



REPORT

Water Report Level 2

Lafarge Goodwood Pit Extension

Submitted to:

Lafarge Canada Inc.

6509 Airport Road
Mississauga, ON
L4V 1S7

Submitted by:

WSP Canada Inc.

121 Commerce Park Drive, Unit L, Barrie, Ontario, L4N 8X1, Canada

+1 705 722 4492

21453907

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Table of Contents

- 1.0 INTRODUCTION 1**
 - 1.1 Policy Considerations..... 1
- 2.0 PROPOSED DEVELOPMENT 2**
 - 2.1 Operational Scenario 2
 - 2.2 Rehabilitated Scenario 3
- 3.0 PHYSICAL SETTING 3**
 - 3.1 Climate 3
 - 3.2 Existing Land Use 3
 - 3.3 Topography and Drainage 3
 - 3.4 Geology and Hydrostratigraphy 4
 - 3.5 Regional Recharge and Groundwater Flow 4
 - 3.6 Water Users 4
 - 3.7 Source Water Protection and Regulated Area Considerations..... 4
- 4.0 FIELD PROGRAM 5**
 - 4.1 Borehole Drilling 5
 - 4.2 Water Level Measurements 5
 - 4.3 Hydraulic Conductivity..... 8
 - 4.4 Water Quality..... 8
- 5.0 WATER BUDGET 9**
 - 5.1 Approach 10
 - 5.2 Catchment Areas 11
 - 5.2.1 Existing Scenario 11
 - 5.2.2 Rehabilitated Scenario..... 12
 - 5.3 Results 12
 - 5.3.1 Existing Scenario 12
 - 5.3.2 Rehabilitated Scenario..... 14

6.0 IMPACT ASSESSMENT 16

6.1 Groundwater Levels 16

6.2 Baseflow 17

6.3 Well Water Quantities 17

6.3.1 Water Well Compliant Action Plan 18

6.4 Water Quality 19

6.5 Water Temperature 19

6.6 Site Water Budget 19

7.0 CONCLUSIONS 20

7.1 Existing Scenario 20

7.2 Operational and Rehabilitated Scenarios 21

8.0 RECOMMENDATIONS 21

9.0 REFERENCES 23

TABLES

Table 1: Monitoring Well Summary and Groundwater Levels 6

Table 2: Land Use Water Budget Input Data 10

Table 3: Existing Scenario Land Use 11

Table 4: Rehabilitated Scenario Land Use 12

Table 5: Existing Scenario Results 13

Table 6: Rehabilitated Scenario Results 15

Table 7: Water Well Records In Drawdown Zone of Influence 18

FIGURES

- Figure 1: Site and Surrounds
- Figure 2: Site Detail
- Figure 3: Site Cross-Section
- Figure 4: Water Users
- Figure 5: Groundwater Hydrographs

Figure 6: Inferred Water Table

Figure 7: Existing Scenario Catchment Areas

Figure 8: Rehabilitated Scenario Catchment Areas

Figure 9: Pit Pond Zone of Influence

APPENDICES

APPENDIX A

Draft Site Plans

APPENDIX B

Borehole Logs

APPENDIX C

Grain Size Data

APPENDIX D

Water Quality Results

APPENDIX E

Groundwater Zone of Influence Calculations

1.0 INTRODUCTION

Golder Associates Ltd. (Golder) was retained by Lafarge Canada Inc. (Lafarge) to prepare a Water Report Level 2 in support of Class A Pit Below Water aggregate licensing application for the proposed Goodwood Pit Extension (the Site). The Site is located at 4900 4th Concession Road in the Township of Uxbridge, Regional Municipality of Durham (Figure 1). The Site is located immediately north of the existing Lafarge Goodwood Pit and is intended to be an extension of that active operation.

The objectives of this study are to 1) characterize the existing hydrogeological and hydrological conditions in the vicinity of the Site; and 2) assess the potential impacts, if any, of the proposed below water extraction on groundwater and surface water resources.

The Study considers three Site conditions:

- 1) Existing Scenario;
- 2) Operational Scenario; and
- 3) Rehabilitated Scenario.

1.1 Policy Considerations

This Study has been prepared to address the requirements of:

- Aggregate Resources of Ontario Standards: A compilation of the four standards adopted by Ontario Regulation 244/97 under the Aggregate Resources Act (August 2020). Pertinent considerations for this report include:
 - A description of the physical setting including local geology, hydrogeology, and surface water systems;
 - “Maximum predicted water table”;
 - Potential effects of the operation on any ground water and surface water features located within the zone of influence, including but not limited to: water wells; springs; aquifers; and surface water; and
 - Monitoring plans.
- The Growth Plan for the Greater Gold Horseshoe (2017);
- Lake Simcoe Protection Plan (2009);
- South Georgian Bay Lake Simcoe Source Protection Plan (Approved 2015, Amended 2018);
- The Provincial Policy Statement (2014);
- Township of Uxbridge Official Plan (Office Consolidation January 2014);
- Durham Regional Official Plan (Consolidation May 2017); and
- Oak Ridges Moraine Conservation Plan (2002).

Key hydrogeologic/hydrologic considerations set out by the above policies include:

- Water resources will be protected, maintained, and, where applicable, enhanced and there will be no unacceptable impacts;
- Identify an appropriate monitoring program to protect water resources;
- Proposed contingency and mitigative measures that will be implemented if unforeseen impacts occur;
- Minimize potential negative impacts, including cross-subwatershed impacts, and identify surface water and groundwater features;
- Ensure municipal drinking water supply and designated vulnerable areas are protected;
- Protect vulnerable surface water and groundwater sensitive features and their hydrogeologic/hydrologic functions;
- Maintain linkages and related functions between surface water features and groundwater features;
- Promote efficient and sustainable use of water resources, including practices for water conservation sustaining water quality;
- Describe how the connectivity between key hydrogeologic/hydrologic features will be maintained before, during and after extraction;
- Describe how private and agricultural water supplies will be protected;
-
- Demonstrate no negative impact to groundwater recharge and discharge;
- Describe measures to protect water resources from contamination from on-Site equipment; and
- Ensure there are no adverse thermal impacts to sensitive nearby water features.

2.0 PROPOSED DEVELOPMENT

A brief overview of the Operations and Rehabilitated Scenarios is provided below. are included in Appendix A.

2.1 Operational Scenario

The proposed licence area is approximately 17.9 hectares (ha) with an extraction area limit of 15.4 ha (Figure 2). Approximately 5.7 ha will be below water. Setbacks are as follows: 15 m along the north boundary, a 30 m setback along the eastern boundary, and no setback along the south and west boundary where the Site is adjacent to the existing Goodwood Pit. The pit lake depth will reach a minimum elevation of approximately 310 masl, corresponding to roughly 38 m to 32 m below current ground surface.

Annual tonnage from the Goodwood Extension will not exceed the current limit of 1,177,000 tonnes per year at the existing Goodwood Pit. Extraction will begin in the Phase 1 area to the west and progress easterly to Phase 2. Operations will be serviced by loaders, a screening plant, and a portable crushing plant. Future extraction of aggregate below the water table may utilize a dragline method. Extracted material would be temporarily stockpiled near the face of the extraction with pore water allowed to drain back into the subsurface.

The Site Operations will not require any pumping or active dewatering.

No fuel will be stored on-Site.

2.2 Rehabilitated Scenario

The proposed Rehabilitated Scenario will consist of a 5.7 ha centrally located pond with 3:1 re-vegetated side slopes reaching the Site setbacks. The Site will be separated from the original (i.e. existing) Goodwood Pit via a small topographic divide along its southern perimeter. The future pond water elevation is estimated to be approximately 321 to 322 metres above sea level (masl), as described further below.

3.0 PHYSICAL SETTING

The following subsections provide a general overview of the Site and surrounds physical setting under the Existing Scenario.

3.1 Climate

The Site is located approximately 22 kilometres (km) west of the Environment Canada (EC) Port Perry climate station. The Port Perry period of record spans 21 years (1984 – 2005) and is a reasonably proximal dataset to characterize average climatological conditions in the vicinity of the Site.

Based on the Port Perry station data, average annual precipitation is 874 millimetres per year (mm/yr) and the average annual temperature is 6 degrees Celsius. Based on Site land use, the evapotranspiration per EC data is 558 mm/yr with a resulting surplus of 316 mm/yr.

3.2 Existing Land Use

Site land use is largely cropland with the approximate eastern third being used as an equestrian facility. Several barn structures are located centrally within the Site. The Site perimeter and a minor portion of its interior is tree plantation.

North of the Site lies the Canadian National Railway (CNR) corridor; beyond that are large estate properties or farmland on Wagg Road (Figure 1). South and west of the Site is the existing Lafarge Goodwood Pit. Several privately owned residential properties are located beyond the southeast corner of the Site on the west side of Concession Road 4 and immediately east of Concession Road 4. Part Lot 20, at the southeast corner of Wagg Road and Concession Road 4, is licensed by S.A.S.E. Aggregates for a Class A Pit Above Water aggregate extraction.

3.3 Topography and Drainage

The Site lies just north of the topographic crest of the Oak Ridges Moraine (ORM), a major physiographic landform in the region (Figure 1). A ground elevation high of approximately 348 masl exists in the southwest corner of the Site whereas a low of approximately 342 masl occurs in the northeast portion of the Site (Figure 2).

There are no permanent surface water features on-Site. According to the property tenant, who has occupied the land since 2001, flowing surface water (i.e. runoff) is typically observed only during the spring melt. During this period, the runoff either: 1) ponds within localized depressions and infiltrates; or 2) exits the Site via topographic lows at the north, east and south of the Site (Figure 2). Additional details regarding local drainage patterns are described Section 5 Water Budget.

Regionally, the Site is located within the Pefferlaw River subwatershed, which itself lies within the Lake Simcoe watershed. The only naturally occurring surface water features mapped within 1 km of the Site are two small

wetland areas located to the northeast along the rail corridor (described herein as “Wetland 1” and Wetland 2”) and a third wetland to the northwest (“Wetland 3”) (Figure 1). None of these wetlands have been evaluated as Provincially Significant. Previous studies have indicated that Wetlands 1 and 2 are perched above the water table (Harden, 2014), a conclusion that aligns with our assessment of aquifer groundwater levels in the area. The same is likely true for Wetland 3, given that proximal well records (MECP, 20211) list water table depths greater than 20 m below ground surface in this area. Based on our review of local drainage patterns, Wetlands 1 and 2 have no hydrologic relationship to the Site. However, Wetland 3 has some relevance to the Study as described later in Section 5 Water Budget.

3.4 Geology and Hydrostratigraphy

The Site aggregate resource is located within the Oak Ridges Moraine Aquifer Complex (ORAC). The ORAC is generally comprised of sands and gravels with localized deposits of finer grained materials. Regional studies suggest that the ORAC is over 50 m thick in this area (Earthfx, 2006). The ORAC is underlain by the Newmarket Till, a thick and regionally extensive aquitard. Figure 3 provides a geologic / hydrostratigraphic cross-section based on Site borehole logs and Ministry of Environment, Conservation and Parks (MECP) Water Well Information System (WWIS) database well records (MECP, 2021¹).

3.5 Regional Recharge and Groundwater Flow

Regional groundwater modelling undertaken for the Regional Municipality of Durham suggests that recharge rates in the vicinity of the Site are on the order of 200 mm/yr (Earthfx, 2010).

Based on the same study, the Site is inferred to be just downgradient of a regional groundwater divide within the ORAC, with groundwater flowing in a generally northwards direction through the Site at groundwater elevations in the +/- 320 masl range (Earthfx, 2010).

3.6 Water Users

The MECP WWIS indicates that there are 31 water wells within 500 m of the Site (Figure 4). The wells are drilled to depths ranging from 28 mbgs to 57 mbgs, with the exception of one observation well which has a depth of 9 mbgs. The majority of the wells are completed within sand/gravel units which are presumed to be part of the ORAC. The primary use for most wells is classed as “Domestic” (27 wells); the remaining wells are “Livestock” (1 well), “Observation Wells” (1 well), and “Unknown” (2 wells). A domestic supply well, Well ID 1909612, is located on the property.

According the MECP Permit To Take Water database (MECP, 2021²), the closest large water user in the area is Wyndance Golf Club (ClubLink Corporation), with the water source mapped about 1.6 km southeast of the Site. The Wyndance Golf Club has a permit (2872-A8YKPF) to withdraw up to 4,900 m³/d of groundwater for irrigation and communal supply purposes.

3.7 Source Water Protection and Regulated Area Considerations

The Site’s location within the South Georgian Bay Lake Simcoe Source Protection Region is examined as part of this Study. Using the Lake Simcoe Region Conservation Authority (LSRCA) interactive mapping tool (accessed online August 2019), the Site’s address was input into the Vulnerable Area Search query with the following result:

“This property is not in a vulnerable area. Policies found in the South Georgian Bay Lake Simcoe (SGBLS) Source Protection Plan do not apply.”

The closest source water protection vulnerable area is a community of Uxbridge municipal Wellhead Protection Area (WHPA), which lies over 6 km northeast of the Site and is thus not a concern in this Study.

In addition, the Site lies external to any LSRCA Regulated Area drainages as governed under O. Reg. 179/06.

4.0 FIELD PROGRAM

A Site field program was initiated in 2018 with the objectives of characterizing hydrogeologic conditions at the Site, including: geologic units, water levels, hydraulic conductivity and water quality. The following subsections describe the methodology and results of the field program in detail.

