226	1,000	71	300
203 - Buffer East	1.6	874	13,900	572	9,100	302	4,800	227	3,600	75	1,200
204 - Rehab Pit	15.4	874	134,500	566	87,100	308	47,500	308	47,500	0	0
TOTAL	17.9	874	156,300	567	101,400	307	55,000	299	53,500	8	1,500

6.0 IMPACT ASSESSMENT

The impact assessment seeks to estimate potential changes to the hydrogeologic / hydrologic system as a result of Site Operations and Rehabilitation Scenarios and the effect these changes may have on groundwater users / receptors. Our analysis focuses on impacts to the following:

- Groundwater levels;
- Baseflow;
- Water well quantities;
- Baseflow to water features;
- Aquifer vulnerability and groundwater quality;
- Groundwater temperature; and
- Site water budget.

6.1 Groundwater Levels

The below-water operation will not involve any pumping or active dewatering. Rather, the majority of pore water removed during extraction will eventually return to the aquifer via passive drainage within the stockpiled material.

Thus, the principal mechanism for Site development to instigate long-term effects on groundwater levels is exposing the water table to the atmosphere. Below-water aggregate extraction results in the eventual creation of a permanent pond that will tend to "flatten" water levels in its vicinity. The area upgradient of the pond experiences water level drawdown whereas the area downgradient of the pond experiences water level rise. Typically, only drawdown is of concern with respect to water quantity impacts. Under this paradigm the Rehabilitation Scenario provides the "worst-case" outcome with respect to long-term drawdown.

It is notable that the positioning of the (eventual) pond at the existing Goodwood Pit to the south would tend to ameliorate long-term Site drawdown because of the water level rise induced at the existing Goodwood Pit's northerly pond edge. However, for the purposes of this impact assessment, such an effect is conservatively not taken into account, and the drawdown effect of the Site pond is examined in isolation.

The Rehabilitation Scenario pond level is expected to approximate the average groundwater level within its area. Under high water table conditions (Figure 6), the water level within the pond area is estimated to be 322 masl based on the contouring program Surfer (Golden Software, 2015). The average annual water level would be close to, but less than, 322 masl (likely between 321 masl to 322 masl). Thus, for the purposes of this impact assessment, an estimate of future drawdown along the upgradient (southern) perimeter of the pond may be obtained by taking the existing upgradient high groundwater elevation of 322.25 masl (at MW18-02) and subtracting the future pond level of 322 masl resulting in a drawdown of 0.25 m at the upgradient pond perimeter. Likewise, a water level *increase* of approximately 0.25 m could be expected along the downgradient (northern) perimeter of the future pond. Water level changes of similar magnitude could be expected during different times of the year as the pond and surrounding groundwater levels would jointly rise and fall over a seasonal time scale.

To estimate the extent of the pond's lateral zone of influence (i.e. where drawdown reaches zero) we use the analytical method *Simple analytical equations for estimating ground water inflow to a mine pit* (Marinelli and

Niccoli, 2000). An illustration of this method along with Site specific inputs and calculations are provided in Appendix E. In summary, the inputs are as follows:

- The pit pond area is expected to be 5.7 ha, or 57,000 m². The effective radius is approximated as:
 - A_{pond} = pi * r_e^2
 - $r_e = [(57,000 \text{ m}^2) / (3.14)]^{1/2}$
 - r_e = 135 m
- Recharge is assumed to be 214 mm/yr per the Site water budget;
- The hydraulic conductivity of the ORAC is assumed to be 2E-4 m/s per grain size analysis;
- The pit pond floor is assumed to be 310 masl. As a result, under high water table conditions:
 - The initial (i.e. pre extraction) saturated thickness is 322.25 m 310 m = 12.25 m
 - The final (i.e. post extraction) saturated thickness is 322 m 310 m = 12 m.
 - Note that 12.25 m minus 12 m results in the aforementioned drawdown of 0.25 m.

Based on the above inputs, the 0.25 m drawdown is calculated to decrease to a point of zero drawdown approximately 474 m from the centre of the pit pond (Figure 9).

6.2 Baseflow

Baseflow is the groundwater contribution of total flow to a surface water feature. Baseflow changes as a result of below-water extraction are related to water level changes (see above). A surface water receiver downgradient of a water level decline may experience decreased baseflow to that feature; conversely, a surface water feature downgradient of a water level rise may experience increased baseflow. Typically, only baseflow decrease is of concern within the context of impact assessment.

The only mapped surface water features in the area lie several hundred metres downgradient of the Site and these features are either known or suspected to be perched above the water table. As such, there are no significant baseflow contributions at these locations. Nonetheless, as water levels will rise downgradient of the pit pond, any theoretical groundwater receptors downgradient would receive a slight increase in baseflow and as such would not experience an adverse effect to water quantity.

6.3 Well Water Quantities

There are four MECP well records within the drawdown zone of influence (Figure 9); one of these is an observation well (7307824) and is not considered in the context of water quantity impacts. The available water column in the remaining water supply wells is compared to the estimated drawdown at each well location as inferred from the previously described analytical calculations (Table 7). Drawdown at the wells is <0.01 m with a corresponding <0.1% reduction in available column. Such minor reductions will not impact well operation. Nonetheless, in the unlikely event that a neighbouring well owner perceives an adverse impact to their well, Lafarge should develop a Water Well Complaint Action Plan. A suggested plan is provided in Section 6.3.1.

Well ID	Construct Date	Depth (mbgs)	Static Level (mbgs)	Water Column (m) ¹	Distance From Pit Pond (m)	Estimated Drawdown (m)	Reduction In Water Column
1917600	Apr-05	54.9	25.4	29.4	400	<0.01	<0.1%
4602905	Feb-62	35.1	8.2	26.8	450	<0.01	<0.1%
1907003	Jun-84	27.7	14.3	13.4	455	<0.01	<0.1%

Table 7: Water Well Records In Drawdown Zone of Influence

Note 1: Water Column = Well Depth – Static Water Level

6.3.1 Water Well Compliant Action Plan

The EMP should include a Water Well Complaint Action Plan. If a water well complaint regarding a domestic well is received by Lafarge within 500 metres of the Site the following actions should be taken:

- The licensee will contact local MECP-licensed well contractors in the event of a well malfunction and the well owner will be immediately offered a temporary water supply at the licensee's expense if the issue cannot be expediently determined and rectified.
- The designated contractor will respond to the well owner and propose a plan to rectify the problem as expediently as possible. The well owner must then provide authorization of the work.
- If the issue raised by the well owner is related to loss of water supply the licensee will have a consultant/contractor determine the likely cause, which can result from a number of factors, including climatic conditions such as drought (owner's expense), pump failure (owner's expense), extended overuse of the well (owner's expense) or the lowering of the water level in the well from extraction activities (Lafarge expense).
- Data from the Site monitoring network shall be consulted as part of the review to determine groundwater level behaviour. This assessment process will be carried out at the expense of Lafarge and the results provided to the well owner.
- The consultant/contractor will be able to readily determine if pump failure is the problem and, should the well owner choose to have the pump repaired or replaced at their expense, the contractor would correct the situation for the well owner.
- If it has been determined that extraction activities did not cause the water supply interference, then the licensee shall provide 24 hours' notice and thereafter discontinue the temporary water supply.
- If it has been determined that extraction activities caused the water supply interference, then the licensee shall continue to supply water to the well owner at the licensee's expense until the problem is rectified and the water supply is restored. The following mitigation measures shall be considered and the appropriate measure(s) implemented at the expense of the licensee:
 - Adjust pump pressure;
 - Lowering of the pump to take advantage of existing water storage within the well;

- Deepening of the well to increase the available water column;
- Widening of the well to increase the available storage of water;
- Relocation of the well to another unaffected area on the property; or
- Drilling of multiple low yield wells.

6.4 Water Quality

The Operational Scenario will not involve the on-Site storage of any fuels, oils or potentially hazardous materials that could be released into the groundwater system. Therefore, water quality is not expected to be adversely impacted. Nonetheless, a Best Management Practices (BMP) manual should be employed to address any potential spills from equipment on-Site and will minimize the potential for aquifer contamination.

It bears mentioning that the Rehabilitated Scenario represents an opportunity to improve water quality as the resulting land use will negate the application of fertilizers and/or pesticides on what would otherwise be crop land.

6.5 Water Temperature

The exposure of the water table to the atmosphere may result in an increase in groundwater temperatures emanating from the Site during summer. This can occasionally be a concern for aquatic species or habitat that require the influx of cool groundwater within a certain temperature range in order to maintain ecological function.

Prior studies in Ontario have indicated that thermal plumes originating from below-water pits typically do not migrate farther than 120 m to 250 m downgradient of the pit pond before their effect becomes negligible (Yang, 1995 and Markle and Schincariol, 2007). No aquatic habitat lies within these distances from the pit pond and therefore the off-Site migration of a thermal plume will not cause any adverse impacts.

6.6 Site Water Budget

As noted in Section 5, the transition from Existing Scenario to Operations and ultimate Rehabilitated Scenario will result in some losses to off-Site runoff as a result of re-grading and land form changes as follows (with reference to Table 6 versus Table 7):

- Catchment 101 (South): The elimination of southerly off-Site runoff (5,400 m³/yr to zero) to the existing Lafarge Goodwood Pit is considered inconsequential as the location of this water (infiltration to the groundwater system via an open pit / pond) is the same under both Existing and Rehabilitated Scenarios.
- Catchment 102 (North): The decrease in northerly off-Site runoff (2,600 m³/yr to 300 m³/yr) may result in a corresponding decrease in inflow to Wetland 3. This reduction may be put into context by comparing the post-extraction decrease in overall catchment. Based on a GIS analysis, the total catchment area to Wetland 3 is approximately 41.6 hectares. The Rehabilitated Condition would result in a loss of catchment of 2.3 ha, or 6% of the total catchment. Given the consistency in land form over the catchment, it is reasonable to assume that this loss in catchment would result in a corresponding 6% reduction in flows, and these reductions would occur largely during the spring freshet when water has been observed to flow off-Site and there is sufficient surface water flow to the wetland. Other than spring freshet the site does not provide surface flow to the Wetland. This is considered a minor loss from a hydrological perspective. The significance of the runoff decrease on natural environment receptors within Wetland 3 is evaluated by under separate cover in the *Natural Environment Reports* (WSP 2023).

Catchment 103 (East): The decrease in easterly off-Site runoff (10,300 m³/yr to 1,200 m³/yr) will result in a decrease in flow to the adjacent farm field across Concession Road 4 during the spring freshet. This action may provide an overall benefit to the land form by mitigating excessive flooding during spring.

7.0 CONCLUSIONS

A Water Report Level 1 and 2 has been prepared in support of a Class A Pit Below Water licensing application for the proposed Goodwood Pit Extension (the Site). Existing, Operational and Rehabilitated Scenarios were considered. The Study involved two main aspects: 1) the establishment of baseline conditions for the Existing Scenario through background data review and field program data collection; and 2) an impact assessment for proposed Operations and Rehabilitated Scenarios. The following conclusions are drawn from the Study:

7.1 Existing Scenario

- Average annual precipitation in the Site vicinity is 874 mm/yr. Evapotranspiration is estimated to be 558 mm/yr with a resulting surplus of 316 mm/yr. The majority of this surplus becomes infiltration (214 mm/yr) with the remainder becoming runoff (102 mm/yr).
- There are no permanent surface water features on-Site. However, flowing surface water (i.e. runoff) is observed during the spring freshet. During this period the runoff either 1) ponds within localized depressions and infiltrates; or 2) exits the Site via topographic lows at the north, east and south of the Site.
- There are three wetlands that are mapped within 1 km of the Site. All are inferred to be perched and not connected to the saturated groundwater system. The only wetland that has any hydrologic connection to the Site is "Wetland 3", located to the northwest. This wetland receives a small portion of its water supply (6%) from runoff leaving the Site in a northerly direction during spring freshet when there is sufficient surface water flow to this wetland.
- The Site aggregate resource is located within the Oak Ridges Moraine Aquifer Complex (ORAC). The ORAC is generally comprised of sands and gravels with localized deposits of finer grained materials. Site drilling suggests the ORAC is at least 37 m thick in the vicinity of the Site. Estimated aquifer hydraulic conductivity ranges from 6E-5 m/s to 1E-3 m/s with a geometric mean of 2E-4 m/s.
- The Site is inferred to be just downgradient of a regional groundwater divide within the ORAC, with groundwater flowing in a generally north to northeasterly direction through the Site at elevations in the 322 to 320 masl range. Depth to groundwater at the Site ranges from 26 m to 24 m.
- There are 30 water wells within 500 m of the Site. All the wells are drilled and completed within the ORAC at depths ranging from 28 mbgs to 57 mbgs. The majority of wells are classed as "Domestic".
- Tested groundwater quality generally met Table 2 SCS criteria. However, impacts from surficial contaminants were evident in elevated chloride, nitrate, and bacteria levels. Petroleum hydrocarbons (PHCs) were initially found in several wells; however, subsequent sampling suggested a general decline in concentrations with the most recent sampling event in May 2021 indicating non-detect in all wells.
- The Site is not located within, nor does it interact with, any Source Protection Plan vulnerable area or any LSRCA Regulated Area.

7.2 Operational and Rehabilitated Scenarios

- The below-water operation will not involve any pumping or active dewatering. Rather, the majority of pore water "removed" during extraction will eventually return to the aquifer via passive drainage within the stockpiled material.
- Below-water aggregate extraction will result in the eventual creation of a permanent pond that will flatten water levels in its immediate vicinity. The area upgradient of the pond (south) will incur water level drawdown whereas the area downgradient of the pond (north) will incur water level rise. The magnitude of water level change is estimated to be on the order of 0.25 m at the pit pond with a zone of influence extending approximately 474 m from the centre of the pond to a point of zero drawdown.
- There is not expected to be any adverse impacts to baseflow at groundwater receptors as result of the aforementioned minor water level changes.
- There is not expected to be any adverse impacts to water quantity at surrounding private water wells as a result of the aforementioned minor water level changes.
- There will be no on-Site storage of any fuels, oils or potentially hazardous materials that could be released into the groundwater system; as such there is not expected to be any adverse impacts to water quality.
- The exposure of the water table to the atmosphere may result in a minor increase in groundwater temperatures emanating from the Site during summer. However, there are no groundwater-dependent aquatic habitats within the thermal zone of influence (120 m to 250 m) downgradient of the Site and thus no receptors will be adversely impacted.
- The transition from Existing Scenario to Operations and ultimate Rehabilitated Scenario will result in some losses to off-Site runoff as a result of re-grading and landform changes:
 - Catchment 101 (South): The elimination of southerly off-Site runoff (5,400 m³/yr to zero) to the existing Lafarge Goodwood Pit is considered inconsequential as the location of this water (infiltration to the groundwater system via an open pit / pond) is the same under both Existing and Rehabilitated Scenarios.
 - Catchment 102 (North): The decrease in northerly off-Site runoff (2,600 m³/yr to 300 m³/yr) may result in a corresponding decrease in inflow to Wetland 3. However, this is estimated to be only a 6% overall reduction in flow to the wetland and is thus considered a minor loss.
 - Catchment 103 (East): The decrease in easterly off-Site runoff (10,300 m³/yr to 1,200 m³/yr) will result in a decrease in flow to the adjacent farm field across Concession Road 4 during the spring freshet. This action may provide an overall benefit to the land form by mitigating excessive flooding during spring.
- This Water Report Level 1 and 2 has addressed all hydrological / hydrogeological policy considerations as set forth in Section 1.1.

8.0 **RECOMMENDATIONS**

The following is recommended:

- Groundwater monitoring shall continue through Operations. The following tasks shall be carried out:
 - Monthly manual groundwater level monitoring at the three on-Site monitoring wells.

- Annual groundwater quality sampling for general chemistry, metals, bacteria, petroleum hydrocarbons and volatile organic compounds.
- The Site domestic well will need to be decommissioned per O.Reg. 903 by a licensed well contractor either prior to extraction or at such time that extraction encroaches on the well location
- A complaint response program will be in place for residential wells within the potential draw down zone influence (i.e., 4639, 4709, 4840 and 4860 Concession Road 4) during extraction to address any complaints for surrounding residents concerning the supply of their water wells. In the event of unanticipated impacts to the water wells as a result of pit operations, mitigation and contingency measures will be implemented to restore the affected quantity or quality of the groundwater supply.
- Prior to extraction below water, a door-to-door water well survey shall be conducted to confirm baseline conditions at private wells within 500 m of the Site.

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Signature Page

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https://golderassociates.sharepoint.com/sites/141632/project files/6 deliverables/report/2023_rev 1/21453907 rep 2023'06'07 lafarge goodwood pit extension water report_final-rev1.docx

FIGURES





RAILWAY

- CONTOUR (5 m INTERVAL)
- WATERCOURSE
- WATERBODY

UNEVALUATED WETLAND

- PROVINCIALLY SIGNIFICANT WETLAND
- EXISTING GOODWOOD PIT SITE BOUNDARY
 - SITE BOUNDARY 500 m BUFFER AROUND SITE





REFERENCE(S) 1. BASEDATA: MNRF LIO OBTAINED APRIL 2019

1. BASEDATA: MINKE LIO UB TAINED APRIL 2019 2. CONTOURS SUPPLIED BY MHEC, FILE NAME "EXISTING FEATURES", DRAWING NO. 1 OF 3, FILE NO. 9526HC, DATED MAY 2019. 3. IMAGERY: SOURCES: ESRI, HERE, GARMIN, INTERMAP, INCREMENT P CORP., GEBCO, USGS, FAO, NPS, NRCAN, GEOBASE, IGN, KADASTER NL, ORDNANCE SURVEY, ESRI JAPAN, METI, ESRI CHINA (HONG KONG), (C) OPENSTREETMAP CONTRIBUTORS, AND THE GIS USER COMMUNITY

COMMUNITY SOURCE: ESRI, MAXAR, EARTHSTAR GEOGRAPHICS, AND THE GIS USER COMMUNITY 4. PROJECTION: TRANSVERSE MERCATOR DATUM: NAD 83 COORDINATE SYSTEM: UTM ZONE

17N CLIENT

LAFARGE CANADA INC.

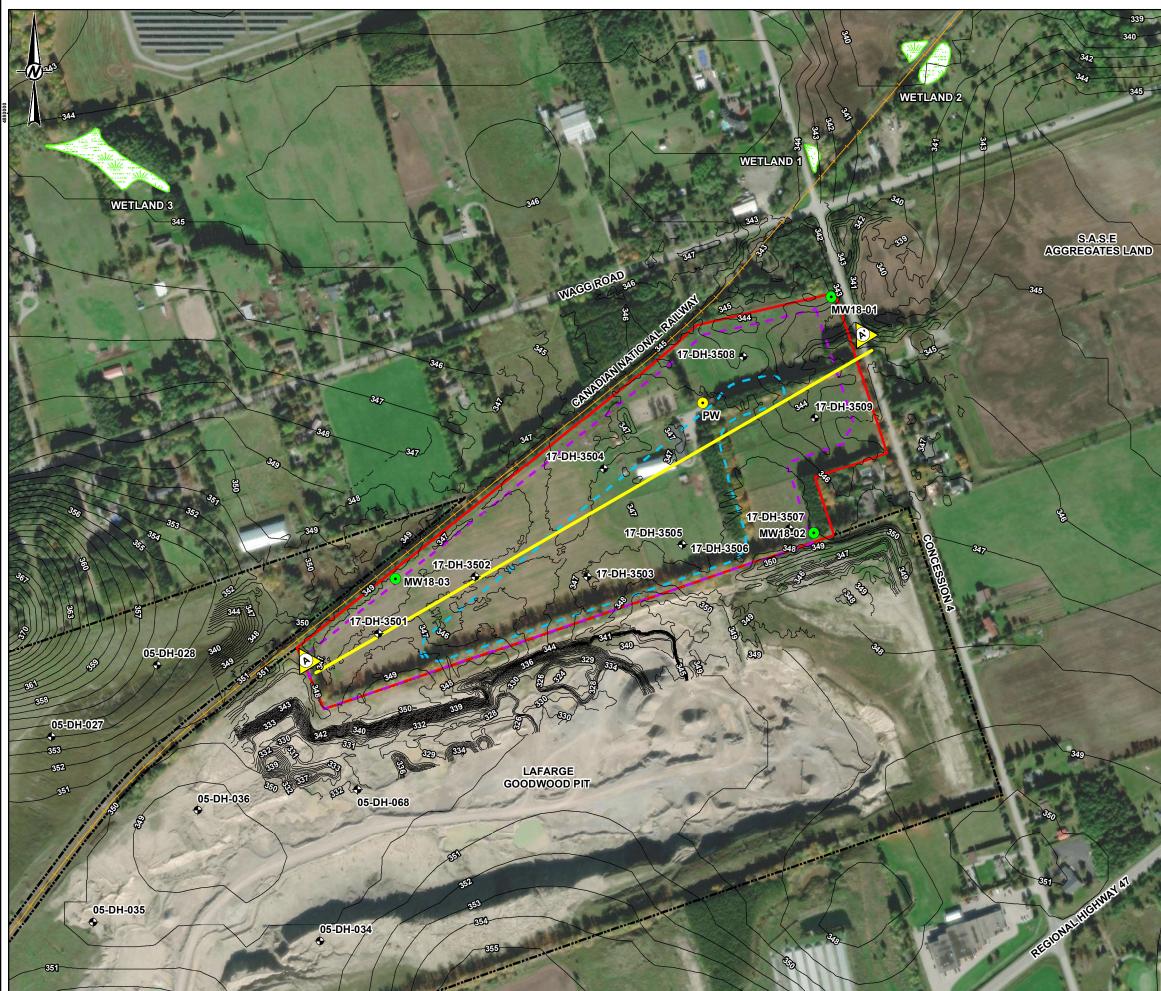
PROJECT

GOODWOOD PIT EXTENSION

4900 CONCESSION ROAD 4, TOWNSHIP OF UXBRIDGE

SITE AND SURROUNDS

CONSULTANT YYYY-MM-DD 2023-05-19 DESIGNED SO PREPARED SO/JT REVIEWED DH APPROVED PROJECT NO. FIGURE CONTROL REV. 21453907 0001 1 1



LEGEND	

•	ON-SITE DOMESTIC WELL LOCATION
•	MONITORING WELL LOCATION
•	RESOURCE EVALUATION BOREHOLE
	CONTOUR (1m INTERVAL)
	CROSS SECTION LINE
	RAILWAY
j	EXISTING GOODWOOD PIT SITE BOUNDARY
	SITE BOUNDARY
24	LIMIT OF EXTRACTION
24	APPROXIMATE BELOW WATER (PIT POND) EXTENTS
(¹⁷⁷)	UNEVALUATED WETLAND

0	100	200	300
1:5,	000	METRES	

 TIS,000
 METRES

 REFERENCE(S)
 I. BASEDATA: MNRF LIO OBTAINED APRIL 2019

 2. CONTOURS SUPPLIED BY MHBC, FILE NAME "EXISTING FEATURES", DRAWING NO. 1 OF 3, FILE NO. 9526HC, DATED MAY 2019.

 3. IMAGERY: SOURCES: ESRI, HERE, GARMIN, INTERMAP, INCREMENT P CORP., GEBCO, USGS, FAO, NPS, NRCAN, GEOBASE, IGN, KADASTER NL, ORDNANCE SURVEY, ESRI JAPAN, METI, ESRI CHINA (HONG KONG), (C) OPENSTREETMAP CONTRIBUTORS, AND THE GIS USER COMMUNITY

 SOURCE: ESRI, MAXAR, EARTHSTAR GEOGRAPHICS, AND THE GIS USER COMMUNITY

 4. PROJECTION: TRANSVERSE MERCATOR DATUM: NAD 83 COORDINATE SYSTEM: UTM ZONE 17N

17N

CLIENT

LAFARGE CANADA INC.

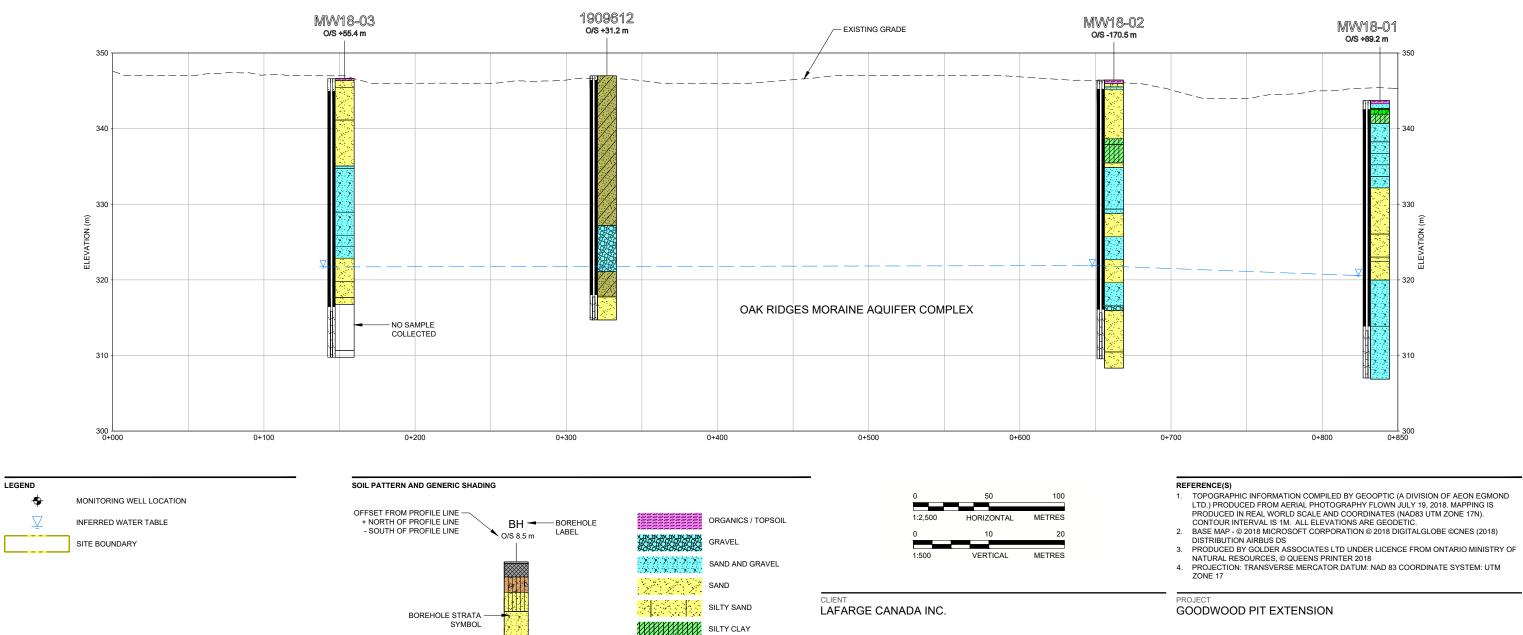
PROJECT GOODWOOD PIT EXTENSION

4900 CONCESSION ROAD 4, TOWNSHIP OF UXBRIDGE

SITE DETAIL

CONSULTANT 2023-05-19 YYYY-MM-DD DESIGNED SO 115 PREPARED SO/JT REVIEWED DH APPROVED FIGURE PROJECT NO. CONTROL REV. 21453907 0001 1





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CLAYEY SAND

CONSULTANT

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ZOO M gg gg	200 m 850.00 m

TITLE

PROJECT NO.