4.1 Borehole Drilling

The Site monitoring network includes three monitoring wells (MW18-01, MW18-02, and MW18-03) as shown on Figure 2. Note that the on-Site domestic well cannot be readily monitored as the well is shut-in with live electrical. A monitoring well summary is provided in Table 1; detailed borehole logs are provided in Appendix B. The following is noted:

Well Location. The monitoring wells are strategically placed near the Site corners to establish Site-wide water level patterns. The well locations and elevations were surveyed by a professional land surveyor.

Construction. The wells were constructed in April 2018. Boreholes were advanced by Choice Sonic Drilling using a SDC 550 track mounted Sonic drill rig. Core barrel dimensions were 165 mm outer diameter / 114 mm inner diameter. Borehole depths ranged from 36.9 metres below ground surface (mbgs) to 38.1 mbgs. The boreholes were completed as monitoring wells using 50.8 mm diameter Schedule 40 PVC pipe with approximately 6.1 m lengths of 10-slot screens positioned within the ORAC / saturated target resource. The pipe stick ups were enclosed within a protective steel casing cemented into the ground and locked. The wells were developed upon installation.

Geology and Hydrostratigraphy. A cross-section illustrating Site geology is provided in Figure 3. In general, the subsurface conditions from hole to hole primarily consists of sand to sand and gravel with the exception of shallow (<3 mbgs) topsoil samples, some of which are observed to contain silty or clayey components. The borehole logs support the conceptual hydrostratigraphy of an unconfined sand and gravel aquifer (ORAC).

4.2 Water Level Measurements

Groundwater level monitoring at the Site began in May 2018 with monthly monitoring events occurring thereafter. All water level monitoring is conducted manually using a water level probe. Water levels are listed in Table 1 whereas groundwater hydrographs are illustrated on Figure 5. Lastly, an inferred high water table map for the unconfined sand and gravel aquifer is provided in Figure 6. The following observations are noted:

- Depth to water ranges from 23.65 mbgs to 26.77 mbgs depending on the well and the time of year. (Table 1). Correspondingly, groundwater elevations range from 319.96 masl to 322.25 masl with a Site-wide average of approximately 321 masl. The groundwater elevations are approximately 11 m to 12 m above the proposed final extraction elevation of 310 masl.
- Groundwater levels at each well fluctuate between approximately +/- 0.4 m during the period of record (Figure 5). Water levels remain fairly stable during May 2018 to March 2019 whereupon they somewhat steadily rise into the summer of 2020 with the highest recorded water levels occurring in July - August 2020. Thereafter water levels being to decline for the remainder of 2020 and into 2021 and 2022. This water table rise/fall

trend, spanning the course of several years, is perhaps less common than the shorter annual rise and fall often observed in other areas of Ontario. It is speculated that this behaviour is, at least in part, attributable to the relatively thick unsaturated zone providing a moderated response to surficial infiltration.

- An inferred high-water table map is developed using water levels measured during the July 31, 2020 event (Figure 6). Consistent with other monitoring events, the on-Site flow pattern during this period is from roughly south to north / northeast. The highest water level occurs to the southeast at MW18-02 (322.25 masl) and the lowest water level occurs to the northeast at MW18-01 (320.97 masl). Under these circumstances the flow gradient across the Site may reach up to 4E-3 m/m, or 0.4%.

Table 1: Monitoring Well Summary and Groundwater Levels

Well ID	MW18-01		MW18-02		MW18-03	
	645,890	4,879,758	645,868	4,879,445	645,315	4,879,385
Ground Elev (masl)	343.74		346.43		346.63	
Pipe Elev (masl)	344.62		347.34		347.47	
Well Depth (mbgs)	36.88		38.10		36.88	
Date	Water Level (mbgs)	Groundwater Elev (masl)	Water Level (mbgs)	Groundwater Elev (masl)	Water Level (mbgs)	Groundwater Elev (masl)
22-May-18	24.44	320.18	25.89	321.45	26.12	321.36
27-Jun-18	24.39	320.23	25.85	321.49	26.10	321.38
17-Jul-18	24.41	320.21	25.86	321.48	26.13	321.34
10-Aug-18	24.42	320.20	25.85	321.49	26.12	321.35
17-Sep-18	24.38	320.24	25.79	321.55	26.07	321.41
22-Oct-18	24.44	320.19	25.84	321.50	26.14	321.33
29-Nov-18	24.48	320.14	25.88	321.46	26.16	321.31
20-Dec-18	24.41	320.22	25.83	321.51	26.07	321.40
07-Feb-19	24.43	320.20	25.84	321.50	26.12	321.36
25-Mar-19	24.40	320.22	25.86	321.48	26.17	321.31
23-Apr-19	24.26	320.36	25.73	321.61	25.99	321.49
14-May-19	24.28	320.34	25.73	321.61	25.98	321.50
10-Jun-19	24.15	320.47	25.61	321.73	25.85	321.62
18-Jul-19	24.14	320.48	25.57	321.77	25.86	321.61
16-Aug-19	24.03	320.59	25.46	321.89	25.76	321.72
13-Sep-19	24.02	320.6	25.43	321.91	25.73	321.74

Well ID	MW18-01		MW18-02		MW18-03	
16-Oct-19	23.95	320.67	25.38	321.96	25.62	321.85
28-Nov-19	24.10	320.524	25.48	321.86	25.78	321.694
23-Dec-19	23.99	320.635	25.39	321.96	25.65	321.82
10-Jan-20	24.02	320.6	25.42	321.92	25.69	321.78
26-Feb-20	23.88	320.74	25.28	322.06	25.58	321.89
17-Mar-20	23.87	320.75	25.26	322.08	25.52	321.95
31-Jul-20	23.65	320.97	25.09	322.25	25.42	322.05
26-Aug-20	23.66	320.96	25.09	322.25	25.42	322.06
29-Sep-20	23.69	320.94	25.12	322.22	25.42	322.05
29-Oct-20	23.75	320.88	25.19	322.15	25.47	322.00
26-Nov-20	23.74	320.88	25.19	322.15	25.45	322.03
23-Dec-20	23.81	320.81	25.24	322.10	25.56	321.91
26-Feb-21	23.94	320.68	25.38	321.97	25.73	321.74
31-Mar-21	23.96	320.66	25.40	321.94	25.68	321.79
30-Apr-21	24.00	320.624	25.45	321.90	25.71	321.757
17-May-21	24.036	320.584	25.53	321.81	25.79	321.68
25-Jun-21	24.09	320.53	25.53	321.81	25.91	321.56
31-Aug-21	24.18	320.44	25.63	321.71	26.1	321.37
27-Sep-21	24.2	320.42	25.6	321.74	26.1	321.37
28-Oct-21	24.22	320.4	25.66	321.68	26.16	321.31
29-Nov-21	24.36	320.26	25.795	321.55	26.16	321.31
22-Dec-21	24.25	320.37	25.8	321.54	26.1	321.37
29-Sep-22	24.59	320.03	26.03	321.31	26.77	320.7
26-Oct-22	24.57	320.05	26.05	321.29	26.38	321.09
29-Nov-22	24.67	319.96	26.13	321.22	26.52	320.95
16-Dec-22	24.64	319.98	26.12	321.22	26.45	321.02

4.3 Hydraulic Conductivity

Hydraulic conductivity is a parameter that quantifies the ease with which water may travel through soil. The hydraulic conductivity of coarser-grained material, such as that found on-Site, may be estimated from laboratory derived grain size distribution curves using the commonly employed Hazen Method as follows (Fetter, 2001):

$$K = C (d_{10})^2$$

Where:

- K is hydraulic conductivity in m/s;
- C is an empirical coefficient, which takes a value between 0.8 and 1.2 for medium to coarse sands (1.0 is used herein); and
- d_{10} is the diameter of the 10th percentile grain size of the material (effective grain size) in cm.

Grain size data for Site soils are obtained from a preceding resource evaluation study (Lafarge, 2017) and are supplied in Appendix C. A total of 25 below-water samples are assessed to provide an understanding of the saturated hydraulic conductivity of the ORAC at the Site.

In summary, the Hazen Method calculations yielded aquifer material hydraulic conductivities ranging from 6E-5 m/s to 1E-3 m/s with a geometric mean of 2E-4 m/s.

Assuming a flow gradient of 4E-3 m/m (see prior section), an effective porosity of 0.25, and a hydraulic conductivity range of 6E-5 m/s to 1E-3 m/s, the groundwater velocity at the Site is estimated to range from 9.6E-7 m/s (0.08 m/day) to 1.6E-5 m/s (1.4 m/day).

4.4 Water Quality

Baseline groundwater quality conditions were evaluated by taking water samples from the three monitoring wells and the on-Site domestic well ("PW") during September and October 2018. Follow up sampling events occurred during April 2019 and October 2020. The samples were collected using dedicated Waterra Model D-25 inertial pumps and 16-millimetre (5/8 inch) inside diameter polyethylene tubing. Prior to sampling, the wells were purged of a minimum of three well volumes and allowed to recover to their approximate static water level at the time of sampling. The samples were collected in pre-supplied laboratory bottles, placed in coolers and delivered within twenty-four hours to an accredited laboratory.

The groundwater samples were analysed for the following parameters:

- Inorganic water quality parameters including metals;
- Microbiology / bacteria;
- Polycyclic Aromatic Hydrocarbons (PAHs);
- Volatile Organic Compounds (VOCs); and
- Petroleum hydrocarbons (PHC) (F1 – F4).

The water quality analysis results are provided in Appendix D. Parameter concentrations were compared to "Table 2: Full Depth Generic Site Condition Standards [SCS] in a Potable Ground Water Condition" from the

Ministry of Environment, Conservation and Parks (MECP) *Soil, ground water and sediment standards for use under Part XV.1 of the Environmental Protection Act*, dated July 1, 2011. The following is noted:

- None of the inorganic parameters including metals were detected at concentrations greater than the Table 2 SCS criteria.
- Chlorides were found in all wells, suggesting impacts from road salting.
- Nitrate was found in MW18-03, suggesting impacts from fertilizer application to farm fields.
- Hardness as (CaCO₃) was greater than 200 mg/L, indicating hard water.
- Sodium was found to be above the Ontario Drinking Water Standard (ODWS) guideline of 20 mg/L for people on sodium restricted diets at MW18-01; however, sodium at the domestic well was less than 20 mg/L.
- Total dissolved solids (TDS) was found to be above the ODWS aesthetic objective of 500 mg/L at MW18-01.
- E. Coli bacteria was present in MW18-01 and total coliforms were found in MW18-03. The presence of bacteria in these wells may be as a result of fertilizer application or faecal waste from farm animals.
- PAHs were not detected in the two wells tested (MW18-02 and MW18-03).
- VOCs were not detected in the four wells.
- PHCs (F3 fraction) were found at MW18-01 in September 2018 at a concentration of 2,400 ug/L, notably above the Table 2 SCS criteria of 500 ug/L. Additionally, PHCs (F2 and F3) were detected at MW18-03 in October 2018 but at concentrations below Table 2 SCS criteria. The location of the PHC detects could suggest that the railway corridor is a potential source. We further note that the on-Site domestic well and upgradient well MW18-02 did not have PHCs. The results of subsequent PHC-focused sampling events are summarized as follows:
 - April 2019: Sampling at all wells did not detect any PHCs except for MW18-03 which had PHC F3 at a concentration below Table 2 SCS.
 - October 2020: Well MW18-03 could not be sampled as a result of the watterra tubing snapping in the well. Sampling at all the remaining wells did not detect any PHCs except for MW18-01 which had a PHC F3 concentration at the Table 2 SCS limit of 500 ug/L.
 - May 2021: Sampling at all wells did not detect any PHCs.

The sampling regime to-date generally suggests that the presence of PHC has been declining at tested wells and, as of the most recent sampling event, is no longer detected. As such, the historic presence of PHC is not considered a factor in this licence application. Nonetheless, we recommend continued confirmatory water quality sampling during operations (Section 8).

5.0 WATER BUDGET

A Site water budget was conducted to estimate the average annual water balance for Existing and Rehabilitated Scenarios. The Operational Scenario was not explicitly considered because the range of its potential effects are captured between the Existing and Rehabilitated Scenario results.

5.1 Approach

The water budget employs Environment Canada procedures (Johnstone and Louie, 1983) and is governed by the following generalized formula:

$$\text{Rainfall} + \text{Snowmelt} - \text{Evapotranspiration} - \text{Change in Soil Storage} = \text{Surplus}$$

The Environment Canada Port Perry station data (1984 – 2005) provides monthly water budget summaries used to infer average annual climatic conditions at the Site. These water budget summaries contain monthly average precipitation, evapotranspiration and surplus values (in mm) for a range of water holding capacities (WHC). For temperate regions, the change in soil storage is relatively stable year-round and represents a minor component of the annualized water budget; as such, it is ignored in this analysis.

The Site's average annual precipitation (rainfall + snowmelt) totals approximately 874 mm/yr. Whereas precipitation values are independent of the Site's physical characteristics, evapotranspiration – and thus surplus – depends on the selected WHC for a given catchment. WHCs are specific to the soil type and land use and may be applied using Table 3.1 of the *Ministry of Environment (MOE) Stormwater Management Planning and Design Manual* (MOE, 2003). The following WHCs are implemented at the Site:

- 150 mm for moderately rooted crops and pasture areas;
- 250 mm for forested areas;
- 100 mm for the pit side;
- Limited evaporation for impermeable surfaces (Surplus = 90% Precipitation); and
- 0 mm for pond water (Potential ET = Actual ET).

Our approach further separates surplus into infiltration and runoff. Infiltration estimates for each land use may be obtained using the infiltration factors shown in Table 3.1 of MOE, 2003. Current and future land use at the Site is identified as either Crop Land (the farm fields), Pasture (sloped and flat), Forest (natural and rehabilitated), Impermeable (rooftops) and Pond Area (flooded extraction area below groundwater table). The infiltration factor for each land use is estimated as the sum of the cover, soil type, and topography factors. Infiltration factors represent the proportion of surplus becoming infiltration with the remainder of the surplus going to runoff. Since the Rehabilitated Site is internally draining (i.e. no discharge from pit bottom to surface), any runoff within the berm area will eventually become infiltration as it reports to the pit. Any infiltration that reaches the saturated groundwater system is assumed to join the regional groundwater system. Infiltration factors applied in this analysis are shown in Table 2.

Table 2: Land Use Water Budget Input Data

VEGETATION COVER	Water Holding Capacity (mm)	Infiltration Factor			
		COVER FACTOR	SOIL TYPE COVER	TOPOGRAPHY FACTOR	FINAL INFILTRATION FACTOR
Crop Land	150	0.1	0.4	0.2	0.7
Pasture	150	0.15	0.4	0.2	0.75

VEGETATION COVER	Water Holding Capacity (mm)	Infiltration Factor			
		COVER FACTOR	SOIL TYPE COVER	TOPOGRAPHY FACTOR	FINAL INFILTRATION FACTOR
Forest and Rehab, Vegetation	250	0.2	0.4	0.2	0.8
Rooftops/ Impermeable surfaces	3	0	0	0	0
Rehab Pit Sidewall	100	0.1	0.4	0.1	0.6
Pit Pond	AET = PET	-	-	-	1

5.2 Catchment Areas

Site catchment (i.e. drainage) areas are delineated for Existing and Rehabilitated Scenarios based on topographic mapping provided by MHBC (Appendix A).

5.2.1 Existing Scenario

Under the Existing Scenario the Site is divided into three catchments based on the direction of natural drainage (Figure 7):

- Catchment 101 (6.3 ha) draining south towards the existing Lafarge Goodwood pit. The existing pit floor appears to have no natural outlet, suggesting it drains internally to infiltration with no external runoff;
- Catchment 102 (2.8 ha) draining north across a low point along the rail line, and from there draining northwest to Wetland 3 approximately 600 m northwest of the Site; and
- Catchment 103 (8.8 ha) draining to the east via sheet flow to a roadside ditch, across Concession Road 4 via culvert and then infiltrating within adjacent farm field.

The Existing scenario includes a combination of Pasture, Cropland, and Impermeable areas (i.e. rooftops, roads). Pertinent characteristics of the Existing Scenario catchment are summarized in Table 3.

Table 3: Existing Scenario Land Use

CATCHMENT	Cropland (ha)	Pasture (ha)	Forest and Rehab. Vegetation (ha)	Rooftops and Impermeable Surfaces (ha)	Rehab Pit Sidewall (ha)	Pond, Wetland, and Flooded Quarry (ha)	Total (ha)
101 - Existing South	5.4	0.0	0.8	0.0	0.0	0.0	6.3
102 - Existing North	0.8	1.7	0.2	0.1	0.0	0.0	2.8
103 - Existing East	0.0	7.2	1.1	0.5	0.0	0.0	8.8
Total	6.2	8.9	2.1	0.6	0.0	0.0	17.9

5.2.2 Rehabilitated Scenario

The Rehabilitated Scenario contemplates a 5.7 ha pit pond with surrounding rehabilitated pit slopes (9.7 ha), assumed to be pasture-type land with long grasses, draining towards the pond (Figure 8). Setback areas outside of the extraction boundary which used to drain to the north and east (2.1 ha) would continue to drain externally to the north and east. The southerly setback area which used to drain south to the adjacent pit (0.5 ha) would be cut off from the adjacent pit and would instead drain to the pond. In the Rehabilitated Scenario all surplus within the former extraction area is considered infiltration – whether that occurs within the rehabilitated sloping or into the pond itself (the pond being a manifestation of the groundwater table at surface). Pertinent characteristics of the Rehabilitated Scenario catchment are summarized in Table 4.

Table 4: Rehabilitated Scenario Land Use

CATCHMENT	Cropland (ha)	Pasture (ha)	Forest and Rehab. Vegetation (ha)	Rooftops and Impermeable Surfaces (ha)	Rehab Pit Sidewall (ha)	Pond, Wetland, and Flooded Quarry (ha)	Total (ha)
201 - Buffer South	0.0	0.4	0.1	0.0	0.0	0.0	0.5
202 - Buffer North	0.0	0.4	0.1	0.0	0.0	0.0	0.5
203 - Buffer East	0.0	1.5	0.1	0.0	0.0	0.0	1.6
204 - Rehab Pit	0.0	0.0	0.0	0.0	9.7	5.7	15.4
Total	0.0	2.2	0.3	0.0	9.7	5.7	17.9

5.3 Results

5.3.1 Existing Scenario

The water budget results for the Existing Scenario catchments are listed in Table 5 in both mm/yr and m³/yr. As mentioned previously, the Port Perry climate station records an average annual precipitation of 874 mm/yr (156,300 m³/yr). Under the Existing Scenario, the overall Site water budget results in an area-weighted average evapotranspiration rate of 558 mm/yr (99,800 m³/yr) and corresponding surplus of 316 mm/yr (56,600 m³/yr) as determined based on data obtained from the Environment Canada dataset and land use information. Based on the catchment infiltration factors, an infiltration rate of 214 mm/yr (38,300 m³/yr) is estimated with a corresponding runoff of 102 mm/yr (18,300 m³/yr).

On an individual catchment scale, the runoff for Catchment 101 (5,400 m³/yr), Catchment 102 (2,600 m³/yr) and Catchment 103 (10,300 m³/yr) are notable in that these are flow exiting the Site. Catchment 101 contributes runoff to the exiting Lafarge Goodwood pit, Catchment 102 contributes runoff to (ultimately) Wetland 3, and Catchment 103 contributes runoff to the agricultural field east of the Site adjacent to Concession Road 4.

Table 5: Existing Scenario Results

CATCHMENT	TOTAL AREA	PRECIPITATION		EVAPO-TRANSPIRATION		SURPLUS		INFILTRATION		RUNOFF	
	(HA)	MM/YR	M ³ /YR	MM/YR	M ³ /YR	MM/YR	M ³ /YR	MM/YR	M ³ /YR	MM/YR	M ³ /YR
101 - Existing South	6.3	874	54,800	574	36,000	300	18,800	214	13,400	86	5,400
102 - Existing North	2.8	874	24,300	562	15,600	312	8,700	218	6,100	94	2,600
103 - Existing East	8.8	874	77,300	545	48,200	329	29,100	213	18,800	116	10,300
TOTAL	17.9	874	156300	558	99,800	316	56,600	214	38,300	102	18,300

5.3.2 Rehabilitated Scenario

The water budget results for the Rehabilitated Scenario catchments are listed in Table 6. Site rainfall remains unchanged (874 mm/yr or 156,300 m³/yr), but there is a slight increase in overall evapotranspiration (567 mm/yr or 101,400 m³/yr), reflecting the increased evaporation from the surface of the pond. It follows that total surplus under the Rehabilitated Conditions (55,000 m³/yr) remains similar to the Existing Scenario (56,6000 m³/yr). Total infiltration under Rehabilitated Conditions (53,500 m³/yr) is significantly greater than the Existing Conditions infiltration (38,300 m³/s). Conversely, there is a reduced runoff in the Rehabilitated Conditions (1,500 m³/yr) as opposed to the Existing Conditions (18,300 m³/yr), as the proposed extraction area would drain internally under Rehabilitated Conditions.

On an individual catchment scale, the runoff exiting the Site to the north (300 m³/yr), south (0 m³/yr), and east (1,200 m³/yr) are reduced relative to the Existing Scenario. The implications of the runoff reductions are discussed in Section 6 Impact Assessment.

Table 6: Rehabilitated Scenario Results

CATCHMENT	TOTAL AREA	PRECIPITATION		EVAPO-TRANSPIRATION		SURPLUS		INFILTRATION		RUNOFF	
	(HA)	MM/YR	M ³ /YR	MM/YR	M ³ /YR	MM/YR	M ³ /YR	MM/YR	M ³ /YR	MM/YR	M ³ /YR
201 - Buffer South	0.5	874	4,000	571	2,600	303	1,400	303	1,400	0	0
202 - Buffer North	0.5	874	3,900	576	2,600	298	1,300	226	1,000	71	300
203 - Buffer East	1.6	874	13,900	572	9,100	302	4,800	227	3,600	75	1,200
204 - Rehab Pit	15.4	874	134,500	566	87,100	308	47,500	308	47,500	0	0
TOTAL	17.9	874	156,300	567	101,400	307	55,000	299	53,500	8	1,500

6.0 IMPACT ASSESSMENT

The impact assessment seeks to estimate potential changes to the hydrogeologic / hydrologic system as a result of Site Operations and Rehabilitation Scenarios and the effect these changes may have on groundwater users / receptors. Our analysis focuses on impacts to the following:

- Groundwater levels;
- Baseflow;
- Water well quantities;
- Baseflow to water features;
- Aquifer vulnerability and groundwater quality;
- Groundwater temperature; and
- Site water budget.

6.1 Groundwater Levels

The below-water operation will not involve any pumping or active dewatering. Rather, the majority of pore water removed during extraction will eventually return to the aquifer via passive drainage within the stockpiled material.

Thus, the principal mechanism for Site development to instigate long-term effects on groundwater levels is exposing the water table to the atmosphere. Below-water aggregate extraction results in the eventual creation of a permanent pond that will tend to “flatten” water levels in its vicinity. The area upgradient of the pond experiences water level drawdown whereas the area downgradient of the pond experiences water level rise. Typically, only drawdown is of concern with respect to water quantity impacts. Under this paradigm the Rehabilitation Scenario provides the “worst-case” outcome with respect to long-term drawdown.

It is notable that the positioning of the (eventual) pond at the existing Goodwood Pit to the south would tend to ameliorate long-term Site drawdown because of the water level rise induced at the existing Goodwood Pit’s northerly pond edge. However, for the purposes of this impact assessment, such an effect is conservatively not taken into account, and the drawdown effect of the Site pond is examined in isolation.

The Rehabilitation Scenario pond level is expected to approximate the average groundwater level within its area. Under high water table conditions (Figure 6), the water level within the pond area is estimated to be 322 masl based on the contouring program Surfer (Golden Software, 2015). The average annual water level would be close to, but less than, 322 masl (likely between 321 masl to 322 masl). Thus, for the purposes of this impact assessment, an estimate of future drawdown along the upgradient (southern) perimeter of the pond may be obtained by taking the existing upgradient high groundwater elevation of 322.25 masl (at MW18-02) and subtracting the future pond level of 322 masl resulting in a drawdown of 0.25 m at the upgradient pond perimeter. Likewise, a water level *increase* of approximately 0.25 m could be expected along the downgradient (northern) perimeter of the future pond. Water level changes of similar magnitude could be expected during different times of the year as the pond and surrounding groundwater levels would jointly rise and fall over a seasonal time scale.

To estimate the extent of the pond’s lateral zone of influence (i.e. where drawdown reaches zero) we use the analytical method *Simple analytical equations for estimating ground water inflow to a mine pit* (Marinelli and

Niccoli, 2000). An illustration of this method along with Site specific inputs and calculations are provided in Appendix E. In summary, the inputs are as follows:

- The pit pond area is expected to be 5.7 ha, or 57,000 m². The effective radius is approximated as:
 - $A_{\text{pond}} = \pi * r_e^2$
 - $r_e = [(57,000 \text{ m}^2) / (3.14)]^{1/2}$
 - $r_e = 135 \text{ m}$
- Recharge is assumed to be 214 mm/yr per the Site water budget;
- The hydraulic conductivity of the ORAC is assumed to be 2E-4 m/s per grain size analysis;
- The pit pond floor is assumed to be 310 masl. As a result, under high water table conditions:
 - The initial (i.e. pre extraction) saturated thickness is 322.25 m – 310 m = 12.25 m
 - The final (i.e. post extraction) saturated thickness is 322 m – 310 m = 12 m.
 - Note that 12.25 m minus 12 m results in the aforementioned drawdown of 0.25 m.

Based on the above inputs, the 0.25 m drawdown is calculated to decrease to a point of zero drawdown approximately 474 m from the centre of the pit pond (Figure 9).

6.2 Baseflow

Baseflow is the groundwater contribution of total flow to a surface water feature. Baseflow changes as a result of below-water extraction are related to water level changes (see above). A surface water receiver downgradient of a water level decline may experience decreased baseflow to that feature; conversely, a surface water feature downgradient of a water level rise may experience increased baseflow. Typically, only baseflow decrease is of concern within the context of impact assessment.

The only mapped surface water features in the area lie several hundred metres downgradient of the Site and these features are either known or suspected to be perched above the water table. As such, there are no significant baseflow contributions at these locations. Nonetheless, as water levels will rise downgradient of the pit pond, any theoretical groundwater receptors downgradient would receive a slight increase in baseflow and as such would not experience an adverse effect to water quantity.