1894152

SITE CROSS SECTION A - A'

YYYY-MM-DD

DESIGNED

PREPARED

REVIEWED

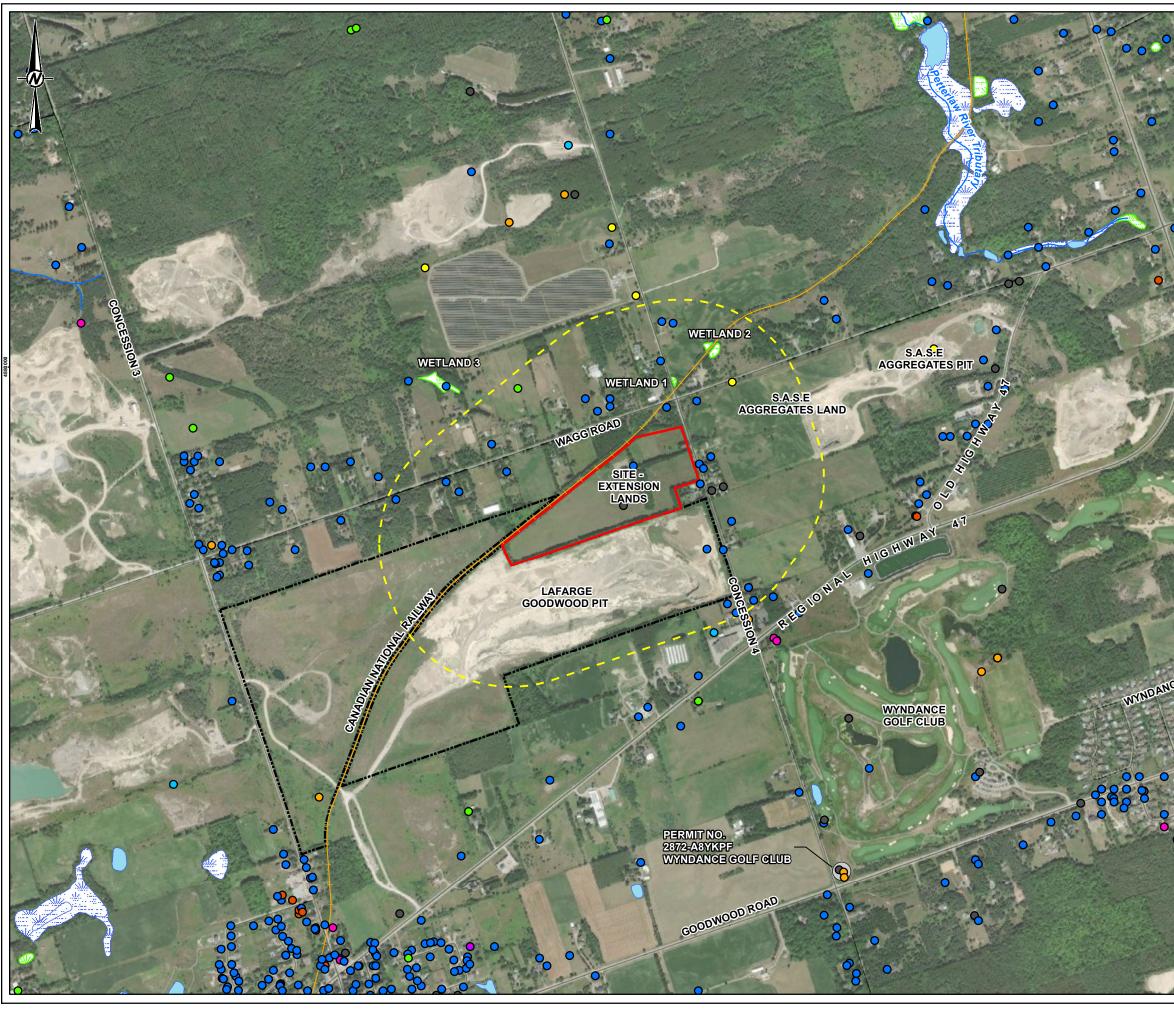
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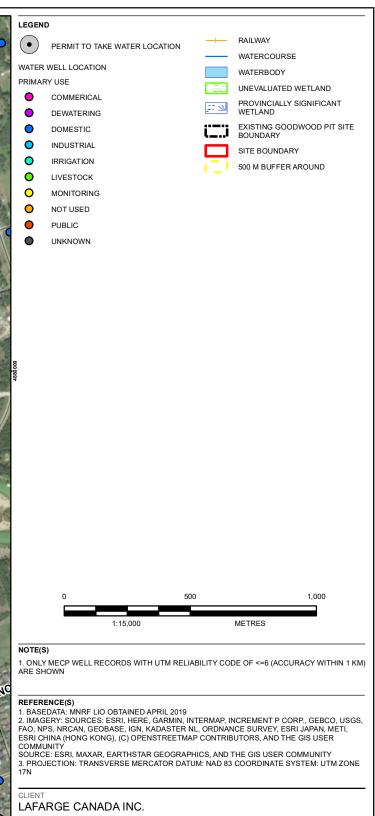
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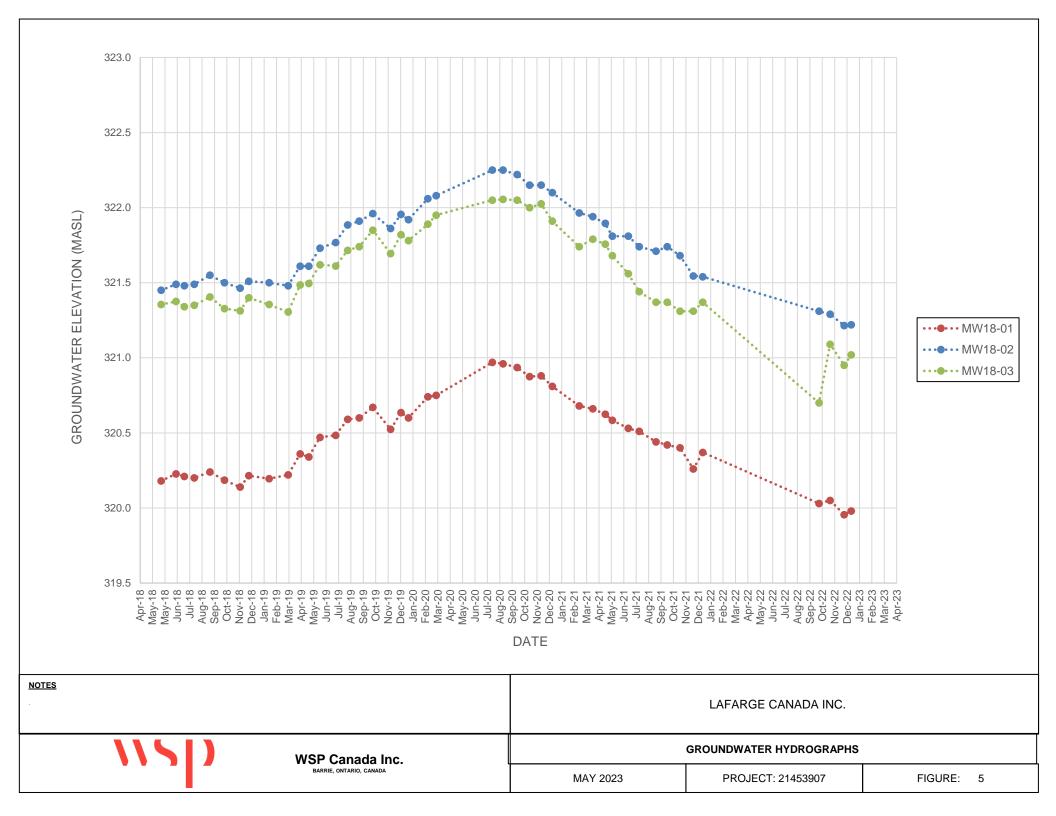
PROJECT

GOODWOOD PIT EXTENSION 4900 CONCESSION ROAD 4, TOWNSHIP OF UXBRIDGE

TITLE WATER USERS

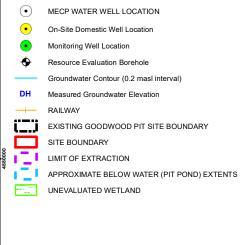
CONSULTANT

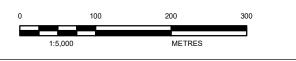
2023-05-19 YYYY-MM-DD DESIGNED SO PREPARED SO/JT REVIEWED DH APPROVED FIGURE PROJECT NO CONTROL REV. 21453907 0001 4 1











NOTE(S)

NO 1E(5) 1. SITE GROUNDWATER ELEVATIONS REFLECT OBSERVED HIGH WATER TABLE AS MEASURED DURING JULY 31, 202. 2. INDIVIDUAL MECP WELL RECORD STATIC WATER LEVELS SPAN TIME PERIOD OF 1958 TO 2017 AND MAY NOT BE REFLECTIVE OF CURRENT CONDITIONS. WATER LEVELS ARE THUS POSTED FOR ILLUSTRATION PURPOSES ONLY.

REFERENCE(S) 1. BASEDATA: MNRF LIO OBTAINED APRIL 2019 2. IMAGERY: SOURCES: ESRI, HERE, GARMIN, INTERMAP, INCREMENT P CORP., GEBCO, USGS, FAO, NPS, NRCAN, GEOBASE, IGN, KADASTER NL, ORDNANCE SURVEY, ESRI JAPAN, METI, ESRI CHINA (HONG KONG), (C) OPENSTREETMAP CONTRIBUTORS, AND THE GIS USER

COMUNITY SOURCE: ESRI, MAXAR, EARTHSTAR GEOGRAPHICS, AND THE GIS USER SOURCE: ESRI, MAXAR, EARTHSTAR GEOGRAPHICS, AND THE GIS USER COMMUNITY 3. PROJECTION: TRANSVERSE MERCATOR DATUM: NAD 83 COORDINATE SYSTEM: UTM ZONE 17N

CLIENT

LAFARGE CANADA INC.

PROJECT

GOODWOOD PIT EXTENSION 4900 CONCESSION ROAD 4, TOWNSHIP OF UXBRIDGE

INFERRED WATER TABLE

CONSULTANT

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LEGEND

- \bullet \bullet ίΞĴ
- ON-SITE DOMESTIC WELL LOCATION
- MONITORING WELL LOCATION
- RESOURCE EVALUATION BOREHOLE

CONTOUR (1m INTERVAL) RAILWAY

- EXISTING SCENARIO CATCHMENT
- EXISTING GOODWOOD PIT SITE BOUNDARY
- SITE BOUNDARY

UNEVALUATED WETLAND

Catchment	Area (ha)	Drainage Direction
101	6.3	South
102	2.8	North
103	8.8	East

0	100	200	300
1:5,0	000	METRES	

REFERENCE(S) 1. BASEDATA: MNRF LIO OBTAINED APRIL 2019 2. IMAGERY: SOURCES: ESRI, HERE, GARMIN, INTERMAP, INCREMENT P CORP., GEBCO, USGS, FAO, NPS, NRCAN, GEOBASE, IGN, KADASTER NL, ORDNANCE SURVEY, ESRI JAPAN, METI, ESRI CHINA (HONG KONG), (C) OPENSTREETMAP CONTRIBUTORS, AND THE GIS USER COMMUNEY

COMUNITY SOURCE: ESRI, MAXAR, EARTHSTAR GEOGRAPHICS, AND THE GIS USER SOURCE: ESRI, MAXAR, EARTHSTAR GEOGRAPHICS, AND THE GIS USER COMMUNITY 3. PROJECTION: TRANSVERSE MERCATOR DATUM: NAD 83 COORDINATE SYSTEM: UTM ZONE 17N

CLIENT

LAFARGE CANADA INC.

PROJECT

GOODWOOD PIT EXTENSION 4900 CONCESSION ROAD 4, TOWNSHIP OF UXBRIDGE

EXISTING SCENARIO CATCHMENT AREAS

2					
C.	CONSULTANT		YYYY-MM-DD	2023-05-19	
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1			APPROVED		
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LEGEND

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- MONITORING WELL LOCATION \bullet
 - RESOURCE EVALUATION BOREHOLE

CONTOUR (1M INTERVAL)

RAILWAY

REHABILITATION SCENARIO CATCHMENT

EXISTING GOODWOOD PIT SITE BOUNDARY

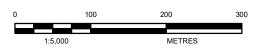
SITE BOUNDARY

PIT POND (321 - 322 MASL)

LIMIT OF EXTRACTION

UNEVALUATED WETLAND

Catchment	Area (ha)	Drainage Direction
201	0.5	South
202	0.5	North
203	1.6	East
204	15.37	Centrally to Pond



NOTE(S)

1. EXISTING LAFARGE GOODWOOD PIT AS SHOWN SUBJECT TO FUTURE CHANGE.

REFERENCE(S) 1. BASEDATA: MNRF LIO OBTAINED APRIL 2019 2. IMAGERY: SOURCES: ESRI, HERE, GARMIN, INTERMAP, INCREMENT P CORP., GEBCO, USGS, FAO, NPS, NRCAN, GEOBASE, IGN, KADASTER NL, ORDNANCE SURVEY, ESRI JAPAN, METI, ESRI CHINA (HONG KONG), (C) OPENSTREETMAP CONTRIBUTORS, AND THE GIS USER

COMUNITY SOURCE: ESRI, MAXAR, EARTHSTAR GEOGRAPHICS, AND THE GIS USER SOURCE: ESRI, MAXAR, EARTHSTAR GEOGRAPHICS, AND THE GIS USER COMMUNITY 3. PROJECTION: TRANSVERSE MERCATOR DATUM: NAD 83 COORDINATE SYSTEM: UTM ZONE 17N

CLIENT

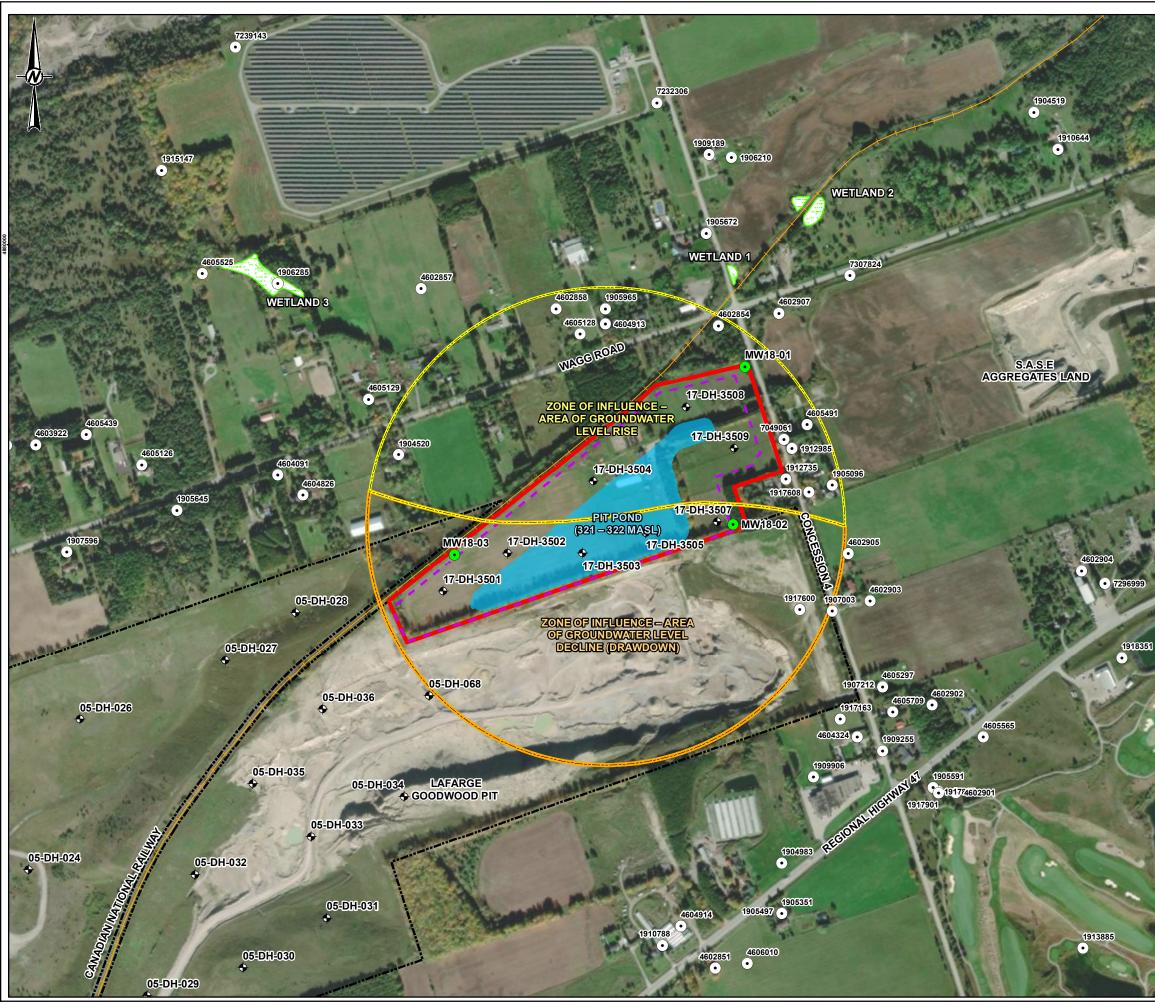
LAFARGE CANADA INC.

PROJECT

GOODWOOD PIT EXTENSION 4900 CONCESSION ROAD 4, TOWNSHIP OF UXBRIDGE

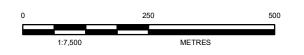
REHABILITATED SCENARIO CATCHMENT AREAS

CONSULTANT YYYY-MM-DD 2023-05-19 DESIGNED SO VV. PREPARED SO/JT REVIEWED DH APPROVED PROJECT NO. FIGURE CONTROL REV. 21453907 0001 8 1



LEGEND

 (\bullet) MECP WATER WELL LOCATION . MONITORING WELL LOCATION • RESOURCE EVALUATION BOREHOLE RAILWAY Ē EXISTING GOODWOOD PIT SITE BOUNDARY SITE BOUNDARY ZONE OF INFLUENCE – AREA OF GROUNDWATER LEVEL DECLINE (DRAWDOWN) ZONE OF INFLUENCE – AREA OF GROUNDWATER LEVEL RISE PIT POND (321 - 322 MASL) LIMIT OF EXTRACTION 6 M 1 UNEVALUATED WETLAND



NOTE(S) 1. EXISTING LAFARGE GOODWOOD PIT AS SHOWN SUBJECT TO FUTURE CHANGE.

REFERENCE(S) 1. BASEDATA: MNRF LIO OBTAINED APRIL 2019 2. IMAGERY: SOURCES: ESRI, HERE, GARMIN, INTERMAP, INCREMENT P CORP., GEBCO, USGS, FAO, NPS, NRCAN, GEOBASE, IGN, KADASTER NL, ORDNANCE SURVEY, ESRI JAPAN, METI, ESRI CHINA (HONG KONG), (C) OPENSTREETMAP CONTRIBUTORS, AND THE GIS USER COMMUNITY SOURCE: ESRI, MAXAR, EARTHSTAR GEOGRAPHICS, AND THE GIS USER COMMUNITY

3. PROJECTION: TRANSVERSE MERCATOR DATUM: NAD 83 COORDINATE SYSTEM: UTM ZONE 17N

CLIENT

LAFARGE CANADA INC.

PROJECT

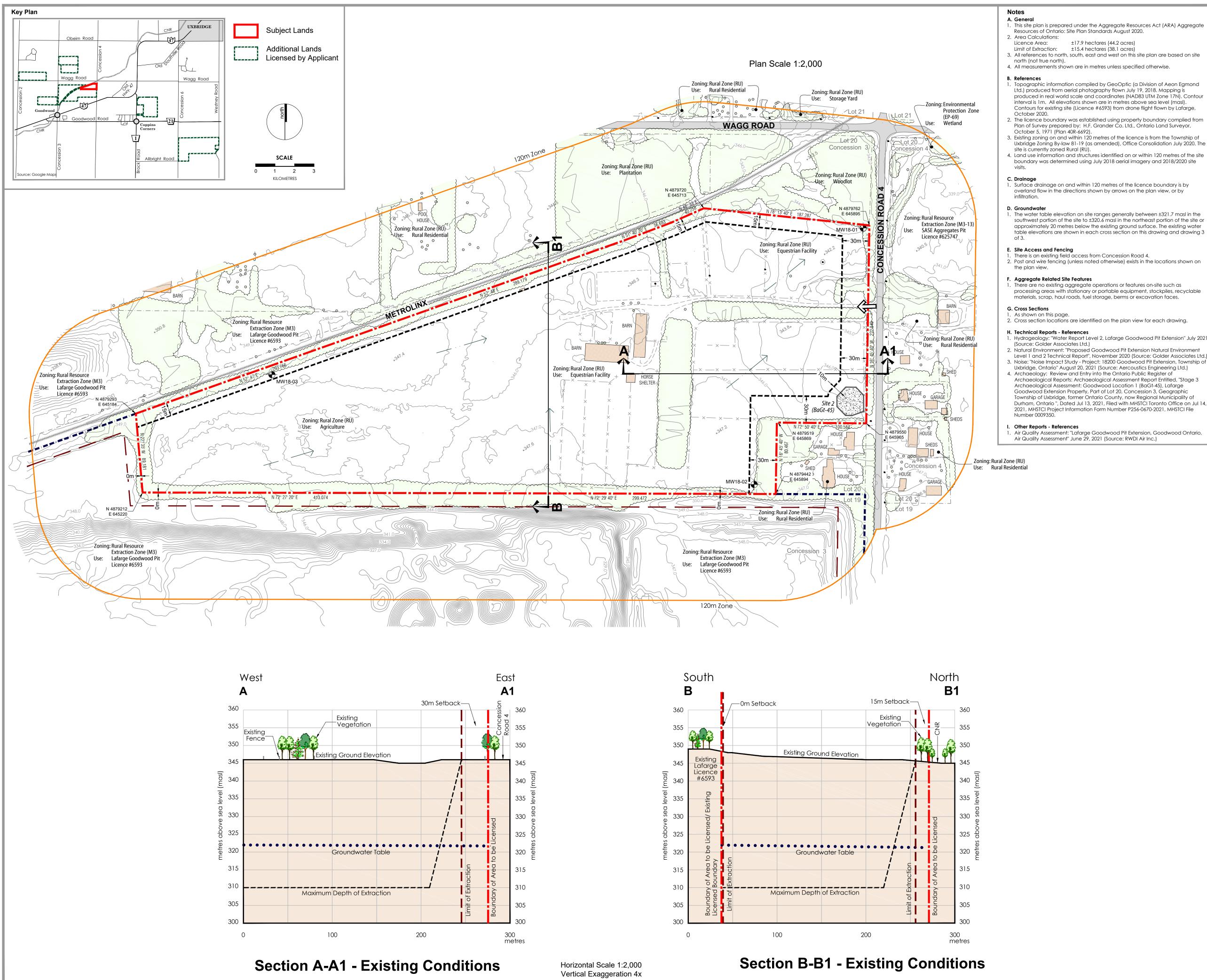
GOODWOOD PIT EXTENSION 4900 CONCESSION ROAD 4, TOWNSHIP OF UXBRIDGE

PIT POND ZONE OF INFLUENCE

CONSULTANT 2023-05-19 YYYY-MM-DD DESIGNED SO VV (PREPARED SO/JT REVIEWED DH APPROVED FIGURE PROJECT NO CONTROL REV. 21453907 9 0001 1

APPENDIX A

Draft Site Plans



	Legal Description
ider the Aggregate Resources Act (ARA) Aggregate Ian Standards August 2020.	PART OF LOT 20 CONCESSION 3
.9 hectares (44.2 acres) 5.4 hectares (38.1 acres) th, east and west on this site plan are based on site	Township of Uxbridge Region of Durham
e in metres unless specified otherwise.	Legend
ompiled by GeoOptic (a Division of Aeon Egmond photography flown July 19, 2018. Mapping is le and coordinates (NAD83 UIM Zone 17N). Contour	Boundar

Contours for existing site (Licence #6593) from drone flight flown by Lafarge, The licence boundary was established using property boundary complied from Plan of Survey prepared by: H.F. Grander Co. Ltd., Ontario Land Surveyor,

Existing zoning on and within 120 metres of the licence is from the Township of Uxbridge Zoning By-law 81-19 (as amended), Office Consolidation July 2020. The site is currently zoned Rural (RU). 4. Land use information and structures identified on or within 120 metres of the site boundary was determined using July 2018 aerial imagery and 2018/2020 site

. Surface drainage on and within 120 metres of the licence boundary is by overland flow in the directions shown by arrows on the plan view, or by

. The water table elevation on site ranges generally between ±321.7 masl in the southwest portion of the site to ± 320.6 masl in the northeast portion of the site or approximately 20 metres below the existing ground surface. The existing water table elevations are shown in each cross section on this drawing and drawing 3

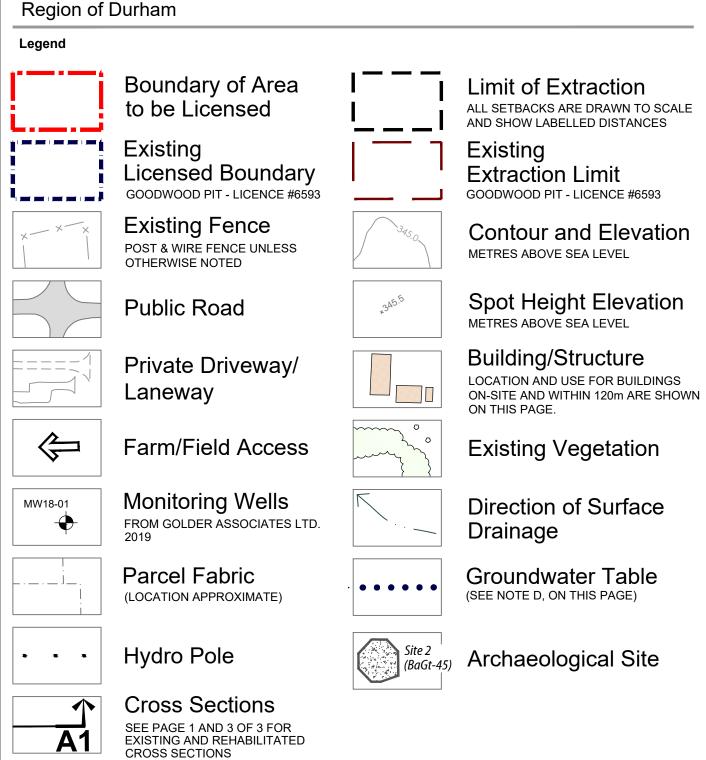
2. Post and wire fencing (unless noted otherwise) exists in the locations shown on

There are no existing aggregate operations or features on-site such as processing areas with stationary or portable equipment, stockpiles, recyclable materials, scrap, haul roads, fuel storage, berms or excavation faces.