6.3 Well Water Quantities

There are four MECP well records within the drawdown zone of influence (Figure 9); one of these is an observation well (7307824) and is not considered in the context of water quantity impacts. The available water column in the remaining water supply wells is compared to the estimated drawdown at each well location as inferred from the previously described analytical calculations (Table 7). Drawdown at the wells is <0.01 m with a corresponding <0.1% reduction in available column. Such minor reductions will not impact well operation. Nonetheless, in the unlikely event that a neighbouring well owner perceives an adverse impact to their well, Lafarge should develop a Water Well Complaint Action Plan. A suggested plan is provided in Section 6.3.1.

Table 7: Water Well Records In Drawdown Zone of Influence

Well ID	Construct Date	Depth (mbgs)	Static Level (mbgs)	Water Column (m) ¹	Distance From Pit Pond (m)	Estimated Drawdown (m)	Reduction In Water Column
1917600	Apr-05	54.9	25.4	29.4	400	<0.01	<0.1%
4602905	Feb-62	35.1	8.2	26.8	450	<0.01	<0.1%
1907003	Jun-84	27.7	14.3	13.4	455	<0.01	<0.1%

Note 1: Water Column = Well Depth – Static Water Level

6.3.1 Water Well Compliant Action Plan

The EMP should include a Water Well Complaint Action Plan. If a water well complaint regarding a domestic well is received by Lafarge within 500 metres of the Site the following actions should be taken:

- The licensee will contact local MECP-licensed well contractors in the event of a well malfunction and the well owner will be immediately offered a temporary water supply at the licensee's expense if the issue cannot be expediently determined and rectified.
- The designated contractor will respond to the well owner and propose a plan to rectify the problem as expediently as possible. The well owner must then provide authorization of the work.
- If the issue raised by the well owner is related to loss of water supply the licensee will have a consultant/contractor determine the likely cause, which can result from a number of factors, including climatic conditions such as drought (owner's expense), pump failure (owner's expense), extended overuse of the well (owner's expense) or the lowering of the water level in the well from extraction activities (Lafarge expense).
- Data from the Site monitoring network shall be consulted as part of the review to determine groundwater level behaviour. This assessment process will be carried out at the expense of Lafarge and the results provided to the well owner.
- The consultant/contractor will be able to readily determine if pump failure is the problem and, should the well owner choose to have the pump repaired or replaced at their expense, the contractor would correct the situation for the well owner.
- If it has been determined that extraction activities did not cause the water supply interference, then the licensee shall provide 24 hours' notice and thereafter discontinue the temporary water supply.
- If it has been determined that extraction activities caused the water supply interference, then the licensee shall continue to supply water to the well owner at the licensee's expense until the problem is rectified and the water supply is restored. The following mitigation measures shall be considered and the appropriate measure(s) implemented at the expense of the licensee:
 - Adjust pump pressure;
 - Lowering of the pump to take advantage of existing water storage within the well;

- Deepening of the well to increase the available water column;
- Widening of the well to increase the available storage of water;
- Relocation of the well to another unaffected area on the property; or
- Drilling of multiple low yield wells.

6.4 Water Quality

The Operational Scenario will not involve the on-Site storage of any fuels, oils or potentially hazardous materials that could be released into the groundwater system. Therefore, water quality is not expected to be adversely impacted. Nonetheless, a Best Management Practices (BMP) manual should be employed to address any potential spills from equipment on-Site and will minimize the potential for aquifer contamination.

It bears mentioning that the Rehabilitated Scenario represents an opportunity to improve water quality as the resulting land use will negate the application of fertilizers and/or pesticides on what would otherwise be crop land.

6.5 Water Temperature

The exposure of the water table to the atmosphere may result in an increase in groundwater temperatures emanating from the Site during summer. This can occasionally be a concern for aquatic species or habitat that require the influx of cool groundwater within a certain temperature range in order to maintain ecological function.

Prior studies in Ontario have indicated that thermal plumes originating from below-water pits typically do not migrate farther than 120 m to 250 m downgradient of the pit pond before their effect becomes negligible (Yang, 1995 and Markle and Schincariol, 2007). No aquatic habitat lies within these distances from the pit pond and therefore the off-Site migration of a thermal plume will not cause any adverse impacts.

6.6 Site Water Budget

As noted in Section 5, the transition from Existing Scenario to Operations and ultimate Rehabilitated Scenario will result in some losses to off-Site runoff as a result of re-grading and land form changes as follows (with reference to Table 6 versus Table 7):

- Catchment 101 (South): The elimination of southerly off-Site runoff (5,400 m³/yr to zero) to the existing Lafarge Goodwood Pit is considered inconsequential as the location of this water (infiltration to the groundwater system via an open pit / pond) is the same under both Existing and Rehabilitated Scenarios.
- Catchment 102 (North): The decrease in northerly off-Site runoff (2,600 m³/yr to 300 m³/yr) may result in a corresponding decrease in inflow to Wetland 3. This reduction may be put into context by comparing the post-extraction decrease in overall catchment. Based on a GIS analysis, the total catchment area to Wetland 3 is approximately 41.6 hectares. The Rehabilitated Condition would result in a loss of catchment of 2.3 ha, or 6% of the total catchment. Given the consistency in land form over the catchment, it is reasonable to assume that this loss in catchment would result in a corresponding 6% reduction in flows, and these reductions would occur largely during the spring freshet when water has been observed to flow off-Site and there is sufficient surface water flow to the wetland. Other than spring freshet the site does not provide surface flow to the Wetland. This is considered a minor loss from a hydrological perspective. The significance of the runoff decrease on natural environment receptors within Wetland 3 is evaluated by under separate cover in the *Natural Environment Reports* (WSP 2023).

- Catchment 103 (East): The decrease in easterly off-Site runoff (10,300 m³/yr to 1,200 m³/yr) will result in a decrease in flow to the adjacent farm field across Concession Road 4 during the spring freshet. This action may provide an overall benefit to the land form by mitigating excessive flooding during spring.

7.0 CONCLUSIONS

A Water Report Level 1 and 2 has been prepared in support of a Class A Pit Below Water licensing application for the proposed Goodwood Pit Extension (the Site). Existing, Operational and Rehabilitated Scenarios were considered. The Study involved two main aspects: 1) the establishment of baseline conditions for the Existing Scenario through background data review and field program data collection; and 2) an impact assessment for proposed Operations and Rehabilitated Scenarios. The following conclusions are drawn from the Study:

7.1 Existing Scenario

- Average annual precipitation in the Site vicinity is 874 mm/yr. Evapotranspiration is estimated to be 558 mm/yr with a resulting surplus of 316 mm/yr. The majority of this surplus becomes infiltration (214 mm/yr) with the remainder becoming runoff (102 mm/yr).
- There are no permanent surface water features on-Site. However, flowing surface water (i.e. runoff) is observed during the spring freshet. During this period the runoff either 1) ponds within localized depressions and infiltrates; or 2) exits the Site via topographic lows at the north, east and south of the Site.
- There are three wetlands that are mapped within 1 km of the Site. All are inferred to be perched and not connected to the saturated groundwater system. The only wetland that has any hydrologic connection to the Site is "Wetland 3", located to the northwest. This wetland receives a small portion of its water supply (6%) from runoff leaving the Site in a northerly direction during spring freshet when there is sufficient surface water flow to this wetland.
- The Site aggregate resource is located within the Oak Ridges Moraine Aquifer Complex (ORAC). The ORAC is generally comprised of sands and gravels with localized deposits of finer grained materials. Site drilling suggests the ORAC is at least 37 m thick in the vicinity of the Site. Estimated aquifer hydraulic conductivity ranges from 6E-5 m/s to 1E-3 m/s with a geometric mean of 2E-4 m/s.
- The Site is inferred to be just downgradient of a regional groundwater divide within the ORAC, with groundwater flowing in a generally north to northeasterly direction through the Site at elevations in the 322 to 320 masl range. Depth to groundwater at the Site ranges from 26 m to 24 m.
- There are 30 water wells within 500 m of the Site. All the wells are drilled and completed within the ORAC at depths ranging from 28 mbgs to 57 mbgs. The majority of wells are classed as "Domestic".
- Tested groundwater quality generally met Table 2 SCS criteria. However, impacts from surficial contaminants were evident in elevated chloride, nitrate, and bacteria levels. Petroleum hydrocarbons (PHCs) were initially found in several wells; however, subsequent sampling suggested a general decline in concentrations with the most recent sampling event in May 2021 indicating non-detect in all wells.
- The Site is not located within, nor does it interact with, any Source Protection Plan vulnerable area or any LSRCA Regulated Area.

7.2 Operational and Rehabilitated Scenarios

- The below-water operation will not involve any pumping or active dewatering. Rather, the majority of pore water “removed” during extraction will eventually return to the aquifer via passive drainage within the stockpiled material.
- Below-water aggregate extraction will result in the eventual creation of a permanent pond that will flatten water levels in its immediate vicinity. The area upgradient of the pond (south) will incur water level drawdown whereas the area downgradient of the pond (north) will incur water level rise. The magnitude of water level change is estimated to be on the order of 0.25 m at the pit pond with a zone of influence extending approximately 474 m from the centre of the pond to a point of zero drawdown.
- There is not expected to be any adverse impacts to baseflow at groundwater receptors as result of the aforementioned minor water level changes.
- There is not expected to be any adverse impacts to water quantity at surrounding private water wells as a result of the aforementioned minor water level changes.
- There will be no on-Site storage of any fuels, oils or potentially hazardous materials that could be released into the groundwater system; as such there is not expected to be any adverse impacts to water quality.
- The exposure of the water table to the atmosphere may result in a minor increase in groundwater temperatures emanating from the Site during summer. However, there are no groundwater-dependent aquatic habitats within the thermal zone of influence (120 m to 250 m) downgradient of the Site and thus no receptors will be adversely impacted.
- The transition from Existing Scenario to Operations and ultimate Rehabilitated Scenario will result in some losses to off-Site runoff as a result of re-grading and landform changes:
 - Catchment 101 (South): The elimination of southerly off-Site runoff (5,400 m³/yr to zero) to the existing Lafarge Goodwood Pit is considered inconsequential as the location of this water (infiltration to the groundwater system via an open pit / pond) is the same under both Existing and Rehabilitated Scenarios.
 - Catchment 102 (North): The decrease in northerly off-Site runoff (2,600 m³/yr to 300 m³/yr) may result in a corresponding decrease in inflow to Wetland 3. However, this is estimated to be only a 6% overall reduction in flow to the wetland and is thus considered a minor loss.
 - Catchment 103 (East): The decrease in easterly off-Site runoff (10,300 m³/yr to 1,200 m³/yr) will result in a decrease in flow to the adjacent farm field across Concession Road 4 during the spring freshet. This action may provide an overall benefit to the land form by mitigating excessive flooding during spring.
- This Water Report Level 1 and 2 has addressed all hydrological / hydrogeological policy considerations as set forth in Section 1.1.

8.0 RECOMMENDATIONS

The following is recommended:

- Groundwater monitoring shall continue through Operations. The following tasks shall be carried out:
 - Monthly manual groundwater level monitoring at the three on-Site monitoring wells.

- Annual groundwater quality sampling for general chemistry, metals, bacteria, petroleum hydrocarbons and volatile organic compounds.
- The Site domestic well will need to be decommissioned per O.Reg. 903 by a licensed well contractor either prior to extraction or at such time that extraction encroaches on the well location
- A complaint response program will be in place for residential wells within the potential draw down zone influence (i.e., 4639, 4709, 4840 and 4860 Concession Road 4) during extraction to address any complaints for surrounding residents concerning the supply of their water wells. In the event of unanticipated impacts to the water wells as a result of pit operations, mitigation and contingency measures will be implemented to restore the affected quantity or quality of the groundwater supply.
- Prior to extraction below water, a door-to-door water well survey shall be conducted to confirm baseline conditions at private wells within 500 m of the Site.

9.0 REFERENCES

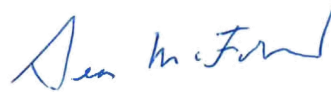
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Signature Page

WSP Canada Inc.



Hayley Wallace, M.E.Sc., P.Eng.
Hydrogeologist

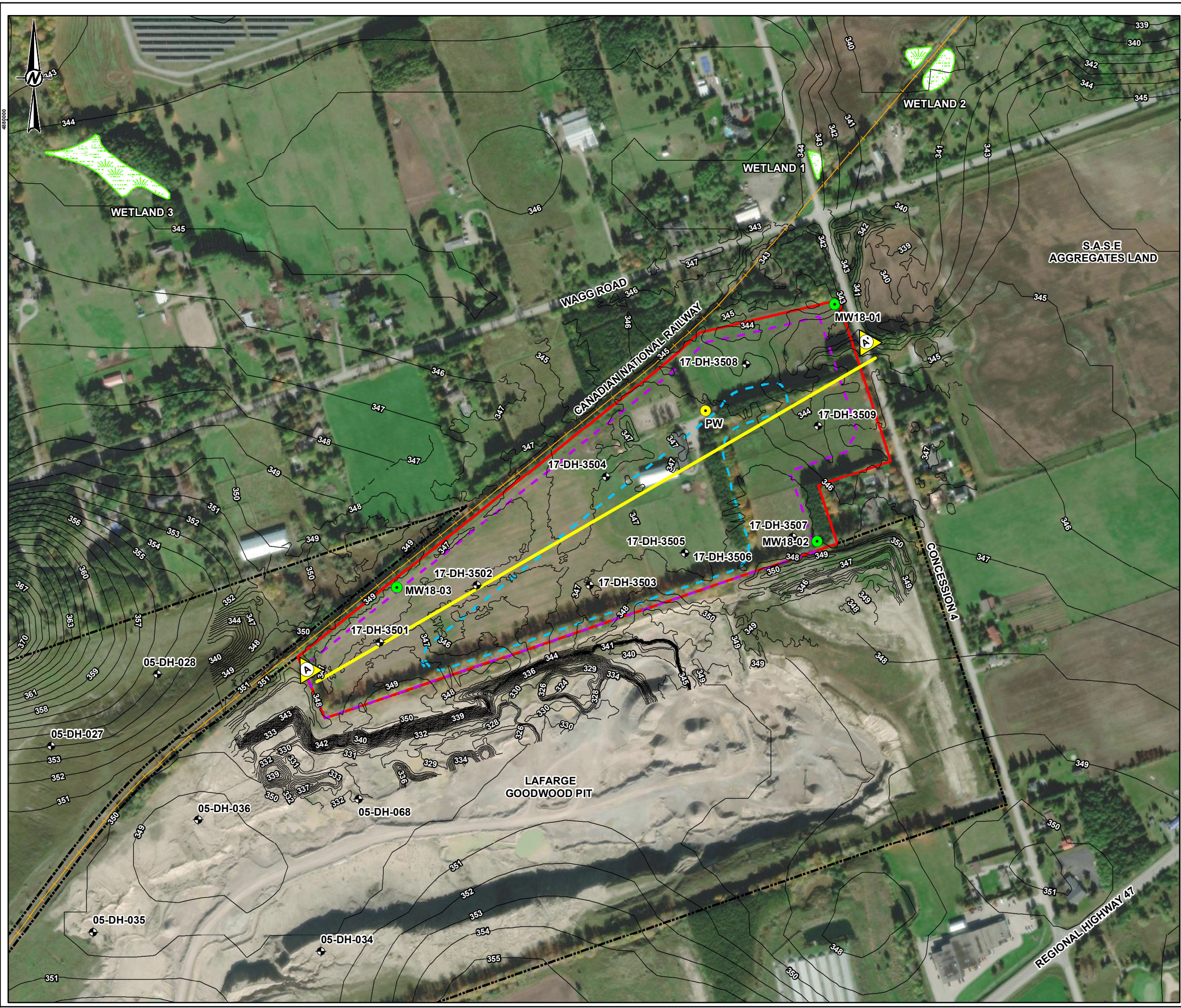


Sean McFarland, Ph.D., P.Geo.
Hydrogeologist, Senior Principal/Fellow

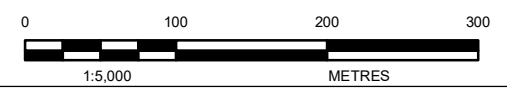
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FIGURES



- LEGEND**
- ON-SITE DOMESTIC WELL LOCATION
 - MONITORING WELL LOCATION
 - ◆ RESOURCE EVALUATION BOREHOLE
 - CONTOUR (1m INTERVAL)
 - CROSS SECTION LINE
 - RAILWAY
 - EXISTING GOODWOOD PIT SITE BOUNDARY
 - SITE BOUNDARY
 - LIMIT OF EXTRACTION
 - APPROXIMATE BELOW WATER (PIT POND) EXTENTS
 - UNEVALUATED WETLAND



REFERENCE(S)

1. BASEDATA: MNR LIO OBTAINED APRIL 2019
2. CONTOURS SUPPLIED BY MHBC, FILE NAME "EXISTING FEATURES", DRAWING NO. 1 OF 3, FILE NO. 9526HC, DATED MAY 2019.
3. IMAGERY: SOURCES: ESRI, HERE, GARMIN, INTERMAP, INCREMENT P CORP., GEBCO, USGS, FAO, NPS, NRCAN, GEOBASE, IGN, KADASTER NL, ORDNANCE SURVEY, ESRI JAPAN, METI, ESRI CHINA (HONG KONG), (C) OPENSTREETMAP CONTRIBUTORS, AND THE GIS USER COMMUNITY
4. PROJECTION: TRANSVERSE MERCATOR DATUM: NAD 83 COORDINATE SYSTEM: UTM ZONE 17N

CLIENT
LAFARGE CANADA INC.

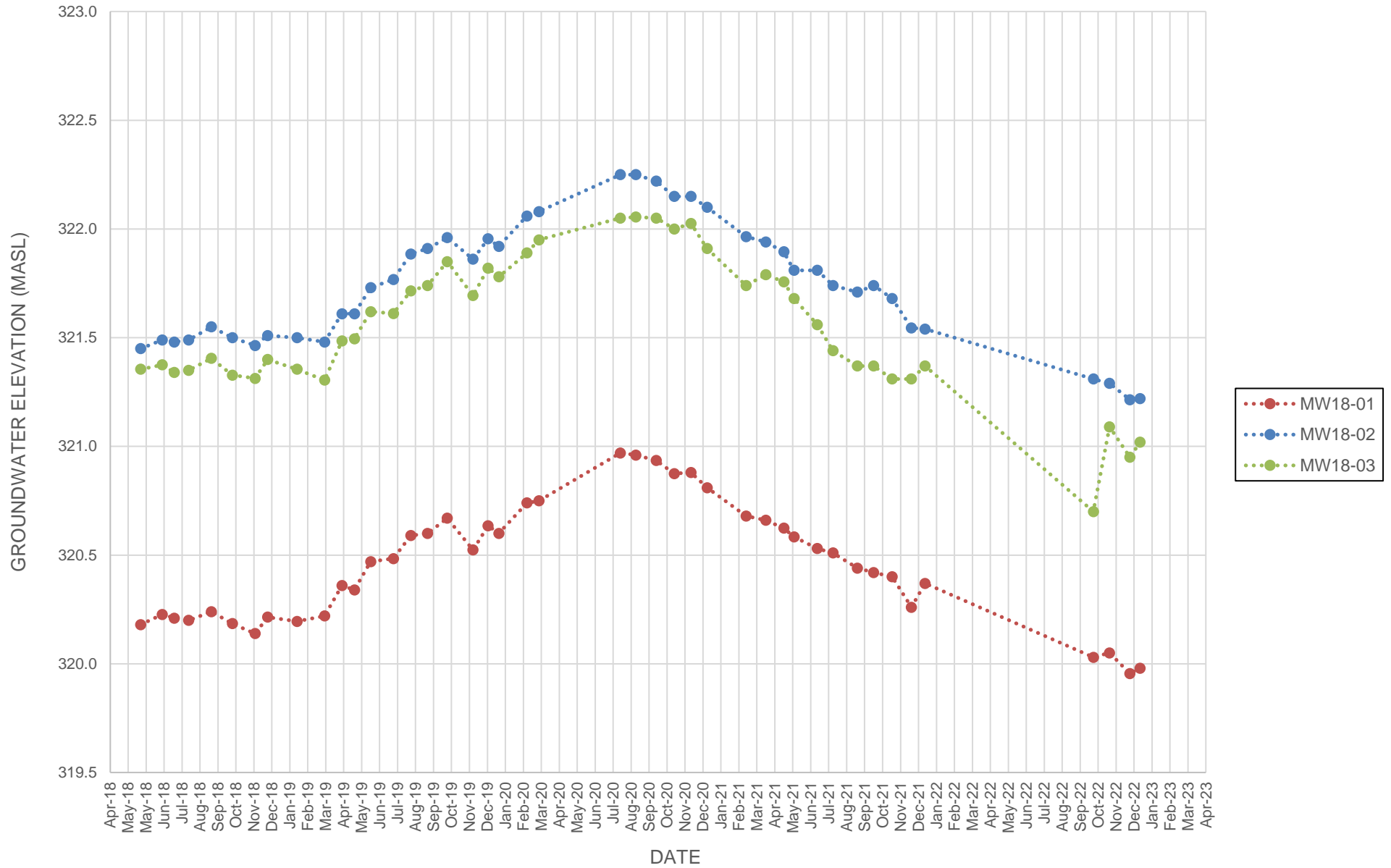
PROJECT
GOODWOOD PIT EXTENSION
4900 CONCESSION ROAD 4, TOWNSHIP OF UXBRIDGE

TITLE
SITE DETAIL

CONSULTANT	YYYY-MM-DD	2023-05-19
	DESIGNED	SO
	PREPARED	SO/JT
	REVIEWED	DH
	APPROVED	

PROJECT NO. 21453907 CONTROL 0001 REV. 1 FIGURE 2

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 IF THIS MEASUREMENT DOES NOT MATCH WHAT IS SHOWN, THE SHEET SIZE HAS BEEN MODIFIED FROM: ANSI B



NOTES

LAFARGE CANADA INC.



WSP Canada Inc.
BARRIE, ONTARIO, CANADA

GROUNDWATER HYDROGRAPHS

MAY 2023

PROJECT: 21453907

FIGURE: 5



- LEGEND**
- ON-SITE DOMESTIC WELL LOCATION
 - MONITORING WELL LOCATION
 - ⊕ RESOURCE EVALUATION BOREHOLE
 - CONTOUR (1m INTERVAL)
 - RAILWAY
 - - - EXISTING SCENARIO CATCHMENT
 - ▭ EXISTING GOODWOOD PIT SITE BOUNDARY
 - ▭ SITE BOUNDARY
 - ▭ UNEVALUATED WETLAND

Catchment	Area (ha)	Drainage Direction
101	6.3	South
102	2.8	North
103	8.8	East



REFERENCE(S)

1. BASEDATA: MNR/LIO OBTAINED APRIL 2019
2. IMAGERY: SOURCES: ESRI, HERE, GARMIN, INTERMAP, INCREMENT P CORP., GEBCO, USGS, FAO, NPS, NRCAN, GEOBASE, IGN, KADASTER NL, ORDNANCE SURVEY, ESRI JAPAN, METI, ESRI CHINA (HONG KONG), (C) OPENSTREETMAP CONTRIBUTORS, AND THE GIS USER COMMUNITY
3. PROJECTION: TRANSVERSE MERCATOR DATUM: NAD 83 COORDINATE SYSTEM: UTM ZONE 17N

CLIENT
LAFARGE CANADA INC.

PROJECT
GOODWOOD PIT EXTENSION
4900 CONCESSION ROAD 4, TOWNSHIP OF UXBRIDGE

TITLE
EXISTING SCENARIO CATCHMENT AREAS

CONSULTANT	DATE	BY
	YYYY-MM-DD	2023-05-19
	DESIGNED	SO
	PREPARED	SO/JT
	REVIEWED	DH
	APPROVED	

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LEGEND

- MONITORING WELL LOCATION
- ◆ RESOURCE EVALUATION BOREHOLE
- CONTOUR (1M INTERVAL)
- RAILWAY
- REHABILITATION SCENARIO CATCHMENT
- EXISTING GOODWOOD PIT SITE BOUNDARY
- ▭ SITE BOUNDARY
- ▭ PIT POND (321 – 322 MASL)
- ▭ LIMIT OF EXTRACTION
- ▭ UNEVALUATED WETLAND

Catchment	Area (ha)	Drainage Direction
201	0.5	South
202	0.5	North
203	1.6	East
204	15.37	Centrally to Pond



NOTE(S)
 1. EXISTING LAFARGE GOODWOOD PIT AS SHOWN SUBJECT TO FUTURE CHANGE.