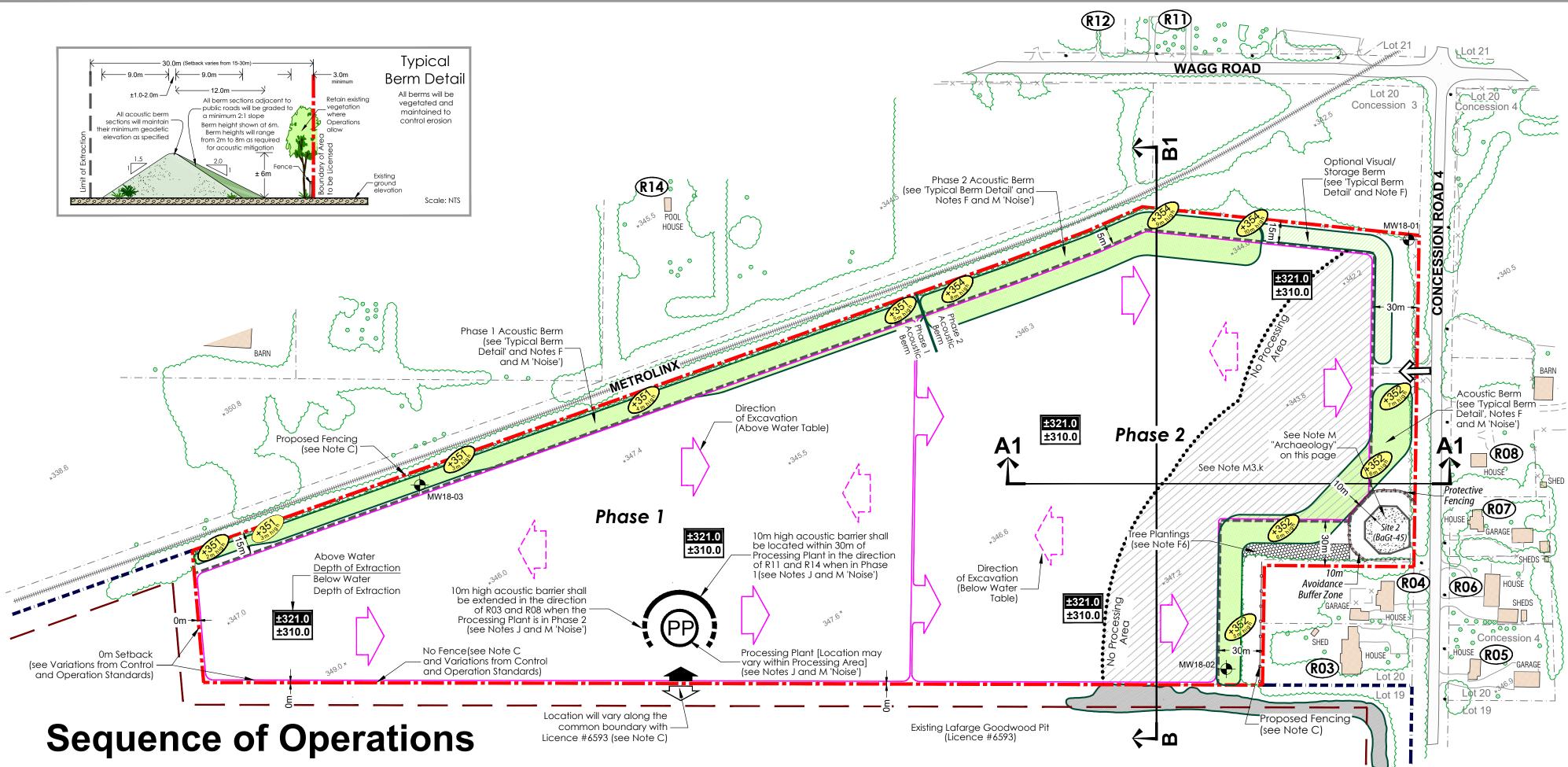
2. Cross section locations are identified on the plan view for each drawing.

. Hydrogeology: "Water Report Level 2, Lafarge Goodwood Pit Extension" July 2021 Natural Environment: "Proposed Goodwood Pit Extension Natural Environment Level 1 and 2 Technical Report", November 2020 (Source: Golder Associates Ltd.) Noise: "Noise Impact Study - Project: 18200 Goodwood Pit Extension, Township of Uxbridge, Ontario" August 20, 2021 (Source: Aercoustics Engineering Ltd.) Archaeology: Review and Entry into the Ontario Public Register of Archaeological Reports: Archaeological Assessment Report Entitled, "Stage 3 Archaeological Assessment: Goodwood Location 1 (BaGt-45), Lafarge Goodwood Extension Property, Part of Lot 20, Concession 3, Geographic

2021, MHSTCI Project Information Form Number P256-0670-2021, MHSTCI File



Site Plan Amendments Description Date No. PLANNING MHBC 200 - 540 BINGEMANS CENTRE DR. KITCHENER, ON, N2B 3X9 | P: 519.576.3650 F: 519.576.0121 | WWW.MHBCPLAN. NDMNRF Approval Stamp Stame Applicant's Signature LAFARGE Chris Galway Building better cities™ Senior Land Manager - East Central Ontario Lafarge Canada Inc. Project **Goodwood Pit Extension** Lafarge Canada Inc. 6509 Airport Road, Mississauga Ontario, L4V 1S7 Tel: (905) 738-7732 Fax: (905) 738-7092 NDMNRF Licence Reference No. Pre-approval review: For Submission to NDMNRF- January 2022 Plan Scale: See Plan Plot Scale 1:2.0 [1mm = 2.0 units] MODEL HORIZONTAL SCALE Drawn By File No. D.G.S. 9526HC Checked By C.P. File Name **EXISTING FEATURES PLAN** Drawing No. 1 OF 3 K:\9526HC-Lafarge-Goodwood Pit Extension-Uxbridge\A\Exfeplan 1of3 January2022.dwg



A. Genera Area Calculations

- Licence Area
- Limit of Extraction ±15.1 hectares (37.3 acres)

±17.9 hectares (44.2 acres)

- . The total tonnage to be excavated annually from this site, in conjunction with the existing Goodwood Pit Licence #6593, will not exceed 1,177,000 tonnes. No buildings or structures (including a scale and scale house) are propose
- 4. The water table elevation on site ranges generally between ±321.7 masl in the southwest portion of the site to ±320.6 masl in the northeast portion of the site or approximately 20 metres below the existing ground surface. The existing water table elevations are shown in each cross section on this drawing and drawing 3 of 3.
- Setbacks will be as shown and labelled on the Sequence of Operations Diagram on this page and page 1 of 3. There will be a 0m setback along the southern property boundary adjacent to Licence #6593 (see Section N Variations from Control and Operation Standards) Agricultural production may continue in areas not under extraction.
- Source Water Protection: The site is located in the Lake Simcoe and Couchiching/Black River Source Protection Area. The site is not mapped as being located in a Well Head Protection Area (WHPA), but is located in a Significant Groundwater Recharge Area and a Highly Vulnerable Aquifer Area. Mitigation measures are outlined in the Hydrogeology notes under Section M Report Recommendations.

B. Hours of Operation

- . Hours of Operation are as described in the Noise notes under Section M Report recommendations.
- C. Site Access and Fencing
- The existing field access on Concession Road 4 may be utilized for monitoring and agricultural access. The access shall be kept closed during hours of non-operation and shall be maintained throughout the life of the licence. Aggregate trucks shall not be permitted to access the site at this location. . The site shall be accessed through the common licence boundary with existing licence #6593 and no gate shall be
- required (see Section N Variations from Control and Operation Standards). The location shown on the plan view is approximate only and may occur anywhere on the common licence boundary during the life of the operation. Portions of the north and west (along Metrolinx right of way) and east (Concession 4 Road) licence boundary that are not currently fenced shall be fenced with post and wire fencing, at least 1.2 metres in height, prior to site preparation
- commencina. . Fencing shall not be required where the licence abuts existing licence #6593 (see Section N Variations from Control and Operation Standards) and in these locations, the boundary will be demarcated by $\pm 1.2m$ high marker posts that are visible from one to the other. If conditions in or around the licensed property change or if either licensed site is surrendered or sold, a 1.2m high fence will be installed. All fencing shall be maintained for the life of the extraction operation.

D. Drainage

Drainage of undisturbed areas will continue in the directions shown on drawing 1 of 3.

E. Site Preparation

- . Prior to site preparation, a Spills Contingency Plan shall be developed to address any potential spills from equipment on-site. 2. Timber resources will be salvaged for use as saw logs, fence posts and fuel wood where appropriate.
- Non-merchantable timber, stumps and brush will be used in for aquatic habitat enhancement or mulched for use in progressive rehabilitation in this licence or existing Licence #6593. Excess material not required for uses mentioned above will be burned (with applicable permits). 3. Topsoil and overburden shall be stripped and stored separately in accordance with the Sequence of Operations
- diagram. 4. Topsoil and overburden shall be placed in berms or used immediately for progressive rehabilitation in this licence or
- adjacent Licence #6525 (see Section N Variations from Control and Operation Standards). . Excess topsoil and overburden not required for immediate use in berms or rehabilitation may be temporarily stockpiled on the pit floor or in Licence #6593. Topsoil and overburden stockpiles shall be located within the limit of extraction and remain a minimum of 30 metres from the licence boundary (except where the licence boundary abuts existing Licence #6593 (see Section N Variations from Control and Operation Standards) and 90 metres from a
- property with a residential use. Temporary topsoil and overburden stockpiles which remain for more than one year shall have their slopes vegetated to control erosion. Seeding shall not be required if these stockpiles have vegetated naturally in the first year.

. Berms and Screening

- Berms shall be constructed to the elevation specified in the locations shown on the plan view prior to extraction/processing operations in each Phase. Locations and heights for all berms are provided on the Sequence of Operations diagram, this page. The heights/elevations shown are the minimum required. Overburden may be stored in separate berms throughout the extraction area.
- . Berm side slopes shall not exceed 1.5:1 on the interior and 2:1 on the exterior facing a public road. Berms that are not adjacent to a public road shall have side slopes not exceeding 1:5:1. See 'Typical Berm Detail' on this page. Berms shall not be located within three metres of the licence boundary except where adjacent to existing Licence
- #6593 (see Section N Variations from Control and Operation Standards). 4. All proposed berms will be constructed in accordance with the "Typical Berm Detail" on this page, and will be vegetated and maintained to control erosion using a low maintenance grass/legume seed mixture (e.g. MTO Seed
- Mix) composed of Creeping red Fescue, Perennial Ryegrass, Kentucky Bluegrass and White Clover. Temporary erosion control will be implemented as required. Berms shall be maintained throughout the operational life of the pit.
- . Trees will be planted on the southeast side of the berm adjacent to the residences on Concession Road 4 to enhance the existing treed area. These trees are to be established within one (1) year of licence issuance. Trees will be maintained and/or replaced if required, throughout the operation of the pit.
- Existing vegetation within the setbacks shall be maintained except where noise attenuation berms are required. G. Site Dewaterina
- . No existing or proposed surface water diversions or discharge has and/or will occur on the proposed extraction area. There will be no dewatering or pumping of water in the extraction area as ponds are included in the final rehabilitation plan.

H. Extraction Sequence

- are schematic and may vary. Phases do not represent any specific or equal time period. The direction of extraction will generally be in accordance with the Sequence of Operations diagram shown on this page. Rehabilitation will be progressive and proceed as limits of extraction (area and depth) are reached. Notwithstanding the operation and ehabilitation notes, demand for certain products or blending of materials may require minor deviations in the extraction and renabilitation sequence. Any major deviations from the operations sequence shown will require approval from MNRF.
- 2. Phase 1 Above Water Extraction a. Site preparation in Phase 1 to include: establishing fencing around the licensed boundary prior to extraction (subject to overrides); removal of vegetation where necessary; initial stripping of overburden/topsoil and construct berms as shown.
- b. Initial set up of portable processing plant on pit floor. c. Continue with stripping of overburden/topsoil as shown. Store any excess material in optional storage berms in areas within the limit of extraction or as shown on the Sequence of Operations.
- d. Begin Phase 1 above water extraction in an easterly direction and to the elevations as shown e. All extraction, processing and transportation equipment operating within this Phase shall comply with the restrictions identified in Noise notes under Section M Report Recommendations.
- f. Phase 1 above water extraction may be extracted to a depth of 321.0 masl. g. All Phase 2 berms to be in place prior to extraction in Phase 2.
- 3. Phase 2 Above Water Extraction a. Complete stripping of overburden/topsoil and construction of Phase 2 berms. Excess material shall be used for progressive rehabilitation in existing licence #6593 or stockpile in this licence for future rehabilitation.
- b. The portable processing plant may be relocated to Phase 2.
- c. Begin Phase 2 above water extraction in an easterly direction and to the elevations as shown. d. Commence progressive rehabilitation of side slope along north boundary of Phase 1.
- e. Extract Phase 2 in an easterly direction from Phase 1. f. All extraction, processing and transportation equipment operating within this Phase shall comply with the restrictions
- identified in Noise notes under Section M Report Recommendations. g. Phase 2 may be extracted to a depth of 321.0 masl.
- 4. Phase 2 Below Water Extraction
- a. Begin Phase 2 below water extraction in a westerly direction and to the elevations as shown. b. Continue progressive rehabilitation of side slope along north boundary of Phase 1. c. All extraction, processing and transportation equipment operating within this Phase shall comply with the restrictions
- identified in Noise notes "d", "f", "j" and "k" under Section M Report Recommendations. d. Phase 2 below water extraction may go to a maximum depth of 310.0 masl.
- e. Initiate progressive rehabilitation of Phase 2 along east and northeast portion of the phase, adjacent to Concession 4
- 5. Phase 1 Below Water Extraction
- a. Continue below water extraction in Phase 1 from Phase 2 in a westerly direction and to the elevations as shown. b. Continue progressive rehabilitation of side slope along west boundary of Phase 1 and initiate rehabilitation work along side slopes adjacent to the properties along Concession 4 Road, in the southeast portion of the phase.
- c. Phase 1 below water extraction may go to a maximum depth of 310.0 masl.
- 6. Phase 3 (not shown) a. Complete extraction activities
- b. Complete progressive rehabilitation and final rehabilitation of the site. c. Remove all machinery, scrap and internal haul roads from site.
- . Extraction Details 7. The maximum depth of extraction is as shown on the plan view. Extraction will occur in 3 lifts (2 lifts above the water table and 1 lift below the water table) through the two phases as shown on the Sequence of Operations Diagram on this page and in accordance with the Ministry of Labour requirements. Below water extraction will occur through the use of a dragline or excavator. The proposed pit floor is to be located at an elevation of 310 masl or ±33 m to ±39 m below the
- existing ground surface. The proposed pit is to be an extension of the existing Lafarge pit to the south and west. Extraction shall be permitted in two Phases simultaneously to allow for transition between Phases. 9. Aggregate stockpiles will be located on the pit floor (interim and final elevations) and will move throughout the life of the operations of the pit. Stockpiles will not be located within 30m of the Licensed boundary, except along the southern shared licence boundary with Licence #6593, as outlined in the Variations from Control and Operation Standards table
- on this page. 10. Internal haul road locations will vary as extraction progresses and will be located on the pit floor.
- Equipment and Processing . The equipment used on site for aggregate operations is listed in Note M Report Recommendations 'Noise', Table B and may include: One Portable Processing Plant, One Dragline or Excavator, Two Extraction Loaders, Two Shipment Loaders,
- Highway Trucks and Conveyors. 2. All processing equipment will be portable (crusher and screener) and subject to the noise controls and be located in close proximity to the extraction face in these Phases in order to maximize acoustical shielding. Within this area, the processing equipment shall remain a minimum of 30 metres from the licence boundary (except where the licence
- boundary abuts existing licence #6593 see Section N Variations from Control and Operation Standards). 3. No permanent processing areas are proposed on site. Portable processing equipment, crushing and screening may be used on site and will be located below grade on the pit floor adjacent to the active pit face. All processing equipment is
- subject to applicable permitting under MOE Environmental Compliance Approvals. See Note M 'Noise' and Sequence of Operations diagram for location of processing plant and limit of 'No Processing Areas' 4. Berms that encroach within the limit of extraction shall be removed, and the underlying aggregate may be extracted, as part of final extraction/rehabilitation of the site.
- K. Fuel Storage
- . No fuel or associated products will be stored on site. Mobile fuelling will occur in accordance with the Gasoline Handling Act, as amended, the Gasoline Handling Code and regulations, as amended, and Liquid Fuels Handling Code.

. This plan depicts a schematic operations sequence for this property. Extraction, stripping and rehabilitation areas shown

h. Prepare Phase 2 for extraction and ensure all requirements in Sections 'C' through 'G' of this drawing are met.

L. Scrap and Recycling There will be no on-site scrap storage. Temporary scrap storage will be located within the scrap storage area in the existing pit (Licence #6593) and will be removed on an on-going basis. No recycling is proposed.

M. Report Recommendation 1. <u>Hydrogeology: "Level1/2</u> Hydrology and Hydrogeology Study, Lafarge Goodwood Pit Extension" September, 2019 (Source: Golder Associates Ltd.) • Groundwater monitoring shall continue through Operations to confirm the conclusions of the impact assessment. The following tasks shall be carried out:

- -Monthly manual groundwater level monitoring at the three on-Site monitoring wells. -Annual groundwater quality sampling for general chemistry, metals, bacteria, petroleum
- hydrocarbons and volatile organic compounds. • The Site domestic well will be decommissioned per O.Reg. 903 by a licensed well contractor either prior to extraction or
- at such time that extraction encroaches on the well location. • A groundwater monitoring program and a complaint response program will be in place for residential wells within the potential draw down zone influence (i.e., 4639, 4709, 4840 and 4860 Concession Road 4) during extraction to address any complaints for surrounding residents concerning the supply of their water wells. In the event of unanticipated impacts to the water wells as a result of pit operations, mitigation and contingency measures will be implemented to restore the affected quantity or quality of the aroundwater supply
- A door-to-door water well survey shall be conducted to confirm baseline conditions at private wells within 500 m of the
- 2. Natural Environment: "Proposed Goodwood Pit Extension Natural Environment Level 1 and 2 Technical Report", November 2020 (Source: Golder Associates Ltd.) Standard best management practices to mitigate disturbance or damage to adjacent natural features include the followina: • Avoid removal of vegetation during the migratory bird nesting period (April 5 - August 26; ECCC 2019). Where
- vegetation removal cannot be avoided during this period, precede disturbance with a nesting survey by a qualified biologist and implement appropriate activity buffers around any active nests found during the survey until the young have fledged. • Remove the barn and trees on the site outside of the bat maternity roosting period (May 1 - July 31) to minimize adverse impacts on bats (not SAR) that may be roosting in these features.

3. <u>Noise:</u> "Noise Impact Study - Project: 18200 Goodwood Pit Extension, Township of Uxbridge, Ontario" January 5, 2021 (Source: Aercoustics Engineering Ltd.) The following noise controls are recommende

a. The Hours of operation are limited as described in Table A. There will be no operations on Public Holidays as per the Employment Standards Act. The pit will not operate on Sundays except as required by a specific contract. A response to emergencies is not limited by the hours of operations shown on the site plan. Table A: Operating Hours

Je se la seconda de la seconda d			
Time of Day	Day of Week	Operations	
06:00 to 07:00	Monday to Saturday	Shipping and Loading Operations Only	
07:00 to 19:00	Monday to Saturday	Full Operation - Extraction, Processing, Loading and Shipping	

b. The aggregate pit equipment shall satisfy the noise emissions levels listed in Table B. If desired, the two Quiet Extraction Loaders (maximum 70 dBA each) may be replaced by one regular Extraction Loader (maximum 74 dBA) wherever the Extraction Loaders are permitted. Table B: Reference Sound Pressure Levels of Aggregate Pit Equipment within Extension

1 Gi	Table B. Reference sound ressole Levels of Aggregate in Equipment within Extension				
	Equipment	Number Permitted	Reference Sound Pressure Level @ 30m (dBA)	1	
	Portable Processing Plant	1	85	1	
	Extraction Loader	2	70	l I	
	Shipment Loader	1	67*	l I	
	Dragline or Excavator	1	73	1	
	Conveyors		44**	1	
	Highway Trucks		66	I	

- * The shipment loaders were assumed to operate at a 50% duty cycle. * Reference sound level for conveyors is reported in dBA per metre at a distance of 30m.
- The sound emissions of all construction equipment involved in site preparation shall comply with the sound level limits specified in the MECP publication NPC-115 "Construction Equipment". New equipment technology or different configurations may allow proposed changes to any portion of the extraction and processing operations including additional equipment to operate on the site, equipment to be substituted, and/or different berm heights, while still meeting the applicable sound level limits. Changes may be permitted to the site operations and noise controls provided that the changes still meet the sound level limits, as confirmed through documentation prepared by a
- sional Engineer specializing in noise control. Prior to any modification, notification shall be given to the MNRF An acoustic barrier is required to be solid, with no gaps or openings, and shall satisfy a minimum area density of 20 kg/m2. It could take the form of a pit face, stockpile, acoustic fence, ISO containers, a combination of these, or any other construction satisfying the requirements of an acoustic barrier.
- Extraction in Phases 1 and 2 shall proceed generally in a northeasterly direction with the extraction loaders operating within 30 m of the working face. The working face shall have a minimum height of 7 metres. All equipment shall remain on the pit floor.
- During all processing operations, a 10 m high acoustic barrier shall be located within 30m of the Portable Processing Plant, between the plant and Receptors R11 and R14. In addition, a 9m high acoustic barrier shall be located within 30m of the Portable Processing Plant, between the plant and Receptors R03 and R08. These barriers can be satisfied by a working face or stockpiles. During below water extraction, only a single Extraction Loader shall operate near the Dragline or

Excavator, or at the working face.

Noise: (cont'd)

- rior to extraction in Phase 1, an acoustic barrier with a minimum top of barrier elevation of 351 m a.s.l. shall be installed, extending the full length along the north boundary of Phase 1, as shown on the site plan. This barrier shall remain in place for the project lifetime. <u>Phase 2</u>
- Prior to extraction in Phase 2, an acoustic barrier with a minimum top of barrier elevation of 354 m a.s. shall be installed, extending the length along the north boundary of Phase 2, meeting the acoustic barrier along the north boundary of Phase 1, as shown on the site plan. An acoustic barrier with a minimum top barrier elevation of 352 m a.s.l shall be installed extending the length along the east
- boundary of Phase 2, as shown on the site plan. A gap in the barrier extending 100 m in each direction from the northeast corner of the site is permitted. This barrier shall remain in place for the remainder of he project lifetime. No processing shall occur in the lands located within a 160 m radius of Receptors R03, R04, R07, and R08 as shown on the site plan.

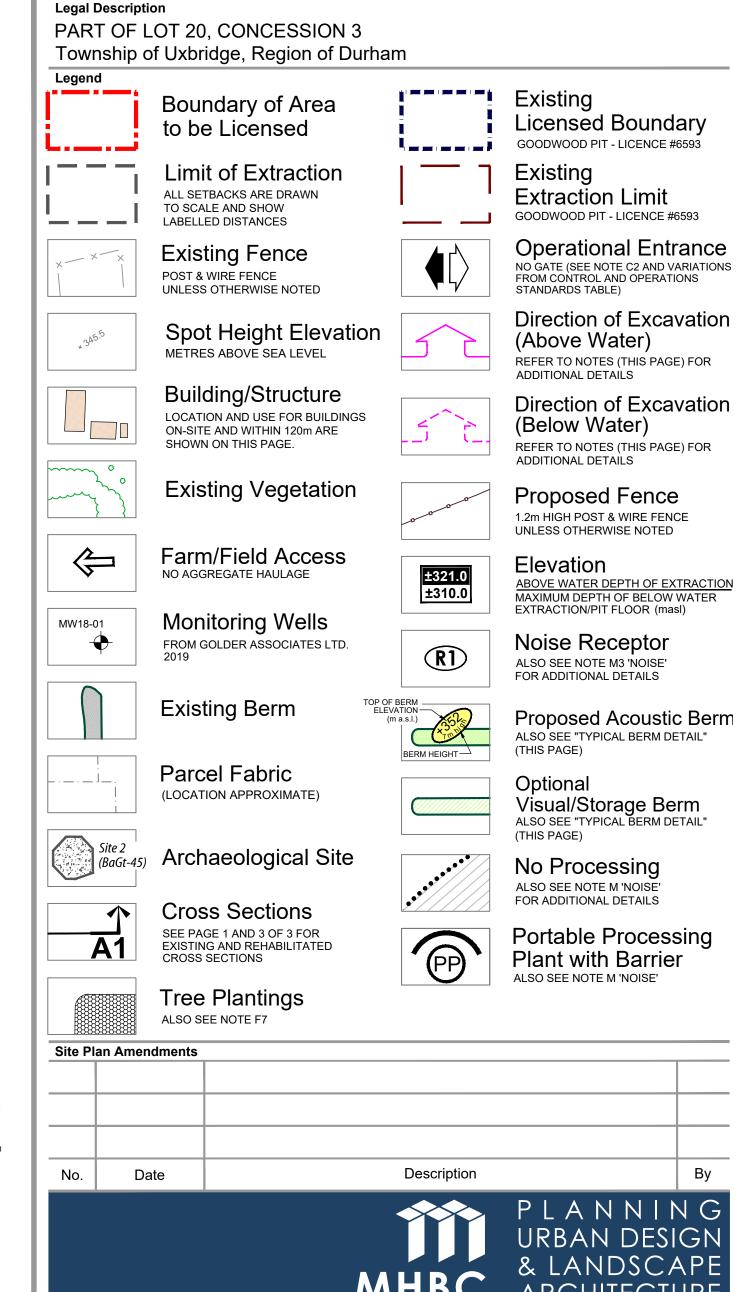
4. <u>Archaeology:</u> Review and Entry into the Ontario Public Register of Archaeological Reports: Archaeological Assessment Report Entitled, "Stage 3 Archaeological Assessment: Goodwood Location 1 (BaGt-45), Lafarge Goodwood Extension Property, Part of Lot 20, Concession 3, Geographic Township of Jxbridge, former Ontario County, now Regional Municipality of Durham, Ontario ", Dated Jul 13, 2021, Filed with MHSTCI Toronto Office on Jul 14, 2021, MHSTCI Project Information Form Number P256-0670-2021, MHSTCI File Number 0009350

- 1. Should previously undocumented archaeological resources be discovered that may be a new archaeological site and therefore subject to Section 48(1) of the Ontario Heritage Act, the proponent or person discovering the archaeological resources must cease alteration of the site immediately and engage a licensed consultant archaeologist to carry out archaeological fieldwork, in compliance with Section 48(1) of the Ontario Heritage Act. 2. The Cemeteries Act, R.S.O. 1990 c. C4 and the Funeral, Burial and Cremation Services Act,
- 2001, S.O. 2002, c.33 (when proclaimed in force) requires that any person discovering human remains must notify the police or coroner and the Deputy Registrar of the Cemeteries Regulation Unit at the Ministry of Consumer and Commercial Relations (416) 326-8392.

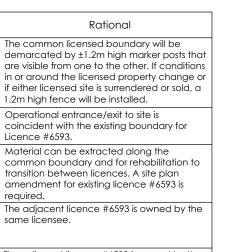
June 29, 2021 (Source: RWDI Air Inc.)

- The pit must operate in accordance with the operating standards pertaining to dust outlined in section 0.12 (2)Ontario Regulation 244/97, which include: a. The licensee or permittee shall apply water or another provincially approved dust suppressant to internal haul roads and processing areas, as necessary to mitigate dust, if the pit or quarry is located
- within 1,000 metres of a sensitive receptor. b. The licensee or permittee shall equip any processing equipment that creates dust with dust suppressing or collection devices if it is located within 300 metres of a sensitive receptor.
- c. The licensee or permittee shall obtain an environmental compliance approval under the Environmental Protection Act where required to carry out operations at the pit or quarry.
- d. The site will operate in accordance with Lafarge's Best Management Practices Plan for The Control of Fugitive Dust Emissions, which may be amended from time to time, considering actual impacts and operational considerations. The recommendations in the BMPP are based on the maximum daily production rates. At lower production rates, the control measures specified in the BMPP can be reduced accordingly, provided dust remains mitigated on site.

N. Variations fro	m Control and Operation Standards	
O.Reg 244/97 Section 0.13	Variation	
(3)(a)	Fencing will not be required along the southern and western boundaries of the area to be licensed where it is coincident with the existing boundary for Licence #6593.	The common lice demarcated by a are visible from or in or around the li if either licensed a 1.2m high fence v
(1)1& (1)2	No gate at the operational entrance/exit to site.	Operational entro coincident with the Licence #6593.
(1)10.i	Setback reduced to 0m from 15m along south and southwest limit of site.	Material can be e common bounde transition betwee amendment for e required.
(1)13.i	Stockpiling/processing may take place within 30m from the boundary of the south limit of the site but not within 90m of adjacent residential lands to the east.	The adjacent lice same licensee.
(1)16	Berms may be located within 3m of the boundary of adjacent Licence #6593.	The adjacent lice same licensee.
(1)17 & (1)18	Topsoil/overburden stripped in the operation of this site may be used in the rehabilitation of the adjacent Lafarge Licence (#6593)	This will allow strip preparation to be rehabilitation in th
(1)19.i	Below water side slopes may vary from a slope that is at least three horizontal metres for every vertical metre (3:1).	Below water slope angle of repose, v from ±2 - ±3 to 1.



5. Air Quality Assessment: "Lafarge Goodwood Pit Extension, Goodwood Ontario, Air Quality Assessment"



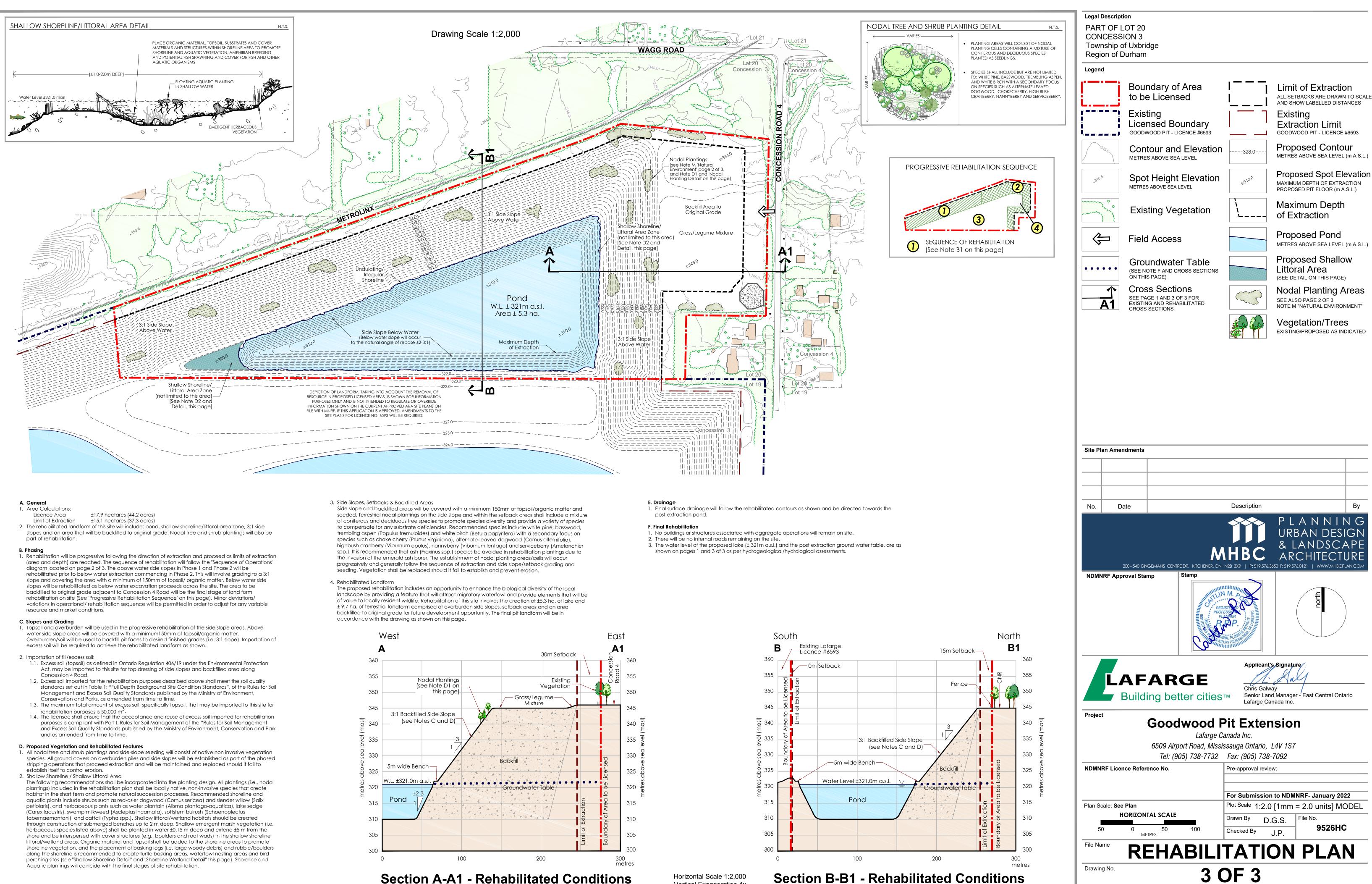
The adjacent licence #6593 is owned by the same licensee. This will allow stripped material from site preparation to be used for progressive rehabilitation in the existing licence. Below water slopes will stabilize at the natural anale of repose, which is estimated to range

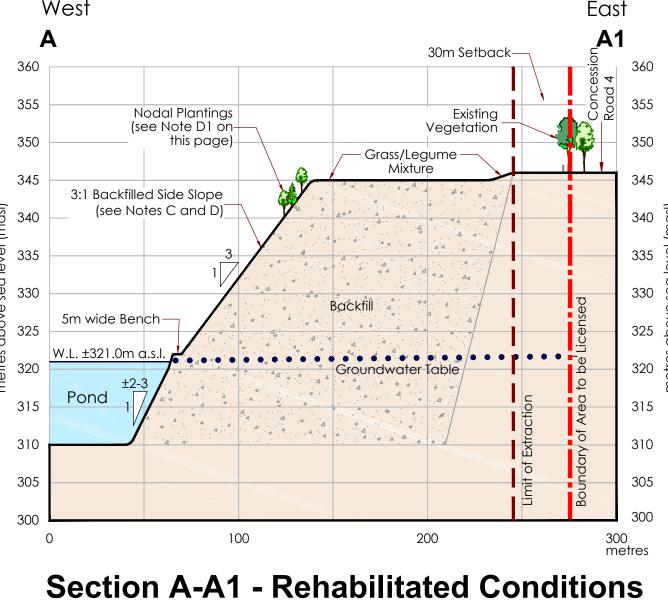
Drawing No.

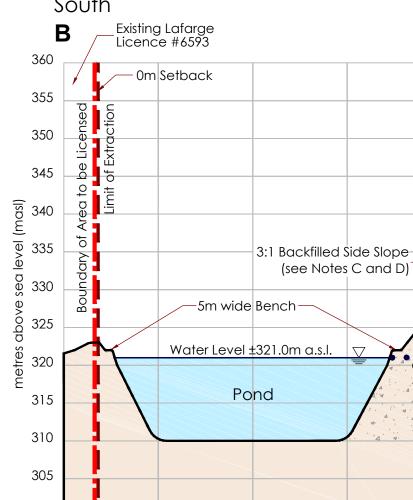


2 OF 3

K:\9526HC-Lafarge-Goodwood Pit Extension-Uxbridge\A\Operplan 2of3 January2022.dwg







K:\9526HC-Lafarge-Goodwood Pit Extension-Uxbridge\A\Rehaplan 3of3 January2022.dwg

Vertical Exaggeration 4x

APPENDIX B

Borehole Logs

Organic Soil Gradation **D**₆₀ $(D_{30})^2$ Organic USCS Group Type of Soil Cu Group Name Cc =or Inorganic D10 Group or Plasticity $D_{10} x D_{60}$ Content Symbol Gravels Poorly <4 ≤1 or ≥3 GP GRAVEL with is (mm) Graded ≤12% by mass of traction is (mm GRAVELS 4.75 fines Well Graded ≥4 GW GRAVEL 1 to 3 INORGANIC (Organic Content ≤30% by mass) (by mass COARSE-GRAINED SOILS (>50% by mass is larger than 0.075 arger than Gravels SILTY (>50% b coarse f Below A n/a GM with Line GRAVEL >12% Above A CLAYEY fines n/a GC Line GRAVEL (by mass) ≤30% Sands Poorly <6 ≤1 or ≥3 SP SAND s of is mm) with Graded ≤12% (≥50% by mass coarse fraction i than 4.75 fines Well Graded ≥6 sw SAND 1 to 3 SANDS (by mass) Sands Below A SILTY SAND n/a SM smaller t with l ine >12% CLAYEY Above A fines n/a SC SAND (by mass Line Field Indicators Organic Soil USCS Group Primary Laboratory Organic Toughness Type of Soil Dry Group Tests Shine Thread Content Symbol Name Dilatancy Inorganic (of 3 mm Strength Test Diameter thread) N/A (can' (Non-Plastic or PI and LL plot below A-Line on Plasticity Chart below) Rapid None None >6 mm roll 3 mm <5% М SILT thread) Liquid Limit (mm None to 3mm to CLAYEY SILT Slow Dull None to low <5% ML Low 6 mm Organic Content ≤30% by mass) mass is smaller than 0.075 <50 SILTS ORGANIC Slow to Dull to 5% to Low to 3mm to Low OL FINE-GRAINED SOILS 30% SILT verv slow medium sliaht 6 mm NORGANIC Slow to Low to 3mm to Low to Slight <5% ΜΗ CLAYEY SILT very slow medium 6 mm medium Liquid Limit ≥50 Dull to Medium to 5% to ORGANIC Medium 1 mm to None ОН to high sliaht 3 mm high 30% SILT Liquid Limit Slight Low to Low to (PI and LL plot above A-Line on Plasticity Chart below) SILTY CLAY None ~ 3 mm CL 0% <30 medium to shiny medium à CLAYS to (≥50% | Liquid Limit Medium Slight 1 mm to Medium 30% CI SILTY CLAY None 30 to 50 to high to shiny 3 mm (see Liquid Limit None High Shiny <1 mm High Note 2) СН CLAY ≥50 30% SILTY PEAT (Organic Content >30% by mass) Peat and mineral soil to HIGHLY ORGANIC SOILS SANDY PEAT mixtures 75% ΡT Predominantly peat, 75% may contain some PEAT mineral soil, fibrous or to 100% amorphous peat 40 Dual Symbol — A dual symbol is two symbols separated by Low Plasticity Medium Plasticity High Plasticity a hyphen, for example, GP-GM, SW-SC and CL-ML For non-cohesive soils, the dual symbols must be used when CLAY the soil has between 5% and 12% fines (i.e. to identify CH 30 transitional material between "clean" and "dirty" sand or gravel. SILTY CLAY CLAYEY SILT MH lasticity Index (PI) For cohesive soils, the dual symbol must be used when the С NIC SILT liquid limit and plasticity index values plot in the CL-ML area 20 of the plasticity chart (see Plasticity Chart at left). SILTY CLAY Borderline Symbol — A borderline symbol is two symbols CL 10 separated by a slash, for example, CL/CI, GM/SM, CL/ML. CLAYEY SILT ML ORGANIC SILT OL A borderline symbol should be used to indicate that the soil SILTY CLAY-CLAYEY SILT, CL-MI has been identified as having properties that are on the transition between similar materials. In addition, a borderline SILT ML (See Note 1) 0 10 20 25.5 30 40 70

The WSP Canada Inc. Soil Classification System is based on the Unified Soil Classification System (USCS)

Liquid Limit (LL) Note 1 - Fine grained materials with PI and LL that plot in this area are named (ML) SILT with slight plasticity. Fine-grained materials which are non-plastic (i.e. a PL cannot be measured) are named SILT. Note 2 - For soils with <5% organic content, include the descriptor "trace organics" for soils with

between 5% and 30% organic content include the prefix "organic" before the Primary name.

symbol may be used to indicate a range of similar soil types within a stratum.

ABBREVIATIONS AND TERMS USED ON RECORDS OF BOREHOLES AND TEST PITS

PARTICLE SIZES OF CONSTITUENTS

Soil Constituent	Particle Size Description	Millimetres	Inches (US Std. Sieve Size)
BOULDERS	Not Applicable	>300	>12
COBBLES	Not Applicable	75 to 300	3 to 12
GRAVEL	Coarse Fine	19 to 75 4.75 to 19	0.75 to 3 (4) to 0.75
SAND	Coarse Medium Fine	2.00 to 4.75 0.425 to 2.00 0.075 to 0.425	(10) to (4) (40) to (10) (200) to (40)
SILT/CLAY	Classified by plasticity	<0.075	< (200)

MODIFIERS FOR SECONDARY AND MINOR CONSTITUENTS

Percentage by Mass	Modifier
>35	Use 'and' to combine major constituents (<i>i.e.</i> , SAND and GRAVEL)
> 12 to 35	Primary soil name prefixed with "gravelly, sandy, SILTY, CLAYEY" as applicable
> 5 to 12	some
≤ 5	trace

PENETRATION RESISTANCE

Standard Penetration Resistance (SPT), N:

The number of blows by a 63.5 kg (140 lb) hammer dropped 760 mm (30 in.) r equired to drive a 50 mm (2 in.) split-spoon sampler for a distance of 300 mm (12 in.). Values reported are as recorded in the field and are uncorrected.

Cone Penetration Test (CPT)

An electronic cone penetrometer with a 60° conical tip and a project end area of 10 cm² pushed through ground at a penetration rate of 2 cm/s. Measurements of tip resistance (q_t), porewater pressure (u) and sleeve frictions are recorded electronically at 25 mm penetration intervals.

Dynamic Cone Penetration Resistance (DCPT); Nd:

The number of blows by a 63.5 kg (140 lb) hammer dropped 760 mm (30 in.) to drive uncased a 50 mm (2 in.) diameter, 60° cone attached to "A" size drill rods for a distance of 300 mm (12 in.).

- **PH:** Sampler advanced by hydraulic pressure
- PM: Sampler advanced by manual pressure
- WH: Sampler advanced by manual pressure WH:
- WR: Sampler advanced by weight of sampler and rod

Compactness ²			
Term SPT 'N' (blows/0.3m) ¹			
Very Loose	0 to 4		
Loose	4 to 10		
Compact	10 to 30		
Dense	30 to 50		
Very Dense	>50		

1. SPT 'N' in accordance with ASTM D1586, uncorrected for the effects of overburden pressure.

2. Definition of compactness terms are based on SPT 'N' ranges as provided in Terzaghi, Peck and Mesri (1996). Many factors affect the recorded SPT 'N' value, including hammer efficiency (which may be greater than 60% in automatic trip hammers), overburden pressure, groundwater conditions, and grainsize. As such, the recorded SPT 'N' value(s) should be considered only an approximate guide to the soil compactness. These factors need to be considered when evaluating the results, and the stated compactness terms should not be relied upon for design or construction.

Field Moisture Condition

Term	Description
Dry	Soil flows freely through fingers.
Moist	Soils are darker than in the dry condition and may feel cool.
Wet	As moist, but with free water forming on hands when handled.

SAMPLES	
AS	Auger sample
BS	Block sample
CS	Chunk sample
DD	Diamond Drilling
DO or DP	Seamless open ended, driven or pushed tube sampler – note size
DS	Denison type sample
GS	Grab Sample
MC	Modified California Samples
MS	Modified Shelby (for frozen soil)
RC	Rock core
SC	Soil core
SS	Split spoon sampler – note size
ST	Slotted tube
то	Thin-walled, open – note size (Shelby tube)
TP	Thin-walled, piston – note size (Shelby tube)
WS	Wash sample

SOIL TESTS

water content
plastic limit
liquid limit
consolidation (oedometer) test
chemical analysis (refer to text)
consolidated isotropically drained triaxial test ¹
consolidated isotropically undrained triaxial test with porewater pressure measurement ¹
relative density (specific gravity, Gs)
direct shear test
specific gravity
sieve analysis for particle size
combined sieve and hydrometer (H) analysis
Modified Proctor compaction test
Standard Proctor compaction test
organic content test
concentration of water-soluble sulphates
unconfined compression test
unconsolidated undrained triaxial test
field vane (LV-laboratory vane test)
unit weight

1. Tests anisotropically consolidated prior to shear are shown as CAD, CAU.

COHESIVE SOILS										
Consistency										
Term	Undrained Shear Strength (kPa)	SPT 'N' ^{1,2} (blows/0.3m)								
Very Soft	<12	0 to 2								
Soft	12 to 25	2 to 4								
Firm	25 to 50	4 to 8								
Stiff	50 to 100	8 to 15								
Very Stiff	100 to 200	15 to 30								
Hard	>200	>30								

 SPT 'N' in accordance with ASTM D1586, uncorrected for overburden pressure effects; approximate only.

 SPT N' values should be considered ONLY an approximate guide to consistency; for sensitive clays (e.g., Champlain Sea clays), the N-value approximation for consistency terms does NOT apply. Rely on direct measurement of undrained shear strength or other manual observations.

Water Content								
Term	Description							
w < PL	Material is estimated to be drier than the Plastic Limit.							
w ~ PL	Material is estimated to be close to the Plastic Limit.							
w > PL	Material is estimated to be wetter than the Plastic Limit.							

Unless otherwise stated, the symbols employed in the report are as follows:

I.	GENERAL	(a) w	Index Properties (continued) water content
π In x	3.1416 natural logarithm of x	w _l or LL w _P or PL	liquid limit plastic limit
log₁₀ g	x or log x, logarithm of x to base 10 acceleration due to gravity	l₀ or PI NP	plasticity index = (w _l – w _p) non-plastic
ť	time	Ws	shrinkage limit
		lL La	liquidity index = $(w - w_p) / I_p$
		lc e _{max}	consistency index = $(w_l - w) / I_p$ void ratio in loosest state
		emin	void ratio in densest state
		ID	density index = $(e_{max} - e) / (e_{max} - e_{min})$
11.	STRESS AND STRAIN		(formerly relative density)
γ	shear strain	(b)	Hydraulic Properties
Δ	change in, e.g. in stress: $\Delta \sigma$	h	hydraulic head or potential rate of flow
3	linear strain volumetric strain	q v	velocity of flow
ε _ν η	coefficient of viscosity	i	hydraulic gradient
υ	Poisson's ratio	k	hydraulic conductivity
σ	total stress		(coefficient of permeability)
σ'	effective stress ($\sigma' = \sigma - u$)	j	seepage force per unit volume
σ'_{vo}	initial effective overburden stress		
σ1, σ2, σ3	principal stress (major, intermediate,		Consolidation (one dimensional)
	minor)	(с) С _с	Consolidation (one-dimensional) compression index
σoct	mean stress or octahedral stress	Oc	(normally consolidated range)
0001	$= (\sigma_1 + \sigma_2 + \sigma_3)/3$	Cr	recompression index
τ	shear stress		(over-consolidated range)
u	porewater pressure	Cs	swelling index
E	modulus of deformation	Cα	secondary compression index
G K	shear modulus of deformation	mv	coefficient of volume change coefficient of consolidation (vertical
ĸ	bulk modulus of compressibility	Cv	direction)
		Ch	coefficient of consolidation (horizontal direction)
		Tv	time factor (vertical direction)
III.	SOIL PROPERTIES	U	degree of consolidation
(a)	Index Properties	σ′ρ OCR	pre-consolidation stress
(α) ρ(γ)	bulk density (bulk unit weight)*	OOK	over-consolidation ratio = $\sigma'_{p} / \sigma'_{vo}$
ρ(γ) ρ _d (γ _d)	dry density (dry unit weight)	(d)	Shear Strength
ρω(γω)	density (unit weight) of water	τp, τr	peak and residual shear strength
ρs(γs)	density (unit weight) of solid particles	φ' δ	effective angle of internal friction
γ'	unit weight of submerged soil	δ	angle of interface friction
-	$(\gamma' = \gamma - \gamma_w)$	μ	coefficient of friction = tan δ
D _R	relative density (specific gravity) of solid	C'	effective cohesion
0	particles ($D_R = \rho_s / \rho_w$) (formerly G_s) void ratio	Cu, Su	undrained shear strength ($\phi = 0$ analysis)
e n	porosity	p p'	mean total stress $(\sigma_1 + \sigma_3)/2$ mean effective stress $(\sigma'_1 + \sigma'_3)/2$
S	degree of saturation	p' q	$(\sigma_1 - \sigma_3)/2$ or $(\sigma'_1 - \sigma'_3)/2$
•		qu	compressive strength ($\sigma_1 - \sigma_3$)
		St	sensitivity
* Densi	ty symbol is ρ . Unit weight symbol is γ	Notes: 1	$\tau = c' + \sigma' \tan \phi'$
	$\gamma = \rho g$ (i.e. mass density multiplied by	2	shear strength = (compressive strength)/2
	eration due to gravity)		(

PROJECT: 1894152 LOCATION: N 4879757.60; E 645890.30

RECORD OF BOREHOLE: MW18-01

SHEET 1 OF 2 DATUM: Geodetic

BORING DATE: April 11, 2018

HAMMER TYPE: AUTOMATIC

SPT/DCPT HAMMER: MASS, 64kg; DROP, 760mm

	D H	SOIL PROFILE	L		SA	MPL	-	RESISTAN	PENETRA ICE, BLOV	TION VS/0.3m	2		k, cm/s	NDUCT	IVITY,	Ţ	NG	PIEZOMET	ER
METRES	BORING METHOD		STRATA PLOT		Ш		BLOWS/0.3m	20	40	60	80	10 ⁻¹			1	I0 ⁻⁴ ⊥	ADDITIONAL LAB. TESTING	OR	
Ξ	RING	DESCRIPTION	ATA	ELEV. DEPTH	NUMBER	TYPE	/SWC	SHEAR ST Cu, kPa	FRENGTH	nat V. - rem V. €	⊢ Q-● ₽ U-O				PERCE		ADDI: AB. T	INSTALLAT	
נ	B		STR	(m)	z		BLO	20	40	60	80	8				32	_ _		
0		GROUND SURFACE		343.74															
0		TOPSOIL, trace rootlets; brown; non-cohesive, moist		0.00 343.28	1	DO												Casing	4 . Y
		SAND and GRAVEL, coarse, trace silt;		0.46		DO												-	
		brown; non-cohesive, moist	VII	342.67		DO												#2 Silica Sand	
		CLAYEY SILT, some sand, trace gravel; light brown; non-cohesive, moist	111	1.22		DO													
		CLAYEY SILT, trace sand, trace gravel;	Ш	341.91	4														
2		light brown; non-cohesive, moist //		1.83	5	DO													
		cohesive, w~PL																	
				340.69	6	DO													
		SAND and GRAVEL, coarse, trace silt; light brown; non-cohesive, moist	• •	3.05	7	DO													
		light brown, non-conesive, moist	\$\$																
4			• •																
			, ,			DO													
			• •		°														
			, ,	000.05												1	1		
		SAND and GRAVEL, coarse, trace silt;		338.25 5.49		1										1	1		
6		dark brown to light brown; non-cohesive, moist	••																
		molot	Č.		9	DO										1	1		
			•••	336.73															
		SAND and GRAVEL, coarse; dark brown	•••	7.01															
		to light brown; non-cohesive, moist																	
8			8\$		10	DO										1	1		
-			•	335.21												1	1		
	<u>.</u>	SAND and GRAVEL, coarse; dark	ŚŻ	8.53		1										1	1		
	Son	brown; non-cohesive, moist	•																
	- I I		• •		11	DO													
10	4 mu		•	333.68												1	1		
	CSD 550 165 mm O.D., 114 mm I.D., Sonic	SAND and GRAVEL, coarse; brown to	• •	10.06		1													
	Ö E	light brown; non-cohesive, moist	Ç,		12	DO										1	1	Bentonite	
	165 m		•••		'											1	1		
				332.16															
12		SAND, fine, trace silt; light brown; non-cohesive, moist		11.58															
			2																
14																			
					13	DO										1	1		
																1	1		
																1	1		
16																			
10																			
																1	1		
																1	1		
				326.06															
		SAND, fine; light brown; non-cohesive,		17.68		1													
18		moist														1	1		
																1	1		
					14	DO													
																1	1		
20		— — — — — — — — — — — — — — — — — — —	<u>````</u> ````	†		† –	1-	+-		-+	-	† — − -	+		·	+			
						I										1			
	TLL O	CALE							GOL									OGGED: JC	
DEF	113								MEMBER O								_	000LD. 00	