REFERENCE(S)
 1. BASEDATA: MNR/LIO OBTAINED APRIL 2019
 2. IMAGERY: SOURCES: ESRI, HERE, GARMIN, INTERMAP, INCREMENT P CORP., GEBCO, USGS, FAO, NPS, NRCAN, GEOBASE, IGN, KADASTER NL, ORDNANCE SURVEY, ESRI JAPAN, METI, ESRI CHINA (HONG KONG), (C) OPENSTREETMAP CONTRIBUTORS, AND THE GIS USER COMMUNITY
 SOURCE: ESRI, MAXAR, EARTHSTAR GEOGRAPHICS, AND THE GIS USER COMMUNITY
 3. PROJECTION: TRANSVERSE MERCATOR DATUM: NAD 83 COORDINATE SYSTEM: UTM ZONE 17N

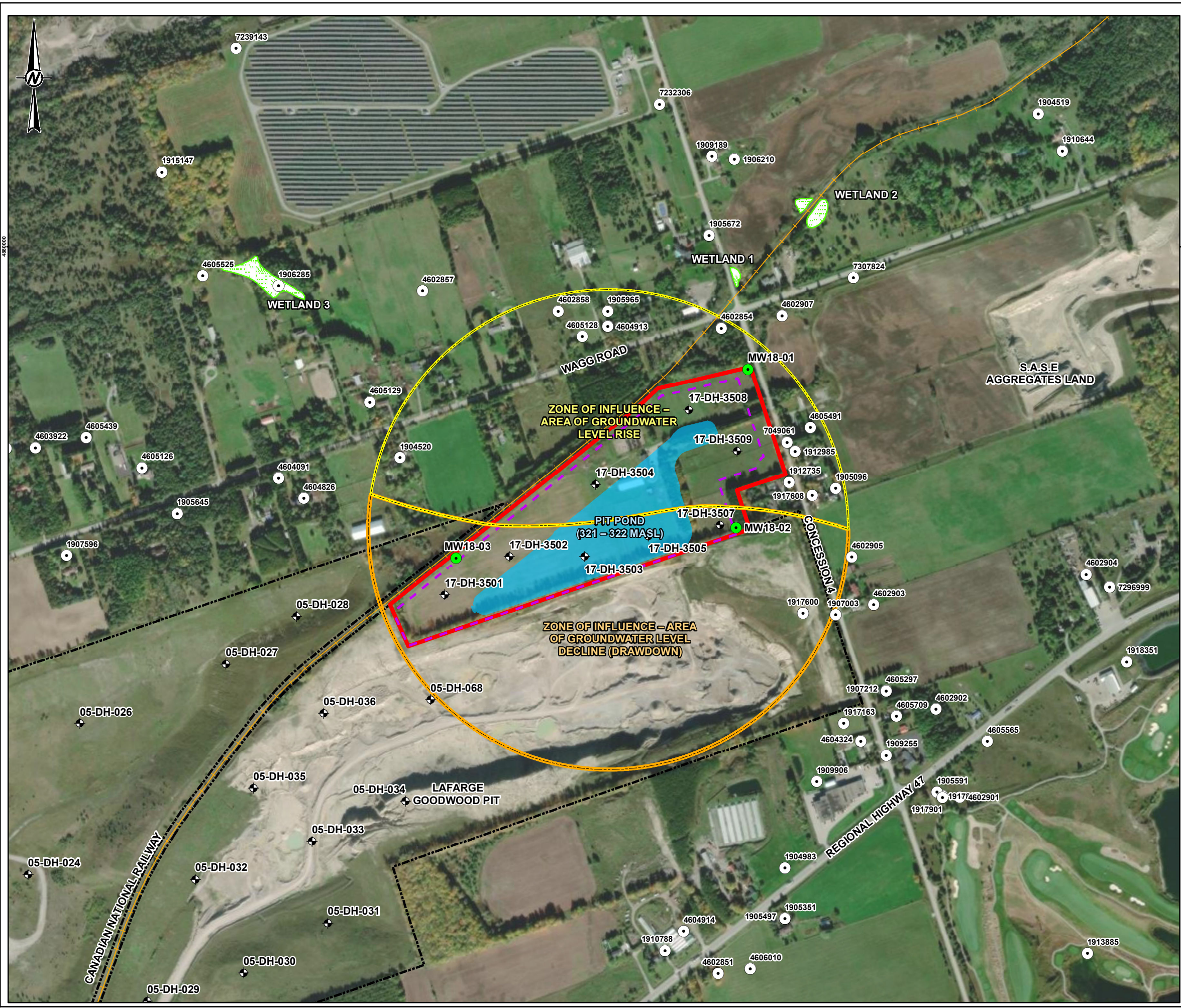
CLIENT
LAFARGE CANADA INC.

PROJECT
**GOODWOOD PIT EXTENSION
 4900 CONCESSION ROAD 4, TOWNSHIP OF UXBRIDGE**

TITLE
REHABILITATED SCENARIO CATCHMENT AREAS

CONSULTANT	YYYY-MM-DD	2023-05-19
	DESIGNED	SO
	PREPARED	SO/JT
	REVIEWED	DH
	APPROVED	

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LEGEND

- MECP WATER WELL LOCATION
- MONITORING WELL LOCATION
- ⊕ RESOURCE EVALUATION BOREHOLE
- RAILWAY
- ⬢ EXISTING GOODWOOD PIT SITE BOUNDARY
- ⬢ SITE BOUNDARY
- ZONE OF INFLUENCE – AREA OF GROUNDWATER LEVEL DECLINE (DRAWDOWN)
- ZONE OF INFLUENCE – AREA OF GROUNDWATER LEVEL RISE
- PIT POND (321 – 322 MASL)
- ⬢ LIMIT OF EXTRACTION
- UNEVALUATED WETLAND



NOTE(S)
 1. EXISTING LAFARGE GOODWOOD PIT AS SHOWN SUBJECT TO FUTURE CHANGE.

REFERENCE(S)
 1. BASEDATA: MNR/LIO OBTAINED APRIL 2019
 2. IMAGERY: SOURCES: ESRI, HERE, GARMIN, INTERMAP, INCREMENT P CORP., GEBCO, USGS, FAO, NPS, NRCAN, GEOBASE, IGN, KADASTER NL, ORDNANCE SURVEY, ESRI JAPAN, METI, ESRI CHINA (HONG KONG), (C) OPENSTREETMAP CONTRIBUTORS, AND THE GIS USER COMMUNITY
 SOURCE: ESRI, MAXAR, EARTHSTAR GEOGRAPHICS, AND THE GIS USER COMMUNITY
 3. PROJECTION: TRANSVERSE MERCATOR DATUM: NAD 83 COORDINATE SYSTEM: UTM ZONE 17N

CLIENT
 LAFARGE CANADA INC.

PROJECT
 GOODWOOD PIT EXTENSION
 4900 CONCESSION ROAD 4, TOWNSHIP OF UXBRIDGE

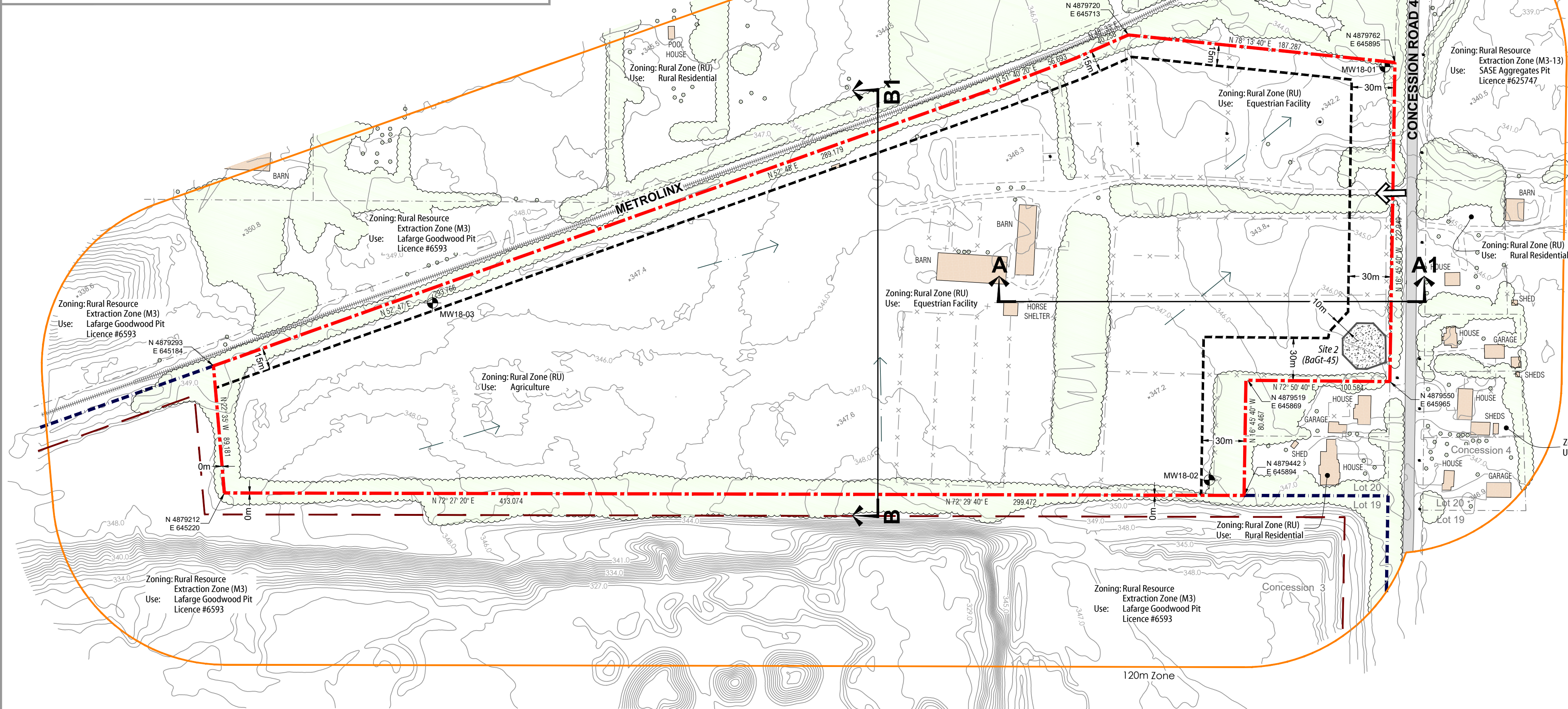
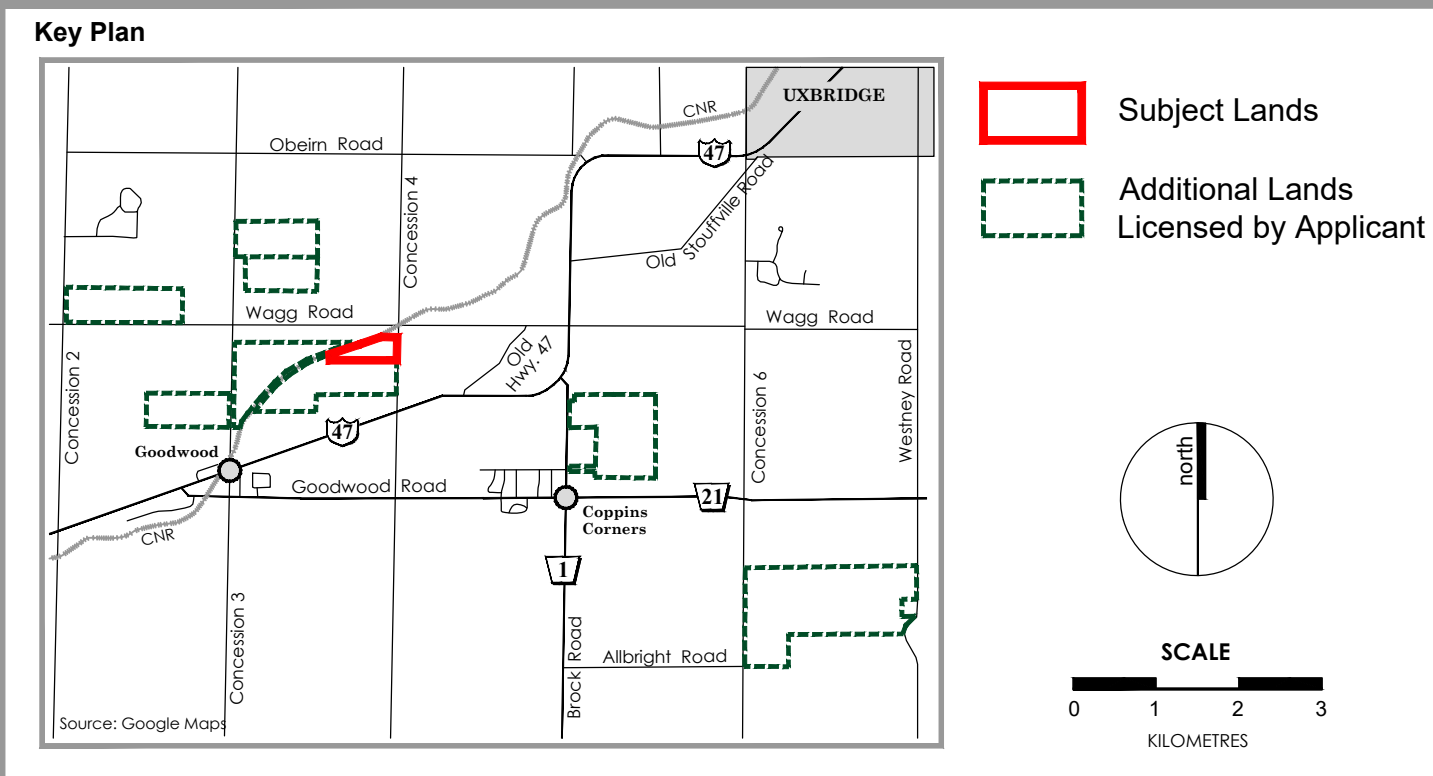
TITLE
 PIT POND ZONE OF INFLUENCE

CONSULTANT	DATE	REVISION
	YYYY-MM-DD	2023-05-19
	DESIGNED	SO
	PREPARED	SO/JT
	REVIEWED	DH
	APPROVED	

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 IF THIS MEASUREMENT DOES NOT MATCH WHAT IS SHOWN, THE SHEET SIZE HAS BEEN MODIFIED FROM: ANSI B