	CT: 1894152 DN: N 4879757.60; E 645890.30	F	REC	OF			F BORI			MV	V18-	01					HEET 2 OF 2 ATUM: Geodetic	
SPT/DCF	PT HAMMER: MASS, 64kg; DROP, 760mm	ı					INO DATE. A	51111,20	10						HAMI	MER T	YPE: AUTOMATIC	
	SOIL PROFILE			SA	MPL	ES	DYNAMIC PE RESISTANCE	NETRATIC	N).3m	$\overline{\boldsymbol{\lambda}}$	HYDRA	AULIC C k, cm/s	ONDUCI	TIVITY,	T			
METRES BORING METHOD	DESCRIPTION	STRATA PLOT	ELEV. DEPTH (m)	NUMBER	TYPE	BLOWS/0.3m	20 J SHEAR STRE Cu, kPa 20	40 60 NGTH na re 40 60	atV. + emV.⊕	Q - • U - O	W. Wr	ATER C		PERCE		ADDITIONAL LAB. TESTING	PIEZOMETER OR STANDPIPE INSTALLATION	
20	CONTINUED FROM PREVIOUS PAGE	1.0																_
	SAND, fine; light brown; non-cohesive, moist		323.01	14	DO													
	SAND, fine, trace silt; brown; non-cohesive, moist		20.73	15	DO													
	SAND, fine; light brown; non-cohesive,		322.40 21.34															
22 24	SAND and GRAVEL, coarse; brown; non-cohesive, moist to wet		319.97 23.77	16	DO												 May 22, 2018	
82 82 550 550 165 mm 0.D., 114 mm 1.D., Sonic				17	DO												Bentonite	
30 32	SAND and GRAVEL, coarse; dark brown; non-cohesive, wet		313.87														#2 Silica Sand	1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1
34				18	DO												PVC Screen	
					DO												17.24	
	END OF BOREHOLE		306.86 36.88			-		+							+		<u> </u>	-
38	NOTES: 1. Groundwater measured in open borehole at a depth of 24.5 m below ground surface upon completion of drilling. 2. Groundwater measured in monitoring well at a depth of 23.6 m below ground surface on May 22, 2018.																	
40 DEPTH S 1 : 100								OLD IBER OF W	ER								DGGED: JC ECKED:	

PROJECT:	1894152
LOCATION:	N 4879444.70; E 645868.30

RECORD OF BOREHOLE: MW18-02

SHEET 1 OF 3 DATUM: Geodetic

BORING DATE: April 10, 2018

HAMMER TYPE: AUTOMATIC

SPT/DCPT	HAMMER	MASS	64ka		760mm
		IVIAGO,	UHRY,	DIXOF,	/00/11/11

a c c c c c c c c c c c c c c c c c c c		DESCRIPTION GROUND SURFACE TOPSOIL, trace rootlets; brown, moist SILTY SAND, trace gravel; brown; non-cohesive, moist SAND and GRAVEL, coarse; brown and grey; non-cohesive, moist SAND, fine, trace gravel; light brown; non-cohesive, moist	STRATA PLOT	ELEV. DEPTH (m) 346.43 0.00 345.52 0.91 345.52 0.91	\overline{Z}	0 0 TYPE	BLOWS/0.3m	20 J SHEAR ST Cu, kPa 20	40 RENGTH	60 nat V. rem V. 60	80 + Q-● ⊕ U-○ 80	w w	ATER C		PERCE	10 ⁻⁴ ⊥ ENT WI 32	ADDITIONAL LAB. TESTING	PIEZOMET OR STANDPIF INSTALLAT	PE
0 2 4 6 8		GROUND SURFACE TOPSOIL, trace rootlets; brown, moist SILTY SAND, trace gravel; brown; non-cohesive, moist SAND and GRAVEL, coarse; brown and grey; non-cohesive, moist SAND, fine, trace gravel; light brown;		DEPTH (m) 346.43 0.00 345.97 0.46 345.52 0.91	$\frac{1}{2}$	DO	BLOWS/(Cu, kPa		rem V.	⊕ U-O	w	p	—0 ^W		WI	ADDIT LAB. TI		
0 2 4 6 8		TOPSOIL, trace rootlets; brown, moist SILTY SAND, trace gravel; brown; non-cohesive, moist SAND and GRAVEL, coarse; brown and grey; non-cohesive, moist SAND, fine, trace gravel; light brown;		(m) 346.43 0.00 345.97 0.46 345.52 0.91	$\frac{1}{2}$	DO	BLO		40			W					₽¤]		
0 2 4 6 8		TOPSOIL, trace rootlets; brown, moist SILTY SAND, trace gravel; brown; non-cohesive, moist SAND and GRAVEL, coarse; brown and grey; non-cohesive, moist SAND, fine, trace gravel; light brown;		0.00 345.97 0.46 345.52 0.91) 1 5 2			20	40	60	80		<u> </u>		4 :	32			
2 4 6 8		TOPSOIL, trace rootlets; brown, moist SILTY SAND, trace gravel; brown; non-cohesive, moist SAND and GRAVEL, coarse; brown and grey; non-cohesive, moist SAND, fine, trace gravel; light brown;		0.00 345.97 0.46 345.52 0.91) 1 5 2							1	1				1		
4 6 8		SILTY SAND, trace gravel; brown; non-cohesive, moist SAND and GRAVEL, coarse; brown and grey; non-cohesive, moist SAND, fine, trace gravel; light brown;		345.97 0.46 345.52 0.91	2 2 3								<u> </u>						P
4 6 8		non-cohesive, moist SAND and GRAVEL, coarse; brown and grey; non-cohesive, moist SAND, fine, trace gravel; light brown;		345.52 0.91	2 2	DO												Casing	4
4 6 8		\grey; non-cohesive, moist SAND, fine, trace gravel; light brown;				•												#2 Silica Sand	
4 6 8		SAND, fine, trace gravel; light brown;	1. A.	1.22	- T	DO													12
4 6 8		non-cohesive, moist																	
6 8 Nonic			12.7		4	DO													
6 8 Nonic			1.22																
6 8 Nonic																			
6 8 Nonic					5	DO													
6 8 Nonic					ľ														
6 8 Nonic																			
8 Sonic																			
8 Sonic]	6	DO						1				1	1		
8 Sonic					ľ							1				1	1		
8 Sonic												1				1	1		
8 Sonic																			
Sonic												1				1	1		
Sonic					7	DO													
Sonic												1				1	1		
Sonic																			
Sonic		SILTY CLAY, trace sand; light brown;	WYX.	338.66								1				1	1		
. Sonic		cohesive, w~PL			8	DO						1				1	1		
, Sonic		SILTY CLAY, some sand; light brown;		337.90 8.53								1				1	1		
0.	Sonic	cohesive, w~PL		0.03	1							1				1	1		
9	<u> </u>											1				1	1		
550 mm	Ē			1	9	DO						1				1	1		
CSD 550	114											1				1	1		
	mm O.D., 114 mm l.D.,											1				1	1	Bentonite	
2 mm	2 mm	SAND, trace silt; light brown;		335.46												1			
161	165	non-cohesive, moist		334.85	10	DO													
		SAND and GRAVEL, coarse; brown to		11.58		1						1				1	1		
12		light brown and grey; non-cohesive, moist										1				1	1		
												1				1	1		
					11	DO						1				1	1		
			`									1				1	1		
14																			
			•••									1				1	1		
												1				1	1		
					40														
16					12	DO						1				1	1		
				329.36	5														
		SAND and GRAVEL, coarse; light brown		17.07	7	DO						1				1	1		
		and grey; non-cohesive, moist SAND, coarse; dark brown to light		328.75	5							1				1	1		
18		brown; non-cohesive, moist		17.00	1														
												1				1	1		
	1 1				14	DO													
												1				1	1		
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20													i.						
				1		+ -	-	$\vdash - + -$		-+-	_	+	<u> </u>	+		+	·		
.		CONTINUED NEXT PAGE		+			-	+-		-+-	-	+		+		+			
DEPTH		CONTINUED NEXT PAGE		4 — — —			-					<u>+</u>				+			

PROJECT:	1894152
LOCATION:	N 4879444.70; E 645868.30

RECORD OF BOREHOLE: MW18-02

SHEET 2 OF 3 DATUM: Geodetic

BORING DATE: April 10, 2018

HAMMER TYPE: AUTOMATIC

SPT/DCPT HAMMER: MASS	, 64kg; DROP,	760mm
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Ľ	IOH.	SOIL PROFILE	- I -	r	37			RESISTANCE	NETRA , BLOW	/S/0.3m	K.	'	JLIC CO k, cm/s		,		¥۴	PIEZOMETER
METRES	BORING METHOD	DESCRIPTION	STRATA PLOT	ELEV.	NUMBER	түре	BLOWS/0.3m	20 I SHEAR STRE Cu, kPa	40 I NGTH	nat V	80 - Q - ● → U - O		TER CO	DNTEN			ADDITIONAL LAB. TESTING	OR STANDPIPE INSTALLATION
5	BOF		STR/	(m)	Ĭ		BLC	20	40		80	Wp 8			24	- WI 32		
20		CONTINUED FROM PREVIOUS PAGE	-									Ĺ						
20		SAND, coarse; dark brown to light brown; non-cohesive, moist			14	DO												
		SAND and GRAVEL, coarse; dark brown		325.70 20.73														
		to light brown and grey; non-cohesive,	' ^ ^	20.73														
		moist																
22			22															
			- 2.2		15	DO												
			<u> </u>															
24		SAND, coarse; brown; non-cohesive,		322.66	-													
		moist																
																		\Box
					16	DO												Bentorlidey 22, 2018
26																		
				319.61 26.82														
		SAND and GRAVEL, coarse; dark brown to brown; non-cohesive, moist	'	20.62														
		<u>.</u> 0																
28		Sonic			4-													
			•		11	DO												
		114 mm																
				316.56														
30		GRAVEL, coarse, trace sand; brown; non-cohesive, moist		29.87		1												
		SAND, fine to coarse, trace gravel; dark	K	315.95 30.48														#2 Silica Sand
		brown to light brown; non-cohesive, moist to wet																
32																		
																		(1
				1	18	DO												
34																		PVC Screen
				310.46														
36		SAND to SAND and GRAVEL, coarse; brown to grey; non-cohesive, wet		35.97]												
		,,,,,,,,																
					19	DO												
38		END OF BOREHOLE	4.5	308.33 38.10	-	$\left \cdot \right $	+			_							_	
		NOTES:																
		1 Groundwater measured in open																
		borehole at a depth of 25.9 m below ground surface upon completion of																
40			_	+	┣-	+ -	-	+	-	+	-	<u> </u>				-+-	· -	
		CONTINUED NEXT PAGE																
DE	РТН	H SCALE					6	\land	י וכ									OGGED: JC

			T: 1894152	F	REC	OF	RD	0	F B(ORE	HOL	.E:	MV	V18-	02				Sł	HEET 3 OF 3
	LOC	CATIC	N: N 4879444.70; E 645868.30				E	BOR	ING DA	TE: Ap	ril 10, 20	18							D	ATUM: Geodetic
	SPT	/DCP	T HAMMER: MASS, 64kg; DROP, 760mm															HAM	MER T	YPE: AUTOMATIC
Щ		НОБ	SOIL PROFILE			SA	MPL	ES	DYNA RESIS	VIC PEN TANCE,	ETRATIC BLOWS/)N 0.3m	~ \	HYDR/	AULIC Co k, cm/s	ONDUCT	IVITY,	T	L VG	PIEZOMETER
H SCA	IKES	3 MET		PLOT	ELEV.	ËR	Ψ	/0.3m			IO 6 I NGTH n			10 ⁻¹⁰ 10 ⁻⁸ 10 ⁻⁶ 10 ⁻⁴ 1 1 U WATER CONTENT PERCENT					TESTI	OR STANDPIPE
DEPTH SCALE	B	BORING METHOD	DESCRIPTION	STRATA PLOT	DEPTH (m)	NUMBER	түре	BLOWS/0.3m	Cu, kP	a	NGTH N	atv. + emV.⊕	U- O						ADDITIONAL LAB. TESTING	INSTALLATION
	-	В	CONTINUED FROM PREVIOUS PAGE	S		┝		8	2	0 4	10 6	8 0	0		<u>8 1</u>	62	4 3	2		
Ē	40 -		2. Groundwater measured in monitoring			┢														
E			well at a depth of 25.0 m below ground surface on May 22, 2018.																	-
-			, ,																	-
-	42																			-
-																				
Ē																				
-																				-
-	44																			-
-																				-
-																				
-	46																			
-	40																			-
-																				
-																				
	48																			-
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≥ -																				-
	50																			-
52.GPJ																				-
18941																				-
	52																			-
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	54																			-
																				-
4 - 0	56																			
- - -	50																			
																				-
	58																			-
SLA																				-
																				-
	60																			-
1A-BHS 001			CALE								ם ונ	ED		•					· /	DGGED: JC
i I A-B	DEF 1:1								\Diamond	MEME	DLD BER OF W	5P								ECKED:

TYSELLam OH 0 GROL 0 GROL 2 GROL 4 TOPE I 4 SANL 6 SANL 8 SANL 10 SANL 11 SANL 12 SANL 14 SANL 16 SANL					E	BOR	ING DATE: April 12, 2018			DATUM: Geodetic
0 GROL COPE SANUE SANUE Brown SANUE SANUE Brown SANUE Moist 2 A 4 SANUE SANUE Moist 4 SANUE SANUE Moist 6 SANUE SANUE Moist 10 000 000 000 000 000 000 000 000 000 00	AMMER: MASS, 64kg; DROP, 760mm								HAMMER	TYPE: AUTOMATIC
0 GROL TOPS SANU SANU Brown SANU Brown 2 -	SOIL PROFILE			SA	MPL	ES	DYNAMIC PENETRATION RESISTANCE, BLOWS/0.3m	HYDRAULIC CONDUCTIVITY, k, cm/s	T	
0 TOPS 2 January Samuella Samue	DESCRIPTION	STRATA PLOT	ELEV. DEPTH (m)	NUMBER	ТҮРЕ	BLOWS/0.3m	SHEAR STRENGTH nat V. + Cu, kPa rem V. ⊕	Q - O WATER CONTENT PERC		PIEZOMETER OR STANDPIPE INSTALLATION
2 Image: Constraint of the second s	OUND SURFACE		346.63							
2 Image: Same set of the	PSOIL, trace rootlets, trace organics; k brown; moist/		0.00	1	DO					Casing
2 Image: Constraint of the second s	ND, fine, trace silt; brown to light wn; non-cohesive, moist		345.41	2	DO					#2 Silica Sand
6 8 SANE non-c 8 9000 °° SANE non-c 10 000 °° SANE SANE Cobbi Moist SANE to light 12 14 100 °° SANE SANE to light 14 I I I 16 I I SANE 16 I SANE	ND, fine; light brown; non-cohesive, ist		1.22	3	DO					
6 8 SANE non-c 8 9000 ° ° ° ° ° ° ° ° ° ° ° ° ° ° ° ° ° °				4	DO					
6 Image: second se										
6 I I non-c 8 999 999 999 999 10 999 999 999 999 12 13 SANE SANE SANE 14 I I I I I I 16 I I I SANE SANE	ND, fine, trace silt; light brown;		<u>341.14</u> 5.49		DO					
10 20 20 20 20 20 20 20 20 20 20 20 20 20	-cohesive, moist		0.40							
12 SANE cobbl moist SANE to ligh 14 SANE				6	DO					
14 SANE	ND and GRAVEL, coarse, trace bles; dark brown; non-cohesive,		<u>335.05</u> 11.58 11.89		DO					Bentonite
SANE	ist // ND and GRAVEL, coarse; dark brown ght brown; non-cohesive, moist				DO					
	ND and GRAVEL, coarse; dark brown ight brown; non-cohesive, moist		<u>328.95</u> 17.68							
20				9	DO					

PROJECT: 1894152 LOCATION: N 4879384.90; E 645314.80

RECORD OF BOREHOLE: MW18-03

SHEET 2 OF 2 DATUM: Geodetic

BORING DATE: April 12, 2018

HAMMER TYPE: AUTOMATIC

SPT/DCPT HAMMER: MASS, 64kg; DROP, 760mm

	ΡĮ	SOIL PROFILE			SA	MPL	ES	DYNAMIC PENE RESISTANCE, E	BLOWS	0.3m	ζ.	HYDRAULIC k, cn	l/s		T	₽ بـ	PIEZOMETER
METRES	BORING METHOD	DESCRIPTION	STRATA PLOT	ELEV. DEPTH	NUMBER	TYPE	BLOWS/0.3m	20 4 I I SHEAR STREN Cu, kPa	GTH r	iat V. +	B0 - Q - ● - U - O		10 ⁻⁸ 1 CONTEN	r Perce		ADDITIONAL LAB. TESTING	PIEZOMETER OR STANDPIPE INSTALLATION
-+	ă		ST	(m)	<u> </u>		ВГ	20 4	06	0	80	8			32	<u> </u>	
20		CONTINUED FROM PREVIOUS PAGE SAND and GRAVEL, coarse; dark brown	(x) (x)														
		to light brown; non-cohesive, moist	•	325.90		DO											
22		SAND and GRAVEL, coarse; dark brown; non-cohesive, moist		20.73		DO											
22		SAND and GRAVEL, coarse; light brown; non-cohesive, moist		324.38		DO											
24		SAND, coarse, some gravel, trace silt; light brown; non-cohesive, moist		322.86 23.77		-											
26					12	DO											Bentonite May 22, 2018
	, Sonic	SAND, coarse, trace gravel; brown to light brown and grey; non-cohesive, moist		319.81 26.82		-											
28 28 28 250	165 mm O.D., 114 mm I.D.,			317.67		DO											
30	165 mm O.	SAND, fine, trace gravel, trace silt; grey; non-cohesive, moist No Sample Collected		28.96 316.76 29.87	14	DO											
																	#2 Silica Sand
32					15	DO											
34																	PVC Screen
36		No Sample Collected		310.66 35.97	16	DO											
		END OF BOREHOLE		309.75 36.88													
38		NOTES: 1. Groundwater measured in open borehole at a depth of 26.1 m below ground surface upon completion of drilling.															
40		2. Groundwater measured in monitoring well at a depth of 25.3 m below ground surface on May 22, 2018.															

Min of th	istry ne		A/AT		tario Water Res NELL		CO	PN
Ontario Env	ironment	IN SPACES PROVIDED		190961		2		1 103
COUNTY OR DISTRICT	2. CHECK 🛛 CO	TOWNSHIP, BOROUGH CITY, TO	· · ·		CON BLOCK TRACT S	14 15		22 23 74 LOT 25-27
Durahom	×	Urbridge			3	DATE COM	PLETED	20
		81 G	4900 Loodwood	, Ontario	LOC 1AO	DAY _2	0 мо 12	\$8
1 2	M 10 12	¹ 7,9,3	83 [345	RC BASIN CODE			
		LOG OF OVERBURDEN A	ND BEDROC	K MATERIALS	(SEE INSTRUCTIONS)			
GENERAL COLOUR	MOST COMMON MATERIAL	OTHER MATER	IALS		GENERAL DESCRIPTION	1	DEPTH FROM	· FEET TO
Brown	Clay	Sand		Layers			0	65
Brown	Gravel			Loose	& Dry		65	85
Yellow	Clay	Strips of Sand		Packed			85	96
Brown	Sand			Clean	& Fine		96	106
					u			
· · · · · · · · · · · · · · · · · · ·								
31	<u> </u> 							1,1 1
							11111	
41 WAT	TER RECORD	51 CASING & OP			SIZE (S) OF OPENING (SLOT NO)	31-33 DIAME	TER 34-38 L	ENGTH 39-40
WATER FOUND AT - FEET	KIND OF WATER	INSIDE DIAM MATERIAL TI INCHES	WALL DE HICKNESS INCHES FROM	TO	MATERIAL AND TYPE		DEPTH TO TOP	1.0. FEET
96] FRESH 3 □ SULPHUR] SALTY 4 □ MINERALS 6 □ GAS	61 10-11 2 STEEL 12 C Galvanized 3 Concrete	188 5	96.	° S.S.			96
] FRESH 3 □SULPHUR ¹⁹ 3 Salty 4 □ Minerals 3 Salty 6 □ Gas	3 □ CONCRETE 4 □ OPEN HOLE 5 □ PLASTIC 17-18 19	2	20-23	61 PLUGG	ING & SEAL		
] FRESH 3 □SULPHUR 24 4 □ MINERALS 3 SALTY 6 □ GAS	1 🗆 STEEL 2 🗆 GALVANIZED 3 🗆 CONCRETE		20-23	FROM TO 10-13 14-17	MATERIAL AND		KER. ETC.)
1	FRESH 3 SULPHUR 29	4 0 OPEN HOLE 5 0 PLASTIC 24-25 26 1 0 STEEL		27-30	18-21 22-25			
30-33 1	FRESH 3 □ SULPHUR 34 4 □ MINERALS	C 2 GALVANIZED 3 CONCRETE 4 OPEN HOLE			26-29 30-33	E0 .		
	SALTY 6 GAS	5 DPLASTIC						
171	2 BAILER	12 GPN HOURS_	17-10 30 MINS		LOCATION			
	PUMPING	LEVELS DURING	MPING COVERY	LOT LINE			FROM ROAD A	ND .
	20	1-28 E9-31 32-34	60 MINUTES 35-37 86 FEET	Lot #	<i>∓</i> , <i>L</i> '		LT-	SN .
S 75 FEET	86 FEET 86 F 38-41 PUMP INTAK				•	لمس	H	
S			EST 41			- the		
RECOMMENDED PU		FEET 1 CLEAR	EST 41				ConT	¥↑
RECOMMENDED PUR	MP TYPE RECOMMEND PUMP	FEET 1 CLEAR	EST 41			2. 111	Cont	¥ ↑
50-53	MP TYPE RECOMMEND PUMP DEEP SETTING	FEET 1 CLEAR FED 43-45 RECOMMENDED PUMPING P5 FEET RATE	EST 41			on 111	Cont	¥↑ N.
FINAL STATUS	MP TYPE RECOMMEND PUMP DEEP SETTING	FEET 1 CLEAR PED 43-45 RECOMMENDED PUMPING P5 FEET RATE 6 ABANDONED, INSUFFIC ELL 6 ABANDONED POOR QUA 7 UNFINISHED	EST 41			on 111	ConJ	¥↑ N.
FINAL STATUS OF WELL	MP TYPE RECOMMEND PUMP DEEP SETTING	FEET 1 CLEAR PED 43-45 RECOMMENDED PUMPING P5 FEET RATE 6 ABANDONED, INSUFFIC ELL 6 ABANDONED POOR QUA 7 UNFINISHED	EST 41			on 111	ConJ	¥↑ N
FINAL STATUS OF WELL WATER	AP TYPE RECOMMEND PUMP DEEP SETTING	FEET 1 CLEAR 1 CLEAR	15 GPM			n 111 on 111	ConT	¥↑ N
FINAL STATUS OF WELL	AP TYPE PLEP RECOMMEND PUMP SETTING	FEET 1 CLEAR 1 PL	15 GPM 15 GPM 15 MPPLY ALITY NING		1.R. 250 % 420 J well	n 111 on 111	ConT	¥↑ N
FINAL STATUS OF WELL WATER USE	AP TYPE RECOMMEND PUMP DEEP SETTING 4 DEEP SETTING 4 TEST HOLE COMMEND S S S D D D D D D D D D D D D D D D D	FEET FEET CLEAR FEET FEET FEET FEET FEET FEET FEET FEE	15 GPM	C. 1 4444 267 #	1.R. 250 % 420 J well	on-	ConT	¥↑ N
FINAL STATUS OF WELL WATER	AP TYPE RECOMMEND PUMP DEEP SETTING	FEET 1 CLEAR FED 43-45 RECOMMENDED 95 FEET RATE 0 ABANDONED. INSUFFIC eLL ABANDONED. POOR QUA 7 UNFINISHED 9 DEWATERING 5 COMMERCIAL 6 MUNICIPAL 7 PUBLIC SUPPLY 8 COOLING OR AIR CONDITION 9 NOT USI 0 BORING NTIONALI 7	15 GPM	C. 1 4444 267 #		on-		Ĩ
FINAL STATUS OF WELL WATER USE METHOD OF CONSTRUCTIO	AP TYPE RECOMMEND PUMP DEEP BETTING A DEEP BETTING A DEEP BETTING A DEEP BETING A DEEP BETTING A DEEP BETTIN	FEET 1 CLEAR PED 43-45 RECOMMENDED 95 FEET RATE 0 ABANDONED, INSUFFIC ELL ABANDONED, INSUFFIC 0 DEWATERING 9 DEWATERING 9 DEWATERING 5 COMMERCIAL 6 MUNICIPAL 7 DUBLIC SUPPLY 8 COOLING OR AIR CONDITION 1 DBORING NTIONALI 1 8 DRINING 1 DIAMOND	15 GPM 15 GPM	C. T Lot # DIO OC DRILLERS REMARKS	1.R. 250 m 250 m 420 ↓ well well 05 064 00	0n- 800 ^m -7	314	79
FINAL STATUS OF WELL WATER USE METHOD OF CONSTRUCTIO	AP TYPE RECOMMEND PUMP DEEP BETTING A DEEP BETTING A DEEP BETTING A DEE BETTING A DEEP BETTIN	FEET 1 CLEAR 120 CLEAR 120 RECOMMENDED 95 FEET 121 RATE	15 GPH 15 GPH	C. T LoT # LoT # DIO OC DRILLERS REMARKS > SOURCE	1.R. 250 m 250 m 420 ↓ well well 05 064 00	0n- 800 ^m -7	314	79
FINAL STATUS OF WELL WATER USE METHOD OF CONSTRUCTIO	AP TYPE RECOMMEND PUMP DEEP BETTING A DEEP BETTING A DEEP BETTING A DEE BETTING A DEEP BETTIN	FEET CLEAR FEET CLEAR PED 43-45 PED 43-45 PED MARING 95 FEET RATE ABANDONED INSUFFIC ABANDONED POOR QUA 7 UNFINISHED 9 DEWATERING 9	ALITY ALITY NING ED NUMBER ALITACTOR'S NUMBER	C. F Lot + Lot + Lot + DIO OC DRILLERS REMARKS DATA SOURCE DATA DATA DATA DATA DATA DATA DATA DIO OC	J.R. 250 % 250 % 420 ↓ 0€ 3 well 05 064 00 31 CONTRACTON 474 3	00	314	79
FINAL STATUS OF WELL WATER USE METHOD OF CONSTRUCTION NAME OF WELL WATER USE	AP TYPE DEEP DEEP RECOMMEND PUMP SETTING A DEEP RECOMMEND PUMP SETTING A DEEP SETTING A DEEP SETTING A DEEP SETTING A DEEP SETTING A DEEP SETTING A DEEP SETTING A DEEP SETTING A DEEP SETTING A DEEP SETTING A DEEP SETTING A DEEP SETTING A DEEP SETTING A DEEP SETTING A	FEET 12 CLEAR 12 CLEAR 12 CLEAR 14 State RECOMMENDED 95 FEET RATE 1 ABANDONED. INSUFFIC ABANDONED POOR QU/ 1 UNFINISHED 9 DEWATERING 1 UNFINISHED 9 DEWATERING 2 COMMERCIAL MUNICIPAL 1 DUBLIC SUPPLY COOLING OR AIR CONDITION 2 DUBLIC SUPPLY 10 DIAMOND 2 DUBLIC SUPPLY 0 DIAMOND 3 DUBLIC SUPPLY 0 DIAMOND 4 DUBLIC SUPPLY 0 DIAMOND 4 DUBLIC SUPPLY 0 DIAMOND 5 DUBLIC SUPPLY 0 DIAMOND 4 DUBLIC SUPLY 0 DIAMOND 5 DUBLIC SUPLY 0 DIAMOND 5 DUBLIC SUPLY 0 DIAMOND 5 DUBLIC SUBLY 0 DIAMOND 5 DU	15 GPN 15 GPN ISB GPN	DIO OC DRILLERS REMARKS DATA SOURCE DATE OF INSPECTION BINDERS	J.R. 250 % 250 % 420 ↓ 0€ 3 well 05 064 00 31 CONTRACTON 474 3	00	314	79
FINAL STATUS OF WELL WATER USE METHOD OF CONSTRUCTION NAME OF WELL ST METHOD OF CONSTRUCTION STRUCTION STRUCTION CONSTRUCTION STRUCTION	MP TYPE RECOMMEND PUMP DEEP SETTING	FEET 12 CLEAR 12 CLEAR 12 CLEAR 14 State RECOMMENDED 95 FEET RATE 1 ABANDONED. INSUFFIC ABANDONED POOR QU/ 1 UNFINISHED 9 DEWATERING 1 UNFINISHED 9 DEWATERING 2 COMMERCIAL MUNICIPAL 1 DUBLIC SUPPLY COOLING OR AIR CONDITION 2 DUBLIC SUPPLY 10 DIAMOND 2 DUBLIC SUPPLY 0 DIAMOND 3 DUBLIC SUPPLY 0 DIAMOND 4 DUBLIC SUPPLY 0 DIAMOND 4 DUBLIC SUPPLY 0 DIAMOND 5 DUBLIC SUPPLY 0 DIAMOND 4 DUBLIC SUPLY 0 DIAMOND 5 DUBLIC SUPLY 0 DIAMOND 5 DUBLIC SUPLY 0 DIAMOND 5 DUBLIC SUBLY 0 DIAMOND 5 DU	15 GPN 15 GPN ISB GPN	DIO OC DRILLERS REMARKS DATA SOURCE DATE OF INSPECTION BINDERS	$\frac{1.8}{250} = \frac{1}{2}$ $\frac{1}{20} = \frac{1}{20}$	00	314	79
FINAL STATUS OF WELL WATER USE METHOD OF CONSTRUCTION NAME OF WELL NAME OF WELL NAME OF WELL SIGNATURE OF AD	AP TYPE DEEP DEEP RECOMMEND PUMP SETTING DEEP SETTING A DEEP SETTING A DEEP SETTING SETI	FEET 1 CLEAR 1 CLEAR	ALITY ALITY ALITY NING ED OTHER DNTRACTOR'S NUMBER ALITY NING ED OTHER DNTRACTOR'S NUMBER ALITY ALITY NING ED DNTRACTOR'S NUMBER ALITY ALITY ALITY NING ED DNTRACTOR'S ALITY	DIO OT DATE OF INSPECTION	$\frac{1.8}{250} = \frac{1}{2}$ $\frac{1}{20} = \frac{1}{20}$	0 0 ^m -7	314	79

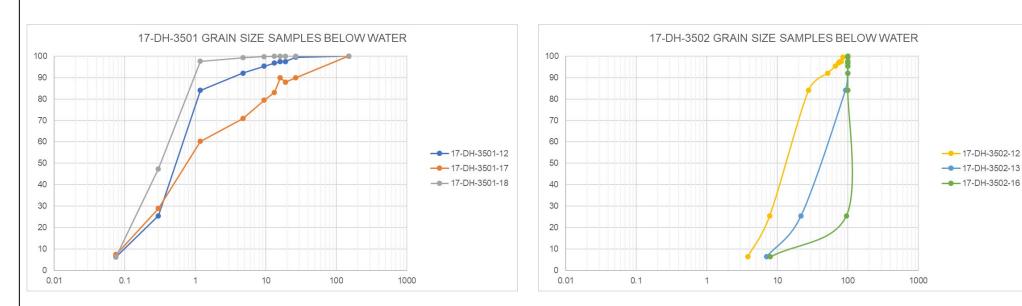
APPENDIX C

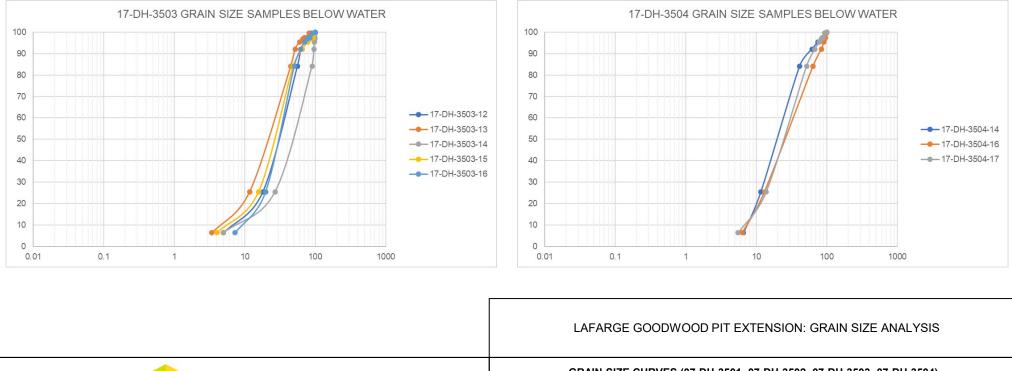
Grain Size Data

21453907 LAFARGE GOODWOOD PIT EXTENSION: GRAIN SIZE ANALYSIS SUMMARY
BELOW WATER SAMPLES

Sample	Depth To (mbgs)	d ₁₀ (mm)	K (m/s)
17-DH-3501-12	19.51	0.1	1.E-04
17-DH-3501-17	32.92	0.09	8.E-05
17-DH-3501-18	35.05	0.085	7.E-05
17-DH-3502-12	21.95	0.38	1.E-03
17-DH-3502-13	23.17	0.13	2.E-04
17-DH-3502-16	28.65	0.079	6.E-05
17-DH-3502-19	33.22	0.12	1.E-04
17-DH-3503-12	27.13	0.15	2.E-04
17-DH-3503-13	28.04	0.28	8.E-04
17-DH-3503-14	30.18	0.11	1.E-04
17-DH-3503-15	31.39	0.19	4.E-04
17-DH-3503-16	33.22	0.10	1.E-04
17-DH-3504-14	22.25	0.26	7.E-04
17-DH-3504-16	24.99	0.25	6.E-04
17-DH-3504-17	27.13	0.22	5.E-04
17-DH-3505-13	27.13	0.09	8.E-05
17-DH-3505-14	28.35	0.09	8.E-05
17-DH-3505-15	30.18	0.16	3.E-04
17-DH-3505-16	31.7	0.15	2.E-04
17-DH-3507-15	28.35	0.11	1.E-04
17-DH-3507-16	30.18	0.16	3.E-04
17-DH-3507-17	33.22	0.12	1.E-04
17-DH-3507-20	-	0.09	8.E-05
17-DH-3508-12	25.91	0.19	4.E-04
17-DH-3508-13	26.82	0.36	1.E-03

Geomean:	2E-04
Max:	1E-03
Min:	6E-05





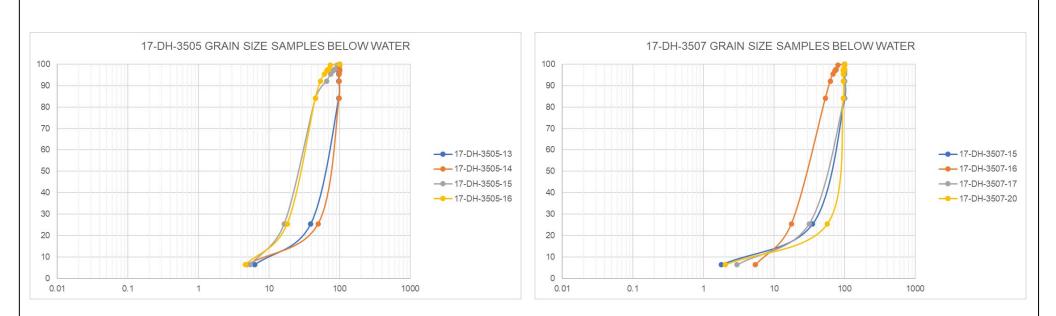
SEPTEMBER 2019

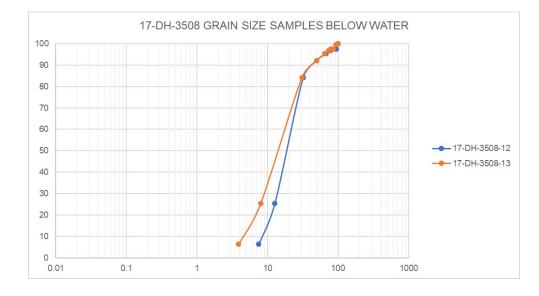
Golder Associates Ltd. BARRIE, ONTARIO, CANADA

GOLDER

GRAIN SIZE CURVES (07-DH-3501, 07-DH-3502, 07-DH-3503, 07-DH-3504)

PROJECT: 1894152





LAFARGE GOODWOOD PIT EXTENSION: GRAIN SIZE ANALYSIS



Golder Associates Ltd.

GRAIN SIZE CURVES (07-DH-3505, 07-DH-3507, 07-DH-3508)

PROJECT: 1894152

APPENDIX D

Water Quality Results



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C.O.C.: G84846

Final Report

REPORT No. B18-28353 (i)

Report To: Golder Associates Ltd. 121 Commerce Park Drive, Unit L, Barrie ON. L4N 8X1 Canada

Attention: Devin Hannan

DATE RECEIVED: 18-Sep-18

DATE REPORTED: 25-Sep-18

SAMPLE MATRIX: Groundwater

Caduceon Environmental Laboratories

CERTIFICATE OF ANALYSIS

110 West Beaver Creek Rd Unit 14 Richmond Hill ON L4B 1J9 Tel: 289-475-5442 Fax: 289-562-1963

JOB/PROJECT NO .: Lafarge - Goodwood Pit

P.O. NUMBER: 1894152 (2000)

WATERWORKS NO.

Parameter	Qty	Site Analyzed	Analyst Initials	Date Analyzed	Lab Method	Reference Method
Alkalinity (as CaCO3)	3	Holly Lane	SYL	19-Sep-18	A-ALK-03 (o)	SM 2320B
Conductivity	3	Holly Lane	SYL	19-Sep-18	A-COND-02 (o)	SM 2510B
Anions	3	Holly Lane	VSC	19-Sep-18	A-IC-01 (o)	SM4110C
pН	3	Holly Lane	SYL	19-Sep-18	A-PH-01 (o)	SM 4500H
B - Bacteriological	3	Barrie	SAN	20-Sep-18	B-EC-001 (b)	SM9222B
Chromium (VI)	3	Holly Lane	JGC	21-Sep-18	D-CRVI-01 (o)	MOE E3056
Mercury	3	Holly Lane	PBK	24-Sep-18	D-HG-02 (o)	SM 3112 B
Metals - ICP-OES	3	Holly Lane	TPR	20-Sep-18	D-ICP-01 (o)	SM 3120
Metals - ICP-MS	3	Holly Lane	TPR	20-Sep-18	D-ICPMS-01 (o)	EPA 200.8
Calculation	3	Holly Lane	JGC	21-Sep-18	D-Ion Balance	Calc.

O. Reg. 153 - Soil, Ground Water and Sediment Standards Tbl. 2 - PGW - Table 2 - Potable Ground Water

R.L. = Reporting Limit

Christine Burke Lab Manager

Site Analyzed=K-Kingston,W-Windsor,O-Ottawa,R-Richmond Hill,B-Barrie
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121 Commerce Park Drive, Unit L,	Richmond Hill ON L4B 1J9
Barrie ON. L4N 8X1 Canada	Tel: 289-475-5442
Attention: Devin Hannan	Fax: 289-562-1963
DATE RECEIVED: 18-Sep-18	JOB/PROJECT NO.: Lafarge - Goodwood Pit
DATE REPORTED: 25-Sep-18	P.O. NUMBER: 1894152 (2000)
SAMPLE MATRIX: Groundwater	WATERWORKS NO.

	Client I.D.		MW 18-01	MW 18-02	PW	O. Reg. 153
	Sample I.D).	B18-28353-1	B18-28353-2	B18-28353-3	Tbl. 2 -
	Date Colle	cted	17-Sep-18	17-Sep-18	17-Sep-18	PGW
Parameter	Units	R.L.				
E coli	cfu/100mL	1	2	< 2	< 2	
pH @25°C	pH Units		7.76	7.95	8.14	
Conductivity @25°C	mS/cm	0.001	1.05	0.515	0.46	
Alkalinity(CaCO3) to pH4.5	mg/L	5	233	179	168	
Carbonate (as CaCO3)	mg/L	5	< 5	< 5	< 5	
Bicarbonate(as CaCO3)	mg/L	5	233	179	168	
Hydroxide (as CaCO3)	mg/L	5	< 5	< 5	< 5	
Hardness (as CaCO3)	mg/L	1	359	248	223	
Chloride	µg/L	500	158000	25100	15100	790000
Fluoride	µg/L	100	< 100	< 100	< 100	
Sodium	µg/L	200	84500	10900	7000	490000
Antimony	µg/L	0.1	5.0	0.6	< 0.1	6
Arsenic	µg/L	0.1	0.3	0.1	< 0.1	25
Barium	µg/L	1	118	49	41	1000
Beryllium	µg/L	0.1	< 0.1	< 0.1	< 0.1	4
Boron	µg/L	5	27	7	< 5	5000
Cadmium	µg/L	0.015	< 0.015	< 0.015	< 0.015	2.7
Chromium	µg/L	2	< 2	< 2	< 2	50
Chromium (VI)	µg/L	2	< 2	1 < 2 1	< 2 1	25
Cobalt	µg/L	0.1	< 0.1	< 0.1	< 0.1	3.8
Copper	µg/L	2	< 2	< 2	2	87
Lead	µg/L	0.02	0.05	0.04	0.10	10
Mercury	µg/L	0.02	< 0.02	< 0.02	< 0.02	0.29

O. Reg. 153 - Soil, Ground Water and Sediment Standards Tbl. 2 - PGW - Table 2 - Potable Ground Water

R.L. = Reporting Limit

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Christine Burke Lab Manager



Client committed. Quality assured.

C.O.C.: G84846

Final Report

REPORT No. B18-28353 (i)

Report To:	Caduceon Environmental Laboratories
Golder Associates Ltd.	110 West Beaver Creek Rd Unit 14
121 Commerce Park Drive, Unit L,	Richmond Hill ON L4B 1J9
Barrie ON. L4N 8X1 Canada	Tel: 289-475-5442
Attention: Devin Hannan	Fax: 289-562-1963
DATE RECEIVED: 18-Sep-18	JOB/PROJECT NO.: Lafarge - Goodwood Pit
DATE REPORTED: 25-Sep-18	P.O. NUMBER: 1894152 (2000)
SAMPLE MATRIX: Groundwater	WATERWORKS NO.

	Client I.D.		MW 18-01	MW 18-02	PW	O. Reg. 153
Sample I.D.).	B18-28353-1	B18-28353-2	B18-28353-3	Tbl. 2 -
	Date Colle	cted	17-Sep-18	17-Sep-18	17-Sep-18	PGW
Parameter	Units	R.L.				
Molybdenum	µg/L	0.1	0.6	0.4	0.3	70
Nickel	µg/L	10	< 10	< 10	< 10	100
Selenium	µg/L	1	< 1	< 1	< 1	10
Silver	µg/L	0.1	< 0.1	< 0.1	< 0.1	1.5
Thallium	µg/L	0.05	< 0.05	< 0.05	< 0.05	2
Uranium	µg/L	0.05	0.87	0.76	0.78	20
Vanadium	µg/L	0.1	0.4	0.2	0.2	6.2
Zinc	µg/L	5	< 5	< 5	12	1100
Anion Sum	meq/L		10.2	5.08	4.56	
Cation Sum	meq/L		10.9	5.47	4.78	
% Difference	%		3.61	3.66	2.35	
Ion Ratio	AS/CS		0.930	0.929	0.954	
Sodium Adsorption Ratio	-		1.94	0.301	0.203	
TDS(ion sum calc.)	mg/L	1	570	277	246	
Conductivity (calc.)	µmho/cm		1050	516	456	
TDS(calc.)/EC(actual)	-		0.544	0.537	0.534	
EC(calc.)/EC(actual)	-		1.00	1.00	0.990	
Langelier Index(25°C)	S.I.		0.693	0.620	0.741	

1 Chromium (VI) result is based on total chromium

O. Reg. 153 - Soil, Ground Water and Sediment Standards Tbl. 2 - PGW - Table 2 - Potable Ground Water

Christine Burke Lab Manager

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Barrie ON. L4N 8X1 Canada	Tel: 289-475-5442
Attention: Devin Hannan	Fax: 289-562-1963
DATE RECEIVED: 18-Sep-18	JOB/PROJECT NO.: Lafarge - Goodwood Pit
DATE REPORTED: 25-Sep-18	P.O. NUMBER: 1894152 (2000)
SAMPLE MATRIX: Groundwater	WATERWORKS NO.

Summary of Exceedances

O. Reg. 153 - Soil, Ground Water and Sediment Standards Tbl. 2 - PGW - Table 2 - Potable Ground Water

R.L. = Reporting Limit

Christine Burke Lab Manager

Site Analyzed=K-Kingston,W-Windsor,O-Ottawa,R-Richmond Hill,B-Barrie
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C.O.C.: G84846

Final Report

REPORT No. B18-28353 (ii)

Report To: Golder Associates Ltd. 121 Commerce Park Drive, Unit L, Barrie ON. L4N 8X1 Canada

Attention: Devin Hannan

DATE RECEIVED: 18-Sep-18

DATE REPORTED: 25-Sep-18

SAMPLE MATRIX: Groundwater

Caduceon Environmental L	aboratories
--------------------------	-------------

110 West Beaver Creek Rd Unit 14 Richmond Hill ON L4B 1J9 Tel: 289-475-5442 Fax: 289-562-1963

JOB/PROJECT NO .: Lafarge - Goodwood Pit

P.O. NUMBER: 1894152 (2000) WATERWORKS NO.

Parameter	Qty	Site Analyzed	Analyst Initials	Date Analyzed	Lab Method	Reference Method
PHC(F2-F4)	3	Kingston	KPR	20-Sep-18	C-PHC-W-001 (k)	MOE E3421
VOC's	3	Richmond Hill	FAL	19-Sep-18	C-VOC-02 (rh)	EPA 8260
PHC(F1)	3	Richmond Hill	FAL	19-Sep-18	C-VPHW-01 (rh)	MOE E3421

O. Reg. 153 - Soil, Ground Water and Sediment Standards Tbl. 2 - PGW - Table 2 - Potable Ground Water

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DATE REPORTED: 25-Sep-18	P.O. NUMBER: 1894152 (2000)
SAMPLE MATRIX: Groundwater	WATERWORKS NO.

	Client I.D. Sample I.D.		MW 18-01	MW 18-02	PW	O. Reg. 153
			B18-28353-1 B18-28353-	B18-28353-2	B18-28353-3	Tbl. 2 -
	Date Colle	cted	17-Sep-18	17-Sep-18	17-Sep-18	PGW
Parameter	Units	R.L.				
Acetone	µg/L	30	< 30	< 30	< 30	2700
Benzene	µg/L	0.5	< 0.5	< 0.5	< 0.5	5
Bromodichloromethane	µg/L	2	< 2	< 2	< 2	16
Bromoform	µg/L	5	< 5	< 5	< 5	25
Bromomethane	µg/L	0.5	< 0.5	< 0.5	< 0.5	0.89
Carbon Tetrachloride	µg/L	0.2	< 0.2	< 0.2	< 0.2	0.79
Monochlorobenzene (Chlorobenzene)	µg/L	0.5	< 0.5	< 0.5	< 0.5	30
Chloroform	µg/L	1	< 1	< 1	< 1	2.4
Dibromochloromethane	µg/L	2	< 2	< 2	< 2	25
Dichlorobenzene,1,2-	µg/L	0.5	< 0.5	< 0.5	< 0.5	3
Dichlorobenzene,1,3-	µg/L	0.5	< 0.5	< 0.5	< 0.5	59
Dichlorobenzene,1,4-	µg/L	0.5	< 0.5	< 0.5	< 0.5	1
Dichlorodifluoromethane	µg/L	2	< 2	< 2	< 2	590
Dichloroethane,1,1-	µg/L	0.5	< 0.5	< 0.5	< 0.5	5
Dichloroethane,1,2-	µg/L	0.5	< 0.5	< 0.5	< 0.5	1.6
Dichloroethylene,1,1-	µg/L	0.5	< 0.5	< 0.5	< 0.5	1.6
Dichloroethene, cis-1,2-	µg/L	0.5	< 0.5	< 0.5	< 0.5	1.6
Dichloroethene, trans-1,2-	µg/L	0.5	< 0.5	< 0.5	< 0.5	1.6
Dichloropropane,1,2-	µg/L	0.5	< 0.5	< 0.5	< 0.5	5
Dichloropropene, cis-1,3-	µg/L	0.5	< 0.5	< 0.5	< 0.5	
Dichloropropene, trans- 1,3-	µg/L	0.5	< 0.5	< 0.5	< 0.5	

O. Reg. 153 - Soil, Ground Water and Sediment Standards Tbl. 2 - PGW - Table 2 - Potable Ground Water

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DATE REPORTED: 25-Sep-18	P.O. NUMBER: 1894152 (2000)
SAMPLE MATRIX: Groundwater	WATERWORKS NO.

ſ	Client I.D. Sample I.D.		MW 18-01	MW 18-02	PW	O. Reg. 153
			B18-28353-1 B18-28	B18-28353-2	8-28353-2 B18-28353-3	Tbl. 2 -
	Date Colle	ected	17-Sep-18	17-Sep-18	17-Sep-18	PGW
Parameter	Units	R.L.				
Dichloropropene 1,3- cis+trans	µg/L	0.5	< 0.5	< 0.5	< 0.5	0.5
Ethylbenzene	µg/L	0.5	< 0.5	< 0.5	< 0.5	2.4
Dibromoethane,1,2- (Ethylene Dibromide)	µg/L	0.2	< 0.2	< 0.2	< 0.2	0.2
Hexane	µg/L	5	< 5	< 5	< 5	51
Methyl Ethyl Ketone	µg/L	20	< 20	< 20	< 20	1800
Methyl Isobutyl Ketone	µg/L	20	< 20	< 20	< 20	640
Methyl-t-butyl Ether	µg/L	2	< 2	< 2	< 2	15
Dichloromethane (Methylene Chloride)	µg/L	0.3	< 0.3	< 0.3	< 0.3	50
Styrene	µg/L	0.5	< 0.5	< 0.5	< 0.5	5.4
Tetrachloroethane,1,1,1,2	µg/L	0.5	< 0.5	< 0.5	< 0.5	1.1
Tetrachloroethane,1,1,2,2	µg/L	0.5	< 0.5	< 0.5	< 0.5	1
Tetrachloroethylene	µg/L	0.5	< 0.5	< 0.5	< 0.5	1.6
Toluene	µg/L	0.5	< 0.5	< 0.5	< 0.5	24
Trichloroethane,1,1,1-	µg/L	0.5	< 0.5	< 0.5	< 0.5	200
Trichloroethane,1,1,2-	µg/L	0.5	< 0.5	< 0.5	< 0.5	4.7
Trichloroethylene	µg/L	0.5	< 0.5	< 0.5	< 0.5	1.6
Trichlorofluoromethane	µg/L	5	< 5	< 5	< 5	150
Vinyl Chloride	µg/L	0.5	< 0.5	< 0.5	< 0.5	0.5
Xylene, m,p-	µg/L	1.0	< 1.0	< 1.0	< 1.0	
Xylene, o-	µg/L	0.5	< 0.5	< 0.5	< 0.5	

O. Reg. 153 - Soil, Ground Water and Sediment Standards

Tbl. 2 - PGW - Table 2 - Potable Ground Water

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Attention: Devin Hannan	Fax: 289-562-1963
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DATE REPORTED: 25-Sep-18	P.O. NUMBER: 1894152 (2000)
SAMPLE MATRIX: Groundwater	WATERWORKS NO.