APPENDIX A

Draft Site Plans



- Notes**
- A. General**
- This site plan is prepared under the Aggregate Resources Act (ARA) Aggregate Resources of Ontario: Site Plan Standards August 2020.
 - Area Calculations:
Licence Area: ±17.9 hectares (44.2 acres)
Limit of Extraction: ±15.4 hectares (38.1 acres)
 - All references to north, south, east and west on this site plan are based on site north (not true north).
 - All measurements shown are in metres unless specified otherwise.
- B. References**
- Topographic information compiled by GeoOptic (a Division of Aeon Edmond Ltd.) produced from aerial photography from July 19, 2018. Mapping is produced in real-world scale and coordinates (NAD83 UTM Zone 17N). Contour interval is 1m. All elevations shown are in metres above sea level (masl). Contours for existing site (Licence #6593) from drone flight flown by Lafarge, October 2020.
 - The licence boundary was established using property boundary compiled from Plan of Survey prepared by: H.F. Grandeur Co. Ltd., Ontario Land Surveyor, October 5, 1971 (Plan 40R-6492).
 - Existing zoning on and within 120 metres of the licence is from the Township of Uxbridge Zoning By-law 81-19 (as amended), Office Consolidation July 2020. The site is currently zoned Rural (RU).
 - Land use information and structures identified on or within 120 metres of the site boundary was determined using July 2018 aerial imagery and 2018/2020 site visits.
- C. Drainage**
- Surface drainage on and within 120 metres of the licence boundary is by overland flow in the directions shown by arrows on the plan view, or by infiltration.
- D. Groundwater**
- The water table elevation on site ranges generally between ±321.7 masl in the southwest portion of the site to ±320.6 masl in the northeast portion of the site or approximately 20 metres below the existing ground surface. The existing water table elevations are shown in each cross section on this drawing and drawing 3 of 3.
- E. Site Access and Fencing**
- There is an existing field access from Concession Road 4.
 - Post and wire fencing (unless noted otherwise) exists in the locations shown on the plan view.
- F. Aggregate Related Site Features**
- There are no existing aggregate operations or features on-site such as processing areas with stationary or portable equipment, stockpiles, recyclable materials, scrap, haul roads, fuel storage, berms or excavation faces.
- G. Cross Sections**
- As shown on this page.
 - Cross section locations are identified on the plan view for each drawing.
- H. Technical Reports - References**
- Hydrogeology: "Water Report Level 2, Lafarge Goodwood Pit Extension" July 2021 (Source: Golder Associates Ltd.)
 - Natural Environment: "Proposed Goodwood Pit Extension Natural Environment Level 1 and 2 Technical Report, November 2020 (Source: Golder Associates Ltd.)
 - Noise: "Noise Impact Study - Project: 18200 Goodwood Pit Extension, Township of Uxbridge, Ontario" August 20, 2021 (Source: Aerocoustics Engineering Ltd.)
 - Archaeology: Review and Entry into the Ontario Public Register of Archaeological Reports: Archaeological Assessment Report Entitled, "Stage 3 Archaeological Assessment: Goodwood Location 1 (BaGt-45), Lafarge Goodwood Extension Property, Part of Lot 20, Concession 3, Geographic Township of Uxbridge, former Ontario County, now Regional Municipality of Durham, Ontario", Dated Jul 13, 2021, Filed with MHSTCI Toronto Office on Jul 14, 2021; MHSTCI Project Information Form Number P256-0670-2021; MHSTCI File Number 009350.
- I. Other Reports - References**
- Air Quality Assessment: "Lafarge Goodwood Pit Extension, Goodwood Ontario, Air Quality Assessment" June 29, 2021 (Source: RWDI Air Inc.)

Legal Description
PART OF LOT 20
CONCESSION 3
Township of Uxbridge
Region of Durham

Legend

- Boundary of Area to be Licensed
- Existing Licensed Boundary
- Existing Fence
- Public Road
- Private Driveway/Laneway
- Farm/Field Access
- Monitoring Wells
- Parcel Fabric
- Hydro Pole
- Cross Sections
- Limit of Extraction
- Existing Extraction Limit
- Contour and Elevation
- Spot Height Elevation
- Building/Structure
- Existing Vegetation
- Direction of Surface Drainage
- Groundwater Table
- Archaeological Site

Site Plan Amendments

No.	Date	Description	By

MHBC PLANNING URBAN DESIGN & LANDSCAPE ARCHITECTURE
200 - 540 BINGEMANS CENTRE DR. KITCHENER, ON. N2B 3V9 | P: 519.576.3650 F: 519.576.0121 | WWW.MHBCPLAN.COM

NDMNR Approval Stamp

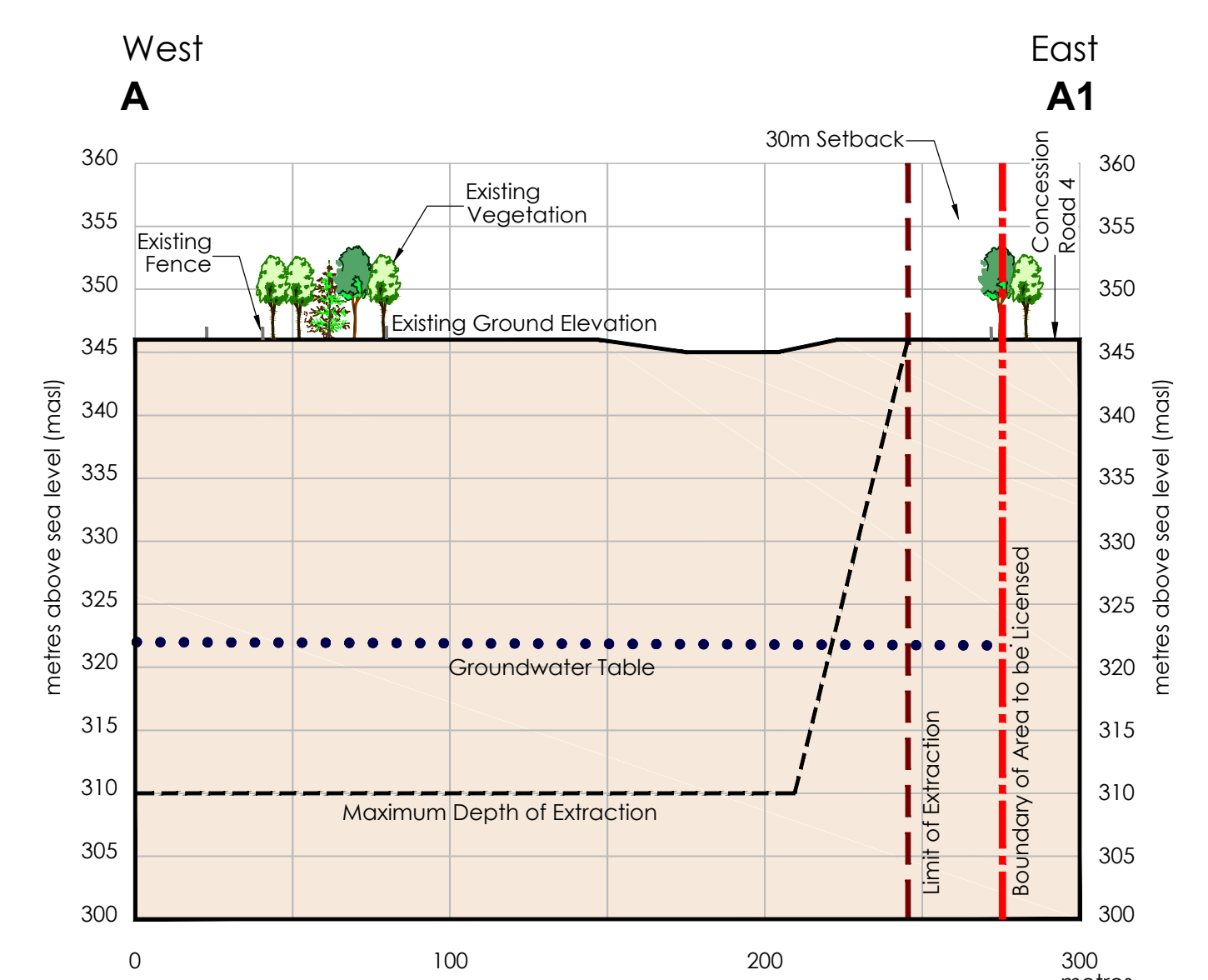
Stamp: *Carlin M. Post*

LAFARGE Building better cities™

Applicant's Signature: *Chris Galway*
Chris Galway
Senior Land Manager - East Central Ontario
Lafarge Canada Inc.

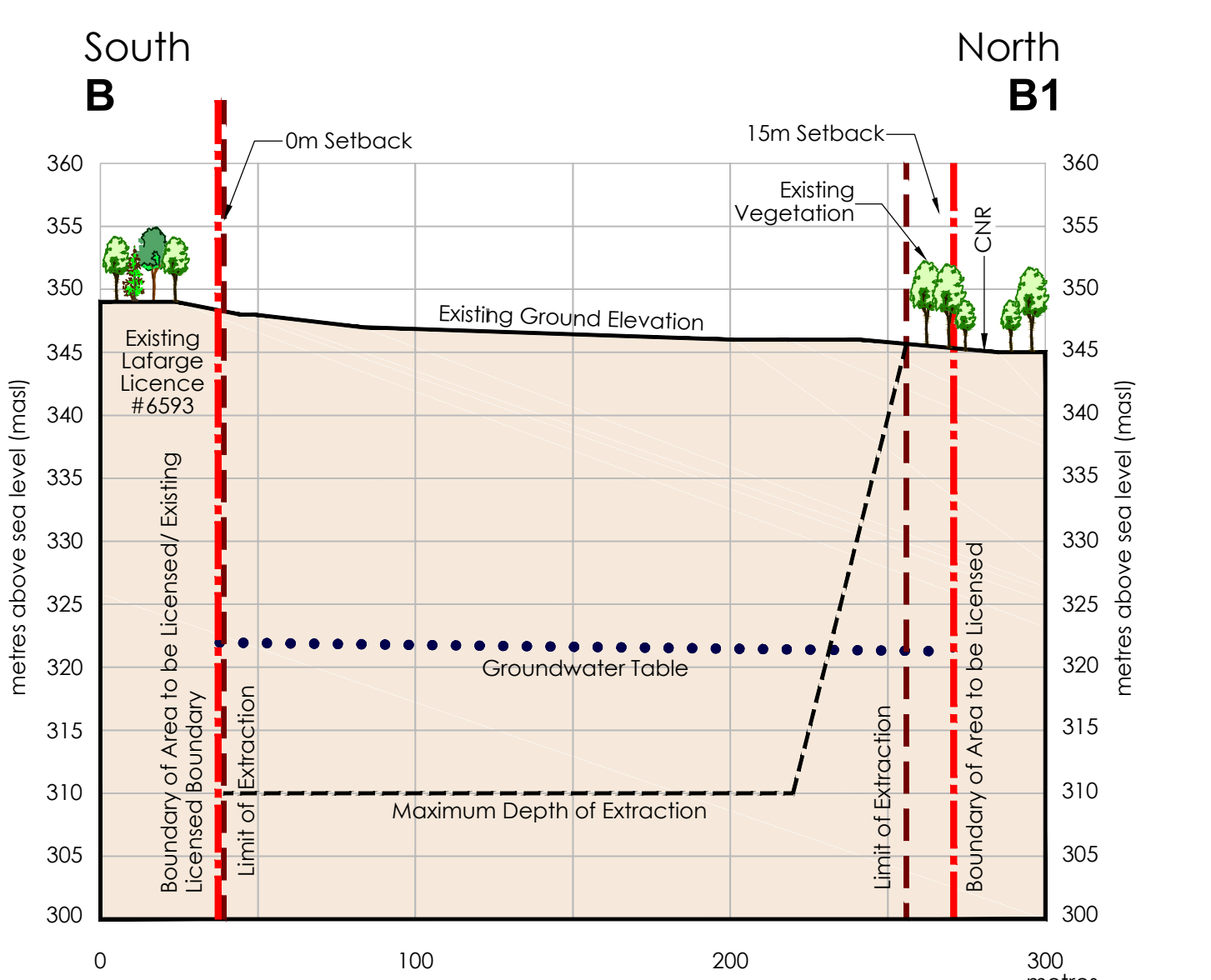
Project
Goodwood Pit Extension
Lafarge Canada Inc.
6509 Airport Road, Mississauga Ontario, L4V 1S7
Tel: (905) 738-7732 Fax: (905) 738-7092

NDMNR Licence Reference No.	Pre-approval review:
Plan Scale: See Plan	Plot Scale: 1:2.0 [1mm = 2.0 units] MODEL
Drawn By: D.G.S.	File No. 9526HC
Checked By: C.P.	