	Client I.D.	Client I.D. Sample I.D.		Client I.D. MW 18-01	MW 18-02 PW	O. Reg. 153	
	Sample I.I			B18-28353-2	B18-28353-3	Tbl. 2 -	
	Date Colle	cted	17-Sep-18	17-Sep-18	17-Sep-18	PGW	
Parameter	Units	R.L.					
Xylene, m,p,o-	µg/L	1.1	< 1.1	< 1.1	< 1.1	300	
PHC F1 (C6-C10)	µg/L	50	< 50	< 50	< 50	750	
PHC F2 (>C10-C16)	µg/L	50	< 50	< 50	< 50	150	
PHC F3 (>C16-C34)	µg/L	400	2400	< 400	< 400	500	
PHC F4 (>C34-C50)	µg/L	400	< 400	< 400	< 400	500	

O. Reg. 153 - Soil, Ground Water and Sediment Standards Tbl. 2 - PGW - Table 2 - Potable Ground Water

R.L. = Reporting Limit

Christine Burke Lab Manager

Test methods may be modified from specified reference method unless indicated by an * Site Analyzed=K-Kingston,W-Windsor,O-Ottawa,R-Richmond Hill,B-Barrie



Client committed. Quality assured.

C.O.C.: G84846

Final Report

REPORT No. B18-28353 (ii)

CERTIFICATE OF ANALYSIS

Report To:	Caduceon Environmental Laboratories		
Golder Associates Ltd.	110 West Beaver Creek Rd Unit 14		
121 Commerce Park Drive, Unit L,	Richmond Hill ON L4B 1J9		
Barrie ON. L4N 8X1 Canada	Tel: 289-475-5442		
Attention: Devin Hannan	Fax: 289-562-1963		
DATE RECEIVED: 18-Sep-18	JOB/PROJECT NO .: Lafarge - Goodwood Pit		
DATE REPORTED: 25-Sep-18	P.O. NUMBER: 1894152 (2000)		
SAMPLE MATRIX: Groundwater	WATERWORKS NO.		

Summary of Exceedances

Table 2 - Potable Ground Water		
MW 18-01	Found Value	Limit
PHC F3 (>C16-C34) (µg/L)	2400	500

O. Reg. 153 - Soil, Ground Water and Sediment Standards Tbl. 2 - PGW - Table 2 - Potable Ground Water

R.L. = Reporting Limit

Christine Burke Lab Manager

Site Analyzed=K-Kingston,W-Windsor,O-Ottawa,R-Richmond Hill,B-Barrie The analytical results reported herein refer to the samples as received. Reproduction of this analytical report in full or in part is prohibited without prior consent from

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Caduceon Environmental Laboratories.



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CERTIFICATE OF ANALYSIS

Final Report

C.O.C.: G84846

REPORT No. B18-28353 (iii)

Report To: **Caduceon Environmental Laboratories** Golder Associates Ltd. 110 West Beaver Creek Rd Unit 14 121 Commerce Park Drive, Unit L, Richmond Hill ON L4B 1J9 Barrie ON. L4N 8X1 Canada Tel: 289-475-5442 Attention: Devin Hannan Fax: 289-562-1963 JOB/PROJECT NO .: Lafarge - Goodwood Pit DATE RECEIVED: 18-Sep-18 DATE REPORTED: 25-Sep-18 P.O. NUMBER: 1894152 (2000) SAMPLE MATRIX: Groundwater WATERWORKS NO.

Parameter	Qty	Site Analyzed	Analyst Initials	Date Analyzed	Lab Method	Reference Method
SVOC	1	Kingston	sge	24-Sep-18	C-NAB-W-001 (k)	EPA 8270

O. Reg. 153 - Soil, Ground Water and Sediment Standards Tbl. 2 - PGW - Table 2 - Potable Ground Water

R.L. = Reporting Limit

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Final Report

REPORT No. B18-28353 (iii)

Caduceon Environmental Laboratories	
110 West Beaver Creek Rd Unit 14	
Richmond Hill ON L4B 1J9	
Tel: 289-475-5442	
Fax: 289-562-1963	
JOB/PROJECT NO.: Lafarge - Goodwood Pit	
P.O. NUMBER: 1894152 (2000)	
WATERWORKS NO.	

	Client I.D. Sample I.D.		MW 18-02		O. Reg. 153
			B18-28353-2 17-Sep-18		Tbl. 2 - PGW
Date Collected		ected			
Parameter	Units	R.L.			
Acenaphthene	µg/L	0.05	< 0.05		4.1
Acenaphthylene	µg/L	0.05	< 0.05		1
Anthracene	µg/L	0.05	< 0.05		2.4
Benzo(a)anthracene	µg/L	0.05	< 0.05		1
Benzo(a)pyrene	µg/L	0.01	< 0.01		0.01
Benzo(b)fluoranthene	µg/L	0.05	< 0.05		0.1
Benzo(b+k)fluoranthene	µg/L	0.1	< 0.1		
Benzo(g,h,i)perylene	µg/L	0.05	< 0.05		0.2
Benzo(k)fluoranthene	µg/L	0.05	< 0.05		0.1
Chrysene	µg/L	0.05	< 0.05		0.1
Dibenzo(a,h)anthracene	µg/L	0.05	< 0.05		0.2
Fluoranthene	µg/L	0.05	< 0.05		0.41
Fluorene	µg/L	0.05	< 0.05		120
Indeno(1,2,3,-cd)pyrene	µg/L	0.05	< 0.05		0.2
Methylnaphthalene,1-	µg/L	0.05	< 0.05		3.2
Methylnaphthalene,2-	µg/L	0.05	< 0.05		3.2
Methylnaphthalene 2-(1-)	µg/L	0.07	< 0.07		3.2
Naphthalene	µg/L	0.05	< 0.05		11
Phenanthrene	µg/L	0.05	< 0.05		1
Pyrene	µg/L	0.05	< 0.05		4.1

O. Reg. 153 - Soil, Ground Water and Sediment Standards Tbl. 2 - PGW - Table 2 - Potable Ground Water

Christine Burke Lab Manager

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Final Report

REPORT No. B18-28353 (iii)

Report To:	Caduceon Environmental Laboratories		
Golder Associates Ltd.	110 West Beaver Creek Rd Unit 14		
121 Commerce Park Drive, Unit L,	Richmond Hill ON L4B 1J9		
Barrie ON. L4N 8X1 Canada	Tel: 289-475-5442		
Attention: Devin Hannan	Fax: 289-562-1963		
DATE RECEIVED: 18-Sep-18	JOB/PROJECT NO.: Lafarge - Goodwood Pit		
DATE REPORTED: 25-Sep-18	P.O. NUMBER: 1894152 (2000)		
SAMPLE MATRIX: Groundwater	WATERWORKS NO.		

Summary of Exceedances

O. Reg. 153 - Soil, Ground Water and Sediment Standards Tbl. 2 - PGW - Table 2 - Potable Ground Water

R.L. = Reporting Limit

Christine Burke Lab Manager

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Test methods may be modified from specified reference method unless indicated by an *

Site Analyzed=K-Kingston,W-Windsor,O-Ottawa,R-Richmond Hill,B-Barrie



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C.O.C.: G75461

Final Report

REPORT No. B18-32587 (i)

CERTIFICATE OF ANALYSIS

Report To:	Caduceon Environmental Laboratories
Golder Associates Ltd.	112 Commerce Park Drive
121 Commerce Park Drive, Unit L,	Barrie ON L4N 8W8
Barrie ON. L4N 8X1 Canada	Tel: 705-252-5743
Attention: Devin Hannan	Fax: 705-252-5746
DATE RECEIVED: 22-Oct-18	JOB/PROJECT NO.: Lafarge - Goodwood Pit
DATE REPORTED: 02-Nov-18	P.O. NUMBER: 1894152/2000/2030
SAMPLE MATRIX: Groundwater	WATERWORKS NO.

		1	Client I.D.		MW 18-03		1
			Sample I.D.		B18-32587-1		
			Date Collect	ed	22-Oct-18		1
Parameter	Units	R.L.	Reference Method	Date/Site Analyzed			
Total Coliform	cfu/100mL	1	SM9222B	22-Oct-18/B	10		
E coli	cfu/100mL	1	SM9222B	22-Oct-18/B	0		
pH @25°C	pH Units		SM 4500H	26-Oct-18/O	8.09		
Conductivity @25°C	µmho/cm	1	SM 2510B	26-Oct-18/O	497		
Alkalinity(CaCO3) to pH4.5	mg/L	5	SM 2320B	26-Oct-18/O	190		
Carbonate (as CaCO3)	mg/L	5	SM 2320B	26-Oct-18/O	< 5		
Bicarbonate(as CaCO3)	mg/L	5	SM 2320B	26-Oct-18/O	190		
Hydroxide (as CaCO3)	mg/L	5	SM 2320B	26-Oct-18/O	< 5		
Hardness (as CaCO3)	mg/L	1	SM 3120	02-Nov-18/O	239		
Chloride	mg/L	0.5	SM4110C	23-Oct-18/O	11.8		
Fluoride	mg/L	0.1	SM4110C	23-Oct-18/O	< 0.1		
Nitrate (N)	mg/L	0.1	SM4110C	23-Oct-18/O	4.7		
Nitrite (N)	mg/L	0.1	SM4110C	23-Oct-18/O	< 0.1		
Sodium	mg/L	0.2	SM 3120	02-Nov-18/O	3.1		
Antimony	mg/L	0.0001	EPA 200.8	23-Oct-18/O	< 0.0001		
Arsenic	mg/L	0.0001	EPA 200.8	23-Oct-18/O	0.0003		
Barium	mg/L	0.001	SM 3120	02-Nov-18/O	0.053		
Beryllium	mg/L	0.0001	EPA 200.8	23-Oct-18/O	< 0.0001		
Boron	mg/L	0.005	SM 3120	02-Nov-18/O	0.009		
Cadmium	mg/L).000015	EPA 200.8	23-Oct-18/O	< 0.000015		
Chromium	mg/L	0.002	SM 3120	02-Nov-18/O	< 0.002		
Chromium (VI)	mg/L	0.002	MOE E3056	24-Oct-18/O	< 0.002		
Cobalt	mg/L	0.0001	EPA 200.8	23-Oct-18/O	< 0.0001		
Copper	mg/L	0.002	SM 3120	02-Nov-18/O	< 0.002		
Lead	mg/L	0.00002	EPA 200.8	23-Oct-18/O	0.00003		

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C.O.C.: G75461

Final Report

REPORT No. B18-32587 (i)

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Report To:	Caduceon Environmental Laboratories
Golder Associates Ltd.	112 Commerce Park Drive
121 Commerce Park Drive, Unit L,	Barrie ON L4N 8W8
Barrie ON. L4N 8X1 Canada	Tel: 705-252-5743
Attention: Devin Hannan	Fax: 705-252-5746
DATE RECEIVED: 22-Oct-18	JOB/PROJECT NO.: Lafarge - Goodwood Pit
DATE REPORTED: 02-Nov-18	P.O. NUMBER: 1894152/2000/2030
SAMPLE MATRIX: Groundwater	WATERWORKS NO.

		ſ	Client I.D.		MW 18-03		
			Sample I.D.		B18-32587-1		
			Date Collect	ed	22-Oct-18		
Parameter	Units	R.L.	Reference Method	Date/Site Analyzed			
Mercury	mg/L	0.00002	SM 3112 B	30-Oct-18/O	< 0.00002		
Molybdenum	mg/L	0.0001	EPA 200.8	23-Oct-18/O	0.0141		
Nickel	mg/L	0.01	SM 3120	02-Nov-18/O	< 0.01		
Selenium	mg/L	0.001	EPA 200.8	23-Oct-18/O	< 0.001		
Silver	mg/L	0.0001	EPA 200.8	23-Oct-18/O	< 0.0001		
Thallium	mg/L	0.00005	EPA 200.8	23-Oct-18/O	< 0.00005		
Uranium	mg/L	0.00005	EPA 200.8	23-Oct-18/O	0.00164		
Vanadium	mg/L	0.0001	EPA 200.8	23-Oct-18/O	0.0003		
Zinc	mg/L	0.005	SM 3120	02-Nov-18/O	< 0.005		
Anion Sum	meq/L		Calc.	02-Nov-18/O	5.13		
Cation Sum	meq/L		Calc.	02-Nov-18/O	4.94		
% Difference	%		Calc.	02-Nov-18/O	1.89		
Ion Ratio	AS/CS		Calc.	02-Nov-18/O	1.04		
Sodium Adsorption Ratio	-		Calc.	02-Nov-18/O	0.0872		
TDS(ion sum calc.)	mg/L	1	Calc.	02-Nov-18/O	266		
Conductivity (calc.)	µmho/cm		Calc.	02-Nov-18/O	484		
TDS(calc.)/EC(actual)	-		Calc.	02-Nov-18/O	0.536		
EC(calc.)/EC(actual)	-		Calc.	02-Nov-18/O	0.975		
Langelier Index(25°C)	S.I.		Calc.	02-Nov-18/O	0.745	 	

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CERTIFICATE OF ANALYSIS

C.O.C.: G75461

R.L. = Reporting Limit

Final Report

REPORT No. B18-32587 (ii)

Report To:	Caduceon Environmental Laboratories
Golder Associates Ltd.	112 Commerce Park Drive
121 Commerce Park Drive, Unit L,	Barrie ON L4N 8W8
Barrie ON. L4N 8X1 Canada	Tel: 705-252-5743
Attention: Devin Hannan	Fax: 705-252-5746
DATE RECEIVED: 22-Oct-18	JOB/PROJECT NO.: Lafarge - Goodwood Pit
DATE REPORTED: 02-Nov-18	P.O. NUMBER: 1894152/2000/2030
SAMPLE MATRIX: Groundwater	WATERWORKS NO.

			Client I.D.		MW 18-03		
			Sample I.D.		B18-32587-1		
			Date Collect	ed	22-Oct-18		
Parameter	Units	R.L.	Reference Method	Date/Site Analyzed			
Acetone	µg/L	30	EPA 8260	24-Oct-18/R	< 30		
Benzene	µg/L	0.5	EPA 8260	24-Oct-18/R	< 0.5		
Bromodichloromethane	µg/L	2	EPA 8260	24-Oct-18/R	< 2		
Bromoform	µg/L	5	EPA 8260	24-Oct-18/R	< 5		
Bromomethane	µg/L	0.5	EPA 8260	24-Oct-18/R	< 0.5		
Carbon Tetrachloride	µg/L	0.2	EPA 8260	24-Oct-18/R	< 0.2		
Monochlorobenzene (Chlorobenzene)	µg/L	0.5	EPA 8260	24-Oct-18/R	< 0.5		
Chloroform	µg/L	1	EPA 8260	24-Oct-18/R	< 1		
Dibromochloromethane	µg/L	2	EPA 8260	24-Oct-18/R	< 2		
Dichlorobenzene,1,2-	µg/L	0.5	EPA 8260	24-Oct-18/R	< 0.5		
Dichlorobenzene,1,3-	µg/L	0.5	EPA 8260	24-Oct-18/R	< 0.5		
Dichlorobenzene,1,4-	µg/L	0.5	EPA 8260	24-Oct-18/R	< 0.5		
Dichlorodifluoromethane	µg/L	2	EPA 8260	24-Oct-18/R	< 2		
Dichloroethane,1,1-	µg/L	0.5	EPA 8260	24-Oct-18/R	< 0.5		
Dichloroethane,1,2-	µg/L	0.5	EPA 8260	24-Oct-18/R	< 0.5		
Dichloroethylene,1,1-	µg/L	0.5	EPA 8260	24-Oct-18/R	< 0.5		
Dichloroethene, cis-1,2-	µg/L	0.5	EPA 8260	24-Oct-18/R	< 0.5		
Dichloroethene, trans-1,2-	µg/L	0.5	EPA 8260	24-Oct-18/R	< 0.5		
Dichloropropane,1,2-	µg/L	0.5	EPA 8260	24-Oct-18/R	< 0.5		
Dichloropropene, cis-1,3-	µg/L	0.5	EPA 8260	24-Oct-18/R	< 0.5		
Dichloropropene, trans-1,3-	µg/L	0.5	EPA 8260	24-Oct-18/R	< 0.5		
Dichloropropene 1,3- cis+trans	µg/L	0.5	EPA 8260	24-Oct-18/R	< 0.5		
Ethylbenzene	µg/L	0.5	EPA 8260	24-Oct-18/R	< 0.5		

Christine Burke Lab Manager

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CERTIFICATE OF ANALYSIS

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Final Report

REPORT No. B18-32587 (ii)

Report To:	Caduceon Environmental Laboratories
Golder Associates Ltd.	112 Commerce Park Drive
121 Commerce Park Drive, Unit L,	Barrie ON L4N 8W8
Barrie ON. L4N 8X1 Canada	Tel: 705-252-5743
Attention: Devin Hannan	Fax: 705-252-5746
DATE RECEIVED: 22-Oct-18	JOB/PROJECT NO .: Lafarge - Goodwood Pit
DATE REPORTED: 02-Nov-18	P.O. NUMBER: 1894152/2000/2030
SAMPLE MATRIX: Groundwater	WATERWORKS NO.

			Client I.D.		MW 18-03		
			Sample I.D.		B18-32587-1		
			Date Collecte	ed	22-Oct-18		
Parameter	Units	R.L.	Reference Method	Date/Site Analyzed			
Dibromoethane,1,2- (Ethylene Dibromide)	µg/L	0.2	EPA 8260	24-Oct-18/R	< 0.2		
Hexane	µg/L	5	EPA 8260	24-Oct-18/R	< 5		
Methyl Ethyl Ketone	µg/L	20	EPA 8260	24-Oct-18/R	< 20		
Methyl Isobutyl Ketone	µg/L	20	EPA 8260	24-Oct-18/R	< 20		
Methyl-t-butyl Ether	µg/L	2	EPA 8260	24-Oct-18/R	< 2		
Dichloromethane (Methylene Chloride)	µg/L	0.3	EPA 8260	24-Oct-18/R	< 0.3		
Styrene	µg/L	0.5	EPA 8260	24-Oct-18/R	< 0.5		
Tetrachloroethane,1,1,1,2-	µg/L	0.5	EPA 8260	24-Oct-18/R	< 0.5		
Tetrachloroethane,1,1,2,2-	µg/L	0.5	EPA 8260	24-Oct-18/R	< 0.5		
Tetrachloroethylene	µg/L	0.5	EPA 8260	24-Oct-18/R	< 0.5		
Toluene	µg/L	0.5	EPA 8260	24-Oct-18/R	< 0.5		
Trichloroethane,1,1,1-	µg/L	0.5	EPA 8260	24-Oct-18/R	< 0.5		
Trichloroethane,1,1,2-	µg/L	0.5	EPA 8260	24-Oct-18/R	< 0.5		
Trichloroethylene	µg/L	0.5	EPA 8260	24-Oct-18/R	< 0.5		
Trichlorofluoromethane	µg/L	5	EPA 8260	24-Oct-18/R	< 5		
Vinyl Chloride	µg/L	0.5	EPA 8260	24-Oct-18/R	< 0.5		
Xylene, m,p-	µg/L	1.0	EPA 8260	24-Oct-18/R	< 1.0		
Xylene, o-	µg/L	0.5	EPA 8260	24-Oct-18/R	< 0.5		
Xylene, m,p,o-	µg/L	1.1	EPA 8260	24-Oct-18/R	< 1.1		
PHC F1 (C6-C10)	µg/L	50	MOE E3421	24-Oct-18/R	< 50		
PHC F2 (>C10-C16)	µg/L	50	MOE E3421	24-Oct-18/K	80		
PHC F3 (>C16-C34)	µg/L	400	MOE E3421	24-Oct-18/K	400		
PHC F4 (>C34-C50)	µg/L	400	MOE E3421	24-Oct-18/K	< 400		

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Christine Burke Lab Manager



CERTIFICATE OF ANALYSIS

C.O.C.: G75461

Report To:

Golder Associates Ltd. 121 Commerce Park Drive, Unit L,

Barrie ON. L4N 8X1 Canada Attention: Devin Hannan DATE RECEIVED: 22-Oct-18

DATE REPORTED: 02-Nov-18

SAMPLE MATRIX: Groundwater

Final Report

REPORT No. B18-32587 (iii)

Caduceon Environmental Laboratories 112 Commerce Park Drive

Barrie ON L4N 8W8 Tel: 705-252-5743 Fax: 705-252-5746

JOB/PROJECT NO .: Lafarge - Goodwood Pit

P.O. NUMBER: 1894152/2000/2030

WATERWORKS NO.

			Client I.D.		MW 18-03		
			Sample I.D.		B18-32587-1		
			Date Collect	ed	22-Oct-18		
Parameter	Units	R.L.	Reference Method	Date/Site Analyzed		L	
Acenaphthene	µg/L	0.05	EPA 8270	25-Oct-18/K	< 0.05		
Acenaphthylene	µg/L	0.05	EPA 8270	25-Oct-18/K	< 0.05		
Anthracene	µg/L	0.05	EPA 8270	25-Oct-18/K	< 0.05		
Benzo(a)anthracene	µg/L	0.05	EPA 8270	25-Oct-18/K	< 0.05		
Benzo(a)pyrene	µg/L	0.01	EPA 8270	25-Oct-18/K	< 0.01		
Benzo(b)fluoranthene	µg/L	0.05	EPA 8270	25-Oct-18/K	< 0.05		
Benzo(b+k)fluoranthene	µg/L	0.1	EPA 8270	25-Oct-18/K	< 0.1		
Benzo(g,h,i)perylene	µg/L	0.05	EPA 8270	25-Oct-18/K	< 0.05		
Benzo(k)fluoranthene	µg/L	0.05	EPA 8270	25-Oct-18/K	< 0.05		
Chrysene	µg/L	0.05	EPA 8270	25-Oct-18/K	< 0.05		
Dibenzo(a,h)anthracene	µg/L	0.05	EPA 8270	25-Oct-18/K	< 0.05		
Fluoranthene	µg/L	0.05	EPA 8270	25-Oct-18/K	< 0.05		
Fluorene	µg/L	0.05	EPA 8270	25-Oct-18/K	< 0.05		
Indeno(1,2,3,-cd)pyrene	µg/L	0.05	EPA 8270	25-Oct-18/K	< 0.05		
Methylnaphthalene,1-	µg/L	0.05	EPA 8270	25-Oct-18/K	< 0.05		
Methylnaphthalene,2-	µg/L	0.05	EPA 8270	25-Oct-18/K	< 0.05		
Methylnaphthalene 2-(1-)	µg/L	0.07	EPA 8270	25-Oct-18/K	< 0.07		
Naphthalene	µg/L	0.05	EPA 8270	25-Oct-18/K	< 0.05		
Phenanthrene	µg/L	0.05	EPA 8270	25-Oct-18/K	< 0.05		
Pyrene	μg/L	0.05	EPA 8270	25-Oct-18/K	< 0.05		

R.L. = Reporting Limit

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Christine Burke Lab Manager



CLIENT NAME: GOLDER ASSOCIATES LTD. 121 COMMERCE PARK DRIVE, UNIT L BARRIE, ON L4N8X1 (705) 722-4492

ATTENTION TO: Devin Hannan

PROJECT: 1894152(2110)

AGAT WORK ORDER: 19T460288

TRACE ORGANICS REVIEWED BY: Pinkal Patel, Report Reviewer

DATE REPORTED: May 01, 2019

PAGES (INCLUDING COVER): 5

VERSION*: 1

Should you require any information regarding this analysis please contact your client services representative at (905) 712-5100

*NOTES		
INDIES		
1		
1		
1		
1		
1		
1		

All samples will be disposed of within 30 days following analysis. Please contact the lab if you require additional sample storage time.

 AGAT Laboratories (V1)
 Page 1 of 5

 Member of: Association of Professional Engineers and Geoscientists of Alberta (APEGA)
 AGAT Laboratories is accredited to ISO/IEC 17025 by the Canadian Association for Laboratory

 Accreditation Inc. (CALA) and/or Standards Council of Canada (SCC) for specific citests listed on the scope of accreditation Inc. (CALA) and/or Standards Council of Subscription of Alberta (ESAA)

 Brivinonmental Services Association of Alberta (ESAA)
 AGAT Laboratories (Mississauga) is also accredited by the Canadian Association for Laboratory Accreditation Inc. (CALA) for specific citests listed on the scope of accreditation Inc. (CALA) for specific drivinking water tests. Accreditations are location and parameter specific. A complete listing of parameters for each location is available from www.cala.ca and/or www.scc.ca. The tests in this report may not necessarily be included in the scope of accreditation. Measurement Uncertainty is not taken into consideration when stating conformity with a specified requirement.

Results relate only to the items tested. Results apply to samples as received. All reportable information as specified by ISO 17025:2017 is available from AGAT Laboratories upon request



Certificate of Analysis

AGAT WORK ORDER: 19T460288 PROJECT: 1894152(2110) 5835 COOPERS AVENUE MISSISSAUGA, ONTARIO CANADA L4Z 1Y2 TEL (905)712-5100 FAX (905)712-5122 http://www.agatlabs.com

CLIENT NAME: GOLDER ASSOCIATES LTD.

SAMPLING SITE:

ATTENTION TO: Devin Hannan

SAMPLED BY:

DATE RECEIVED: 2019-04-25									DATE REPORTED: 2019-05-01
		SAMPLE DESC	RIPTION:	MW18-01	MW18-02	MW18-03	PW	DUP	
		SAMPL	SAMPLE TYPE:		Water 2019-04-24	Water	Water	Water	
		DATE SAMPLED:		2019-04-24		2019-04-24	2019-04-24	2019-04-24	
Parameter	Unit	G / S	RDL	154946	154947	154948	154949	154950	
enzene	µg/L	5.0	0.20	<0.20	<0.20	<0.20	<0.20	<0.20	
oluene	µg/L	24	0.20	<0.20	<0.20	<0.20	<0.20	<0.20	
thylbenzene	µg/L	2.4	0.10	<0.10	<0.10	<0.10	<0.10	<0.10	
ylene Mixture	µg/L	300	0.20	<0.20	<0.20	<0.20	<0.20	<0.20	
1 (C6 - C10)	µg/L	750	25	<25	<25	<25	<25	<25	
1 (C6 to C10) minus BTEX	µg/L	750	25	<25	<25	<25	<25	<25	
2 (C10 to C16)	µg/L	150	100	<100	<100	<100	<100	<100	
3 (C16 to C34)	µg/L	500	100	<100	<100	110	<100	120	
4 (C34 to C50)	µg/L	500	100	<100	<100	<100	<100	<100	
Gravimetric Heavy Hydrocarbons	µg/L	500	500	NA	NA	NA	NA	NA	
Surrogate	Unit	Acceptable	Limits						
erphenyl	%	60-14	0	60	62	112	61	72	
Property Uses - Co	arse Texture	d Soils							Water Condition - Potable Ground Water - All Typ able standard for regulatory interpretation.

O Reg 153(511) - PHCs E1 - E4 (Water)

Xylenes total is a calculated parameter. The calculated value is the sum of m&p-Xylene and o-Xylene.

C6–C10 (F1 minus BTEX) is a calculated parameter. The calculated value is F1 minus BTEX.

The C10 - C16, C16 - C34, and C34 - C50 fractions are calculated using the average response factor for n-C10, n-C16, and nC34.

Gravimetric Heavy Hydrocarbons are not included in the Total C16 - C50 and are only determined if the chromatogram of the C34 - C50 Hydrocarbons indicated that hydrocarbons >C50 are present. The chromatogram has returned to baseline by the retention time of nC50.

Total C6-C50 results are corrected for BTEX contribution.

This method complies with the Reference Method for the CWS PHC and is validated for use in the laboratory.

nC6 and nC10 response factors are within 30% of Toluene response factor.

nC10, nC16 and nC34 response factors are within 10% of their average.

C50 response factor is within 70% of nC10 + nC16 $\,$ nC34 average.

Linearity is within 15%.

Extraction and holding times were met for this sample.

Fractions 1-4 are quantified with the contribution of PAHs. Under Ontario Regulation 153/04, results are considered valid without determining the PAH contribution if not requested by the client. NA = Not Applicable

Analysis performed at AGAT Toronto (unless marked by *)

Certified By:

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5835 COOPERS AVENUE MISSISSAUGA, ONTARIO CANADA L4Z 1Y2 TEL (905)712-5100 FAX (905)712-5122 http://www.agatlabs.com

Quality Assurance

CLIENT NAME: GOLDER ASSOCIATES LTD.

PROJECT: 1894152(2110)

SAMPLING SITE:

AGAT WORK ORDER: 19T460288

ATTENTION TO: Devin Hannan

SAMPLED BY:

Trace Organics Analysis

					0											
RPT Date: May 01, 2019			0	DUPLICATE			REFEREN	NCE MATERIAL		METHOD BLANK SPIKE		MAT	MATRIX SPIKE			
PARAMETER	Batch	Sample	Dup #1	Dup #2	RPD	Method Blank	Measured		cceptable Limits Recover	Recovery	Acceptable Limits		Recovery	1 1 1 1 1	Acceptable Limits	
		ld					Value	Lower	Upper	-	Lower	Upper		Lower	Upper	
O. Reg. 153(511) - PHCs F1 - F	4 (Water)															
Benzene	146693		< 0.20	< 0.20	NA	< 0.20	105%	50%	140%	85%	60%	130%	119%	50%	140%	
Foluene	146693		< 0.20	< 0.20	NA	< 0.20	108%	50%	140%	80%	60%	130%	114%	50%	140%	
Ethylbenzene	146693		< 0.10	< 0.10	NA	< 0.10	104%	50%	140%	75%	60%	130%	106%	50%	140%	
Xylene Mixture	146693		< 0.20	< 0.20	NA	< 0.20	89%	50%	140%	76%	60%	130%	103%	50%	140%	
F1 (C6 - C10)	146693		< 25	< 25	NA	< 25	98%	60%	140%	89%	60%	140%	81%	60%	140%	
F2 (C10 to C16)		тw	< 100	< 100	NA	< 100	94%	60%	140%	72%	60%	140%	62%	60%	140%	
F3 (C16 to C34)		TW	< 100	< 100	NA	< 100	98%	60%	140%	85%	60%	140%	87%	60%	140%	
F4 (C34 to C50)		TW	< 100	< 100	NA	< 100	96%	60%	140%	96%	60%	140%	92%	60%	140%	

Comments: Tap water analysis has been performed as QC sample testing for duplicate and matrix spike due to insufficient sample volume.

When the average of the sample and duplicate results is less than 5x the RDL, the Relative Percent Difference (RPD) will be indicated as Not Applicable (NA).

Certified By:

Imkal Jata

AGAT QUALITY ASSURANCE REPORT (V1)

AGAT Laboratories is accredited to ISO/IEC 17025 by the Canadian Association for Laboratory Accreditation Inc. (CALA) and/or Standards Council of Canada (SCC) for specific tests listed on the scope of accreditation. AGAT Laboratories (Mississauga) is also accredited by the Canadian Association for Laboratory Accreditation Inc. (CALA) for specific tests tests. Accreditations are location and parameter specific. A complete listing of parameters for each location is available from www.cala.ca and/or www.scc.ca. The tests in this report may not necessarily be included in the scope of accreditation. RPDs calculated using raw data. The RPD may not be reflective of duplicate values shown, due to rounding of final results.

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5835 COOPERS AVENUE MISSISSAUGA, ONTARIO CANADA L4Z 1Y2 TEL (905)712-5100 FAX (905)712-5122 http://www.agatlabs.com

Method Summary

CLIENT NAME: GOLDER ASSOCIATES LTD.

PROJECT: 1894152(2110)

SAMPLING SITE:

AGAT WORK ORDER: 19T460288

ATTENTION TO: Devin Hannan

SAMPLING SITE:		SAMPLED BY:	
PARAMETER	AGAT S.O.P	LITERATURE REFERENCE	ANALYTICAL TECHNIQUE
Trace Organics Analysis	·		•
Benzene	VOL-91-5010	MOE PHC-E3421	P&T GC/MS
Toluene	VOL-91-5010	MOE PHC-E3421	P&T GC/MS
Ethylbenzene	VOL-91-5010	MOE PHC-E3421	P&T GC/MS
Xylene Mixture	VOL-91-5010	MOE PHC-E3421	P&T GC/MS
F1 (C6 - C10)	VOL-91- 5010	MOE PHC-E3421	P&T GC/FID
F1 (C6 to C10) minus BTEX	VOL-91-5010	MOE PHC-E3421	P&T GC/FID
F2 (C10 to C16)	VOL-91-5010	MOE PHC-E3421	GC/FID
F3 (C16 to C34)	VOL-91-5010	MOE PHC-E3421	GC/FID
F4 (C34 to C50)	VOL-91-5010	MOE PHC-E3421	GC/FID
Gravimetric Heavy Hydrocarbons	VOL-91-5010	MOE PHC-E3421	BALANCE
Terphenyl	VOL-91-5010	MOE PHC-E3421	GC/FID

		A	La	bora	tor	ries	Ph: 90		ssissau 2,5100	335 Coope ga. Ontario Fax: 905. Dearth.aga	L4Z : 7 12.5 1	22 22	Wo		r #: _	10			028	38	
	ustody Record	lf this is a	DrInking Water	sample, plea		Prinking Water Chain of Custody For	_					1		ival Ten			Son	15	2	420	5
Company:	Golde	r As Hann	nan;	dte:) (Plea	Regulatory Requirements lease check all applicable boxes) Regulation 153/04	i⊑ ſ werUse			cory Req		nent	No	stody S ites:	_		□ Ye		□No		/A
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Phone: 70 Reports to be sent to: 1. Email:	2 hanna	n@ ac	Ider	. CON	- 1	Agriculture	Storm			rov. Water bjectives (ther			Rus	5h TAT 3 E	(Rush S			Busine	ss —	Next Busine	ess
2. Email:	Scornell	6301	der.	com		Coarse	dicate One			Indicate C	ne	-			/S		μD	ays	harges May	Day	
Project Inform Project: Site Location: Sampled By:	189415 Lafarge	2 Ge	10)	60	R	Is this submission for a Record of Site Condition ?		Cer		Auideline are of Ana			F	*TA	is ex	clusive	of wee	ekends a		ush TAT bry holidays T AGAT CPM	-
AGAT Quote #: Invoice Inform Company: Contact: Address: Email:	Please note: If quotation number is		be billed full price fo		В	Ground Water Oil Paint Soil Soil	Field Filtered - Metals, Hg, CrVI	and Inorganics	□ All Metals □ 153 Metals (excl. Hydrides) O □ Hydride Metals □ 153 Metals (Incl. Hydrides) 80	1WS DC: DCN DFOC DHg	Full Metals Scan	Nutrients: DTP DNH, DTKN DNO, DNO, DNO, HO.	: OVOC DBTEX DTHM	F1 - F4		Total 🗆 Aroclors	Organochlorine Pesticides TCLP: 🗆 M&I 🗆 VOCs 🗆 ABNs 🗆 B(a)P 🔤 CBs				
Sample	eldentification	Date Sampled	Time Sampled	Containers	Sample Matrix	Comments/ Special Instructions	Y/N	Metals a	All Met	ORPs: CB- CC ⁶⁺ CEC CpH CSAR	Full Met	Nutrient	Volatiles:	PHCs F3 ABNs	PAHs	PCBs: []	Organoo	Sewer Use			
MW MW PM DU	18-01 18-02 18-03 18-03	04/24/19	11:40 12:15 1:00 1:30 1:00	JJJJJJ-J-	GW		V														
Samples Relinquistied By (Prin	It Name and Sign	Comple	1 Date / Li	1./\a	:.00 P	Schoples Received By (Print Name and Ege	P					Date	176	10 TIME	71-	25					_
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78 15/1 015				1						Pink C		ient I Y	ellow (Time Copy - A	GAT I	White	Nº:	U8		9 e ^{rd March 16, 20:}	18



Your Project #: 21453907 Site Location: GOODWOOL Your C.O.C. #: 827225-01-01

Attention: Dawn Hoyle

Golder Associates Ltd 121 Commerce Park Drive Unit L Barrie, ON CANADA L4N 8X1

> Report Date: 2021/05/28 Report #: R6651649 Version: 1 - Final

CERTIFICATE OF ANALYSIS

BV LABS JOB #: C1D2546

Received: 2021/05/17, 14:44

Sample Matrix: Water # Samples Received: 6

		Date	Date		
Analyses	Quantity	Extracted	Analyzed	Laboratory Method	Analytical Method
Petroleum Hydro. CCME F1 & BTEX in Water	6	N/A	2021/05/20	CAM SOP-00315	CCME PHC-CWS m
Petroleum Hydrocarbons F2-F4 in Water (1)	5	2021/05/19	2021/05/20	CAM SOP-00316	CCME PHC-CWS m

Remarks:

Bureau Veritas is accredited to ISO/IEC 17025 for specific parameters on scopes of accreditation. Unless otherwise noted, procedures used by Bureau Veritas are based upon recognized Provincial, Federal or US method compendia such as CCME, MELCC, EPA, APHA.

All work recorded herein has been done in accordance with procedures and practices ordinarily exercised by professionals in Bureau Veritas' profession using accepted testing methodologies, quality assurance and quality control procedures (except where otherwise agreed by the client and Bureau Veritas in writing). All data is in statistical control and has met quality control and method performance criteria unless otherwise noted. All method blanks are reported; unless indicated otherwise, associated sample data are not blank corrected. Where applicable, unless otherwise noted, Measurement Uncertainty has not been accounted for when stating conformity to the referenced standard.

Bureau Veritas liability is limited to the actual cost of the requested analyses, unless otherwise agreed in writing. There is no other warranty expressed or implied. Bureau Veritas has been retained to provide analysis of samples provided by the Client using the testing methodology referenced in this report. Interpretation and use of test results are the sole responsibility of the Client and are not within the scope of services provided by Bureau Veritas, unless otherwise agreed in writing. Bureau Veritas is not responsible for the accuracy or any data impacts, that result from the information provided by the customer or their agent.

Solid sample results, except biota, are based on dry weight unless otherwise indicated. Organic analyses are not recovery corrected except for isotope dilution methods.

Results relate to samples tested. When sampling is not conducted by Bureau Veritas, results relate to the supplied samples tested.

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Reference Method suffix "m" indicates test methods incorporate validated modifications from specific reference methods to improve performance.

* RPDs calculated using raw data. The rounding of final results may result in the apparent difference.

(1) All CCME PHC results met required criteria unless otherwise stated in the report. The CWS PHC methods employed by Bureau Veritas Laboratories conform to all prescribed elements of the reference method and performance based elements have been validated. All modifications have been validated and proven equivalent following "Alberta Environment's Interpretation of the Reference Method for the Canada-Wide Standard for Petroleum Hydrocarbons in Soil Validation of Performance-Based Alternative Methods September 2003". Documentation is available upon request. Modifications from Reference Method for the Canada-wide Standard for Petroleum Hydrocarbons in Soil-Tier 1 Method: F2/F3/F4 data reported using validated cold solvent extraction instead of Soxhlet extraction.

Page 1 of 8

Bureau Veritas Laboratories 6740 Campobello Road, Mississauga, Ontario, LSN 2L8 Tel: (905) 817-5700 Toll-Free: 800-563-6266 Fax: (905) 817-5777 www.bvlabs.com



Your Project #: 21453907 Site Location: GOODWOOL Your C.O.C. #: 827225-01-01

Attention: Dawn Hoyle

Golder Associates Ltd 121 Commerce Park Drive Unit L Barrie, ON CANADA L4N 8X1

> Report Date: 2021/05/28 Report #: R6651649 Version: 1 - Final

CERTIFICATE OF ANALYSIS

BV LABS JOB #: C1D2546 Received: 2021/05/17, 14:44

Encryption Key

Please direct all questions regarding this Certificate of Analysis to your Project Manager. Ema Gitej, Senior Project Manager Email: emese.gitej@bureauveritas.com Phone# (905)817-5829

This report has been generated and distributed using a secure automated process.

BV Labs has procedures in place to guard against improper use of the electronic signature and have the required "signatories", as per ISO/IEC 17025, signing the reports. For Service Group specific validation please refer to the Validation Signature Page.



PETROLEUM HYDROCARBONS (CCME)

BV Labs ID		POX994					
Sampling Date		2021/05/17 08:00					
COC Number		827225-01-01					
	UNITS	TRIP BLANK	RDL	QC Batch			
BTEX & F1 Hydrocarbons							
F1 (C6-C10)	ug/L	<25	25	7361905			
F1 (C6-C10) - BTEX	ug/L	<25	25	7361905			
Surrogate Recovery (%)							
1,4-Difluorobenzene	%	105		7361905			
4-Bromofluorobenzene	%	87		7361905			
D10-o-Xylene	%	102		7361905			
D4-1,2-Dichloroethane	%	103		7361905			
RDL = Reportable Detection Limit							
de baten – quaity control ba	QC Batch = Quality Control Batch						

Page 3 of 8 Bureau Veritas Laboratories 6740 Campobello Road, Mississauga, Ontario, LSN 2L8 Tel: (905) 817-5700 Toll-Free: 800-563-6266 Fax: (905) 817-5777 www.bvlabs.com



O.REG 153 PHCS, BTEX/F1-F4 (WATER)

BV Labs ID		POX989	POX990	POX991	POX992	POX993		
Compling Data		2021/05/17	2021/05/17	2021/05/17	2021/05/17	2021/05/17		
Sampling Date		09:25	09:25	10:25	11:30	12:15		
COC Number		827225-01-01	827225-01-01	827225-01-01	827225-01-01	827225-01-01		
	UNITS	MW18-03	DUPE-1	PW	MW18-02	MW18-01	RDL	QC Batch
BTEX & F1 Hydrocarbons								
F1 (C6-C10)	ug/L	<25	<25	<25	<25	<25	25	7361905
F1 (C6-C10) - BTEX	ug/L	<25	<25	<25	<25	<25	25	7361905
F2-F4 Hydrocarbons	•		•	•		•		
F2 (C10-C16 Hydrocarbons)	ug/L	<100	<100	<100	<100	<100	100	7360004
F3 (C16-C34 Hydrocarbons)	ug/L	<200	<200	<200	<200	<200	200	7360004
F4 (C34-C50 Hydrocarbons)	ug/L	<200	<200	<200	<200	<200	200	7360004
Reached Baseline at C50	ug/L	Yes	Yes	Yes	Yes	Yes		7360004
Surrogate Recovery (%)								
1,4-Difluorobenzene	%	103	102	104	103	103		7361905
4-Bromofluorobenzene	%	95	81	90	93	95		7361905
D10-o-Xylene	%	99	105	105	99	106		7361905
D4-1,2-Dichloroethane	%	105	108	104	106	105		7361905
o-Terphenyl	%	81	88	87	89	88		7360004
RDL = Reportable Detection L	.imit		1	1		1		
QC Batch = Quality Control B								



TEST SUMMARY

BV Labs ID: POX989 Sample ID: MW18-03 Matrix: Water					Collected: Shipped: Received:	2021/05/17 2021/05/17
Test Description	Instrumentation	Batch	Extracted	Date Analyzed	Analyst	
Petroleum Hydro. CCME F1 & BTEX in Water	HSGC/MSFD	7361905	N/A	2021/05/20	Abdikarim	Ali
Petroleum Hydrocarbons F2-F4 in Water	GC/FID	7360004	2021/05/19	2021/05/20	Ravinder G	aidhu
BV Labs ID: POX990 Sample ID: DUPE-1 Matrix: Water					Collected: Shipped: Received:	2021/05/17 2021/05/17
Test Description	Instrumentation	Batch	Extracted	Date Analyzed	Analyst	
Petroleum Hydro. CCME F1 & BTEX in Water	HSGC/MSFD	7361905	N/A	2021/05/20	Abdikarim	Ali
Petroleum Hydrocarbons F2-F4 in Water	GC/FID	7360004	2021/05/19	2021/05/20	Ravinder G	aidhu
BV Labs ID: POX991 Sample ID: PW Matrix: Water					Collected: Shipped: Received:	2021/05/17 2021/05/17
Test Description	Instrumentation	Batch	Extracted	Date Analyzed	Analyst	
Petroleum Hydro. CCME F1 & BTEX in Water	HSGC/MSFD	7361905	N/A	2021/05/20	Abdikarim	Ali
Petroleum Hydrocarbons F2-F4 in Water	GC/FID	7360004	2021/05/19	2021/05/20	Ravinder G	aidhu
BV Labs ID: POX992 Sample ID: MW18-02 Matrix: Water					Collected: Shipped: Received:	2021/05/17 2021/05/17
Test Description	Instrumentation	Batch	Extracted	Date Analyzed	Analyst	
Petroleum Hydro. CCME F1 & BTEX in Water	HSGC/MSFD	7361905	N/A	2021/05/20	Abdikarim	Ali
Petroleum Hydrocarbons F2-F4 in Water	GC/FID	7360004	2021/05/19	2021/05/20	Ravinder G	aidhu
BV Labs ID: POX993 Sample ID: MW18-01 Matrix: Water					Collected: Shipped: Received:	2021/05/17 2021/05/17
Test Description	Instrumentation	Batch	Extracted	Date Analyzed	Analyst	
Petroleum Hydro. CCME F1 & BTEX in Water	HSGC/MSFD	7361905	N/A	2021/05/20	Abdikarim	Ali
Petroleum Hydrocarbons F2-F4 in Water	GC/FID	7360004	2021/05/19	2021/05/20	Ravinder G	aidhu
BV Labs ID: POX994 Sample ID: TRIP BLANK Matrix: Water					Collected: Shipped: Received:	2021/05/17 2021/05/17
Sample ID: TRIP BLANK	Instrumentation	Batch	Extracted	Date Analyzed	Shipped:	

Bureau Veritas Laboratories 6740 Campobello Road, Mississauga, Ontario, LSN 2L8 Tel: (905) 817-5700 Toll-Free: 800-563-6266 Fax: (905) 817-5777 www.bvlabs.com

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GENERAL COMMENTS

Each te	emperature is the	average of up to t	hree cooler temperatures taken at receipt				
1	Package 1	11.0°C					
Sample	POX994 [TRIP BI	ANK] :Sample an	alyzed for F1/BTEX as per client request.				
Results relate only to the items tested.							

Page 6 of 8 Bureau Veritas Laboratories 6740 Campobello Road, Mississauga, Ontario, L5N 2L8 Tel: (905) 817-5700 Toll-Free: 800-563-6266 Fax: (905) 817-5777 www.bvlabs.com



QUALITY ASSURANCE REPORT

QA/QC								
Batch	Init	QC Type	Parameter	Date Analyzed	Value	Recovery	UNITS	QC Limits
7360004	RGA	Matrix Spike	o-Terphenyl	2021/05/19		91	%	60 - 130
			F2 (C10-C16 Hydrocarbons)	2021/05/19		98	%	60 - 130
			F3 (C16-C34 Hydrocarbons)	2021/05/19		95	%	60 - 130
			F4 (C34-C50 Hydrocarbons)	2021/05/19		97	%	60 - 130
7360004	RGA	Spiked Blank	o-Terphenyl	2021/05/19		91	%	60 - 130
			F2 (C10-C16 Hydrocarbons)	2021/05/19		101	%	60 - 130
			F3 (C16-C34 Hydrocarbons)	2021/05/19		99	%	60 - 130
			F4 (C34-C50 Hydrocarbons)	2021/05/19		100	%	60 - 130
7360004	RGA	Method Blank	o-Terphenyl	2021/05/19		89	%	60 - 130
			F2 (C10-C16 Hydrocarbons)	2021/05/19	<100		ug/L	
			F3 (C16-C34 Hydrocarbons)	2021/05/19	<200		ug/L	
			F4 (C34-C50 Hydrocarbons)	2021/05/19	<200		ug/L	
7360004	RGA	RPD	F2 (C10-C16 Hydrocarbons)	2021/05/19	NC		%	30
			F3 (C16-C34 Hydrocarbons)	2021/05/19	NC		%	30
			F4 (C34-C50 Hydrocarbons)	2021/05/19	NC		%	30
7361905	AAI	Matrix Spike	1,4-Difluorobenzene	2021/05/19		100	%	70 - 130
			4-Bromofluorobenzene	2021/05/19		99	%	70 - 130
			D10-o-Xylene	2021/05/19		102	%	70 - 130
			D4-1,2-Dichloroethane	2021/05/19		101	%	70 - 130
			F1 (C6-C10)	2021/05/19		82	%	60 - 140
7361905	AAI	Spiked Blank	1,4-Difluorobenzene	2021/05/19		101	%	70 - 130
			4-Bromofluorobenzene	2021/05/19		98	%	70 - 130
			D10-o-Xylene	2021/05/19		103	%	70 - 130
			D4-1,2-Dichloroethane	2021/05/19		99	%	70 - 130
			F1 (C6-C10)	2021/05/19		82	%	60 - 140
7361905	AAI	Method Blank	1,4-Difluorobenzene	2021/05/19		103	%	70 - 130
			4-Bromofluorobenzene	2021/05/19		91	%	70 - 130
			D10-o-Xylene	2021/05/19		99	%	70 - 130
			D4-1,2-Dichloroethane	2021/05/19		105	%	70 - 130
			F1 (C6-C10)	2021/05/19	<25		ug/L	
			F1 (C6-C10) - BTEX	2021/05/19	<25		ug/L	
7361905	AAI	RPD	F1 (C6-C10)	2021/05/19	NC		%	30
			F1 (C6-C10) - BTEX	2021/05/19	NC		%	30

Duplicate: Paired analysis of a separate portion of the same sample. Used to evaluate the variance in the measurement.

Matrix Spike: A sample to which a known amount of the analyte of interest has been added. Used to evaluate sample matrix interference.

Spiked Blank: A blank matrix sample to which a known amount of the analyte, usually from a second source, has been added. Used to evaluate method accuracy.

Method Blank: A blank matrix containing all reagents used in the analytical procedure. Used to identify laboratory contamination.

Surrogate: A pure or isotopically labeled compound whose behavior mirrors the analytes of interest. Used to evaluate extraction efficiency.

NC (Duplicate RPD): The duplicate RPD was not calculated. The concentration in the sample and/or duplicate was too low to permit a reliable RPD calculation (absolute difference <= 2x RDL).



VALIDATION SIGNATURE PAGE

The analytical data and all QC contained in this report were reviewed and validated by:

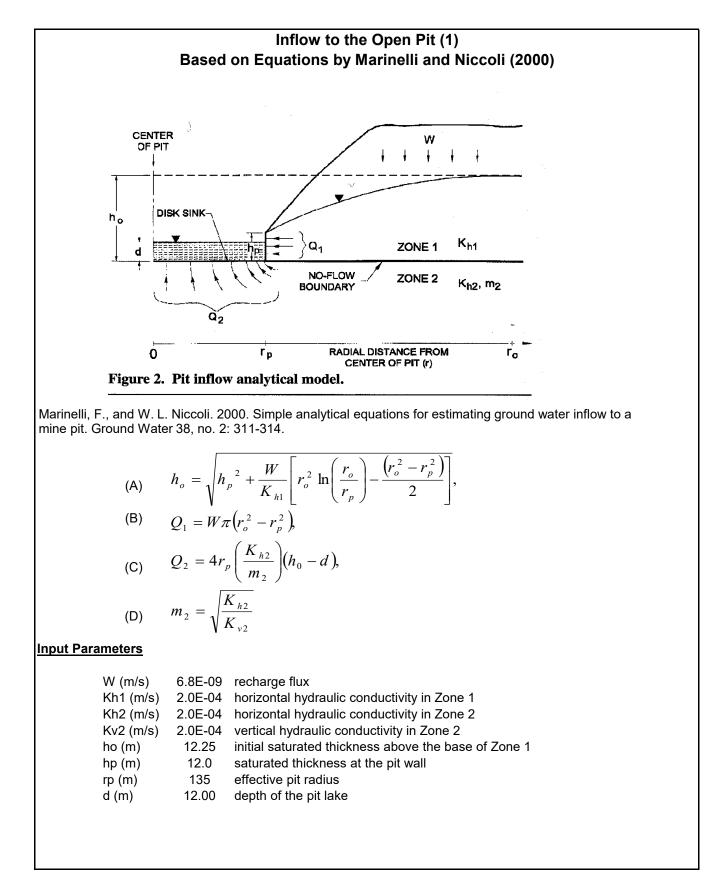


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BV Labs has procedures in place to guard against improper use of the electronic signature and have the required "signatories", as per ISO/IEC 17025, signing the reports. For Service Group specific validation please refer to the Validation Signature Page.

APPENDIX E

Groundwater Zone of Influence Calculations



Inflow to the Open Pit (2) Based on Equations by Marinelli and Niccoli (2000)

Inflow from Zone 1

ro (m)	474.0	radius of influence calculated by iterating equation A / goalseek
(known ho)	(ho ca	alculated using eq. A)
12.2500	=	12.251

Drawdown with Distance Based on Equations by Marinelli and Niccoli (2000)

r (m)	=	450.0	selected distance from pit centre
H (m) at r	=	12.25	head at selected distance from pit centre
Drawdown (m) at r	=	0.001	drawdown at selected distance from pit centre