Section A-A1 - Existing Conditions

Horizontal Scale 1:2,000
Vertical Exaggeration 4x



Section B-B1 - Existing Conditions

K:\9526HC-Lafarge-Goodwood Pit Extension-Uxbridge\A\Explan 1of3 January2022.dwg

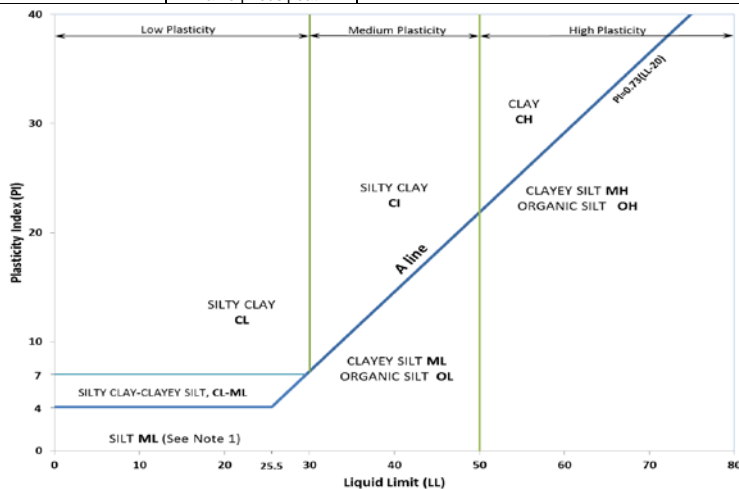
APPENDIX B

Borehole Logs

METHOD OF SOIL CLASSIFICATION

The WSP Canada Inc. Soil Classification System is based on the Unified Soil Classification System (USCS)

Organic or Inorganic	Soil Group	Type of Soil	Gradation or Plasticity	$C_u = \frac{D_{60}}{D_{10}}$	$C_c = \frac{(D_{30})^2}{D_{10} \times D_{60}}$	Organic Content	USCS Group Symbol	Group Name			
INORGANIC (Organic Content ≤30% by mass)	COARSE-GRAINED SOILS (>50% by mass is larger than 0.075 mm)	GRAVELS (>50% by mass of coarse fraction is larger than 4.75 mm)	Poorly Graded	<4	≤1 or ≥3	≤30%	GP	GRAVEL			
			Well Graded	≥4	1 to 3		GW	GRAVEL			
			Below A Line	n/a			GM	SILTY GRAVEL			
			Above A Line	n/a			GC	CLAYEY GRAVEL			
		SANDS (≥50% by mass of coarse fraction is smaller than 4.75 mm)	Poorly Graded	<6	≤1 or ≥3		SP	SAND			
			Well Graded	≥6	1 to 3		SW	SAND			
			Below A Line	n/a			SM	SILTY SAND			
			Above A Line	n/a			SC	CLAYEY SAND			
			Laboratory Tests		Field Indicators			Organic Content	USCS Group Symbol	Primary Name	
					Dilatancy		Dry Strength				Shine Test
INORGANIC (Organic Content ≤30% by mass)	FINE-GRAINED SOILS (≥50% by mass is smaller than 0.075 mm)	SILTS (Non-Plastic or PI and LL plot below A-Line on Plasticity Chart below)	Liquid Limit	Rapid	None	None	>6 mm	N/A (can't roll 3 mm thread)	<5%	ML	SILT
			<50	Slow	None to Low	Dull	3mm to 6 mm	None to low	<5%	ML	CLAYEY SILT
				Slow to very slow	Low to medium	Dull to slight	3mm to 6 mm	Low	5% to 30%	OL	ORGANIC SILT
			Liquid Limit ≥50	Slow to very slow	Low to medium	Slight	3mm to 6 mm	Low to medium	<5%	MH	CLAYEY SILT
		None		Medium to high	Dull to slight	1 mm to 3 mm	Medium to high	5% to 30%	OH	ORGANIC SILT	
		CLAYS (PI and LL plot above A-Line on Plasticity Chart below)	Liquid Limit <30	None	Low to medium	Slight to shiny	~ 3 mm	Low to medium	0% to 30% (see Note 2)	CL	SILTY CLAY
			Liquid Limit 30 to 50	None	Medium to high	Slight to shiny	1 mm to 3 mm	Medium		CI	SILTY CLAY
			Liquid Limit ≥50	None	High	Shiny	<1 mm	High		CH	CLAY
		HIGHLY ORGANIC SOILS (Organic Content >30% by mass)	Peat and mineral soil mixtures						30% to 75%	PT	SILTY PEAT, SANDY PEAT
			Predominantly peat, may contain some mineral soil, fibrous or amorphous peat						75% to 100%		PEAT



Note 1 – Fine grained materials with PI and LL that plot in this area are named (ML) SILT with slight plasticity. Fine-grained materials which are non-plastic (i.e. a PL cannot be measured) are named SILT.
 Note 2 – For soils with <5% organic content, include the descriptor “trace organics” for soils with between 5% and 30% organic content include the prefix “organic” before the Primary name.

Dual Symbol — A dual symbol is two symbols separated by a hyphen, for example, GP-GM, SW-SC and CL-ML. For non-cohesive soils, the dual symbols must be used when the soil has between 5% and 12% fines (i.e. to identify transitional material between “clean” and “dirty” sand or gravel. For cohesive soils, the dual symbol must be used when the liquid limit and plasticity index values plot in the CL-ML area of the plasticity chart (see Plasticity Chart at left).

Borderline Symbol — A borderline symbol is two symbols separated by a slash, for example, CL/CI, GM/SM, CL/ML. A borderline symbol should be used to indicate that the soil has been identified as having properties that are on the transition between similar materials. In addition, a borderline symbol may be used to indicate a range of similar soil types within a stratum.

ABBREVIATIONS AND TERMS USED ON RECORDS OF BOREHOLES AND TEST PITS

PARTICLE SIZES OF CONSTITUENTS

Soil Constituent	Particle Size Description	Millimetres	Inches (US Std. Sieve Size)
BOULDERS	Not Applicable	>300	>12
COBBLES	Not Applicable	75 to 300	3 to 12
GRAVEL	Coarse	19 to 75	0.75 to 3
	Fine	4.75 to 19	(4) to 0.75
SAND	Coarse	2.00 to 4.75	(10) to (4)
	Medium	0.425 to 2.00	(40) to (10)
	Fine	0.075 to 0.425	(200) to (40)
SILT/CLAY	Classified by plasticity	<0.075	< (200)

MODIFIERS FOR SECONDARY AND MINOR CONSTITUENTS

Percentage by Mass	Modifier
>35	Use 'and' to combine major constituents (i.e., SAND and GRAVEL)
> 12 to 35	Primary soil name prefixed with "gravelly, sandy, SILTY, CLAYEY" as applicable
> 5 to 12	some
≤ 5	trace

PENETRATION RESISTANCE

Standard Penetration Resistance (SPT), N:

The number of blows by a 63.5 kg (140 lb) hammer dropped 760 mm (30 in.) required to drive a 50 mm (2 in.) split-spoon sampler for a distance of 300 mm (12 in.). Values reported are as recorded in the field and are uncorrected.

Cone Penetration Test (CPT)

An electronic cone penetrometer with a 60° conical tip and a project end area of 10 cm² pushed through ground at a penetration rate of 2 cm/s. Measurements of tip resistance (q_t), porewater pressure (u) and sleeve frictions are recorded electronically at 25 mm penetration intervals.

Dynamic Cone Penetration Resistance (DCPT); N_d:

The number of blows by a 63.5 kg (140 lb) hammer dropped 760 mm (30 in.) to drive uncased a 50 mm (2 in.) diameter, 60° cone attached to "A" size drill rods for a distance of 300 mm (12 in.).

- PH:** Sampler advanced by hydraulic pressure
PM: Sampler advanced by manual pressure
WH: Sampler advanced by static weight of hammer
WR: Sampler advanced by weight of sampler and rod

SAMPLES

AS	Auger sample
BS	Block sample
CS	Chunk sample
DD	Diamond Drilling
DO or DP	Seamless open ended, driven or pushed tube sampler – note size
DS	Denison type sample
GS	Grab Sample
MC	Modified California Samples
MS	Modified Shelby (for frozen soil)
RC	Rock core
SC	Soil core
SS	Split spoon sampler – note size
ST	Slotted tube
TO	Thin-walled, open – note size (Shelby tube)
TP	Thin-walled, piston – note size (Shelby tube)
WS	Wash sample

SOIL TESTS

w	water content
PL, w _p	plastic limit
LL, w _L	liquid limit
C	consolidation (oedometer) test
CHEM	chemical analysis (refer to text)
CID	consolidated isotropically drained triaxial test ¹
CIU	consolidated isotropically undrained triaxial test with porewater pressure measurement ¹
D _R	relative density (specific gravity, G _s)
DS	direct shear test
GS	specific gravity
M	sieve analysis for particle size
MH	combined sieve and hydrometer (H) analysis
MPC	Modified Proctor compaction test
SPC	Standard Proctor compaction test
OC	organic content test
SO ₄	concentration of water-soluble sulphates
UC	unconfined compression test
UU	unconsolidated undrained triaxial test
V (FV)	field vane (LV-laboratory vane test)
γ	unit weight

1. Tests anisotropically consolidated prior to shear are shown as CAD, CAU.

NON-COHESIVE (COHESIONLESS) SOILS

Compactness²

Term	SPT 'N' (blows/0.3m) ¹
Very Loose	0 to 4
Loose	4 to 10
Compact	10 to 30
Dense	30 to 50
Very Dense	>50

- SPT 'N' in accordance with ASTM D1586, uncorrected for the effects of overburden pressure.
- Definition of compactness terms are based on SPT 'N' ranges as provided in Terzaghi, Peck and Mesri (1996). Many factors affect the recorded SPT 'N' value, including hammer efficiency (which may be greater than 60% in automatic trip hammers), overburden pressure, groundwater conditions, and grain size. As such, the recorded SPT 'N' value(s) should be considered only an approximate guide to the soil compactness. These factors need to be considered when evaluating the results, and the stated compactness terms should not be relied upon for design or construction.

Field Moisture Condition

Term	Description
Dry	Soil flows freely through fingers.
Moist	Soils are darker than in the dry condition and may feel cool.
Wet	As moist, but with free water forming on hands when handled.

COHESIVE SOILS

Consistency

Term	Undrained Shear Strength (kPa)	SPT 'N' ^{1,2} (blows/0.3m)
Very Soft	<12	0 to 2
Soft	12 to 25	2 to 4
Firm	25 to 50	4 to 8
Stiff	50 to 100	8 to 15
Very Stiff	100 to 200	15 to 30
Hard	>200	>30

- SPT 'N' in accordance with ASTM D1586, uncorrected for overburden pressure effects; approximate only.
- SPT 'N' values should be considered ONLY an approximate guide to consistency; for sensitive clays (e.g., Champlain Sea clays), the N-value approximation for consistency terms does NOT apply. Rely on direct measurement of undrained shear strength or other manual observations.

Water Content

Term	Description
w < PL	Material is estimated to be drier than the Plastic Limit.
w ~ PL	Material is estimated to be close to the Plastic Limit.
w > PL	Material is estimated to be wetter than the Plastic Limit.

LIST OF SYMBOLS

Unless otherwise stated, the symbols employed in the report are as follows:

I. GENERAL

π	3.1416
$\ln x$	natural logarithm of x
$\log_{10} x$	x or log x, logarithm of x to base 10
g	acceleration due to gravity
t	time

II. STRESS AND STRAIN

γ	shear strain
Δ	change in, e.g. in stress: $\Delta \sigma$
ε	linear strain
ε_v	volumetric strain
η	coefficient of viscosity
ν	Poisson's ratio
σ	total stress
σ'	effective stress ($\sigma' = \sigma - u$)
σ'_{vo}	initial effective overburden stress
$\sigma_1, \sigma_2, \sigma_3$	principal stress (major, intermediate, minor)
σ_{oct}	mean stress or octahedral stress $= (\sigma_1 + \sigma_2 + \sigma_3)/3$
τ	shear stress
u	porewater pressure
E	modulus of deformation
G	shear modulus of deformation
K	bulk modulus of compressibility

III. SOIL PROPERTIES

(a) Index Properties

$\rho(\gamma)$	bulk density (bulk unit weight)*
$\rho_d(\gamma_d)$	dry density (dry unit weight)
$\rho_w(\gamma_w)$	density (unit weight) of water
$\rho_s(\gamma_s)$	density (unit weight) of solid particles
γ'	unit weight of submerged soil ($\gamma' = \gamma - \gamma_w$)
D_R	relative density (specific gravity) of solid particles ($D_R = \rho_s / \rho_w$) (formerly G_s)
e	void ratio
n	porosity
S	degree of saturation

(a) Index Properties (continued)

w	water content
w_l or LL	liquid limit
w_p or PL	plastic limit
I_p or PI	plasticity index = $(w_l - w_p)$
NP	non-plastic
w_s	shrinkage limit
I_L	liquidity index = $(w - w_p) / I_p$
I_C	consistency index = $(w_l - w) / I_p$
e_{max}	void ratio in loosest state
e_{min}	void ratio in densest state
I_D	density index = $(e_{max} - e) / (e_{max} - e_{min})$ (formerly relative density)

(b) Hydraulic Properties

h	hydraulic head or potential
q	rate of flow
v	velocity of flow
i	hydraulic gradient
k	hydraulic conductivity (coefficient of permeability)
j	seepage force per unit volume

(c) Consolidation (one-dimensional)

C_c	compression index (normally consolidated range)
C_r	recompression index (over-consolidated range)
C_s	swelling index
C_α	secondary compression index
m_v	coefficient of volume change
C_v	coefficient of consolidation (vertical direction)
C_h	coefficient of consolidation (horizontal direction)
T_v	time factor (vertical direction)
U	degree of consolidation
σ'_p	pre-consolidation stress
OCR	over-consolidation ratio = σ'_p / σ'_{vo}

(d) Shear Strength

τ_p, τ_r	peak and residual shear strength
ϕ'	effective angle of internal friction
δ	angle of interface friction
μ	coefficient of friction = $\tan \delta$
c'	effective cohesion
c_u, s_u	undrained shear strength ($\phi = 0$ analysis)
p	mean total stress $(\sigma_1 + \sigma_3)/2$
p'	mean effective stress $(\sigma'_1 + \sigma'_3)/2$
q	$(\sigma_1 - \sigma_3)/2$ or $(\sigma'_1 - \sigma'_3)/2$
q_u	compressive strength $(\sigma_1 - \sigma_3)$
S_t	sensitivity

* Density symbol is ρ . Unit weight symbol is γ where $\gamma = \rho g$ (i.e. mass density multiplied by acceleration due to gravity)

Notes: 1
2

$$\tau = c' + \sigma' \tan \phi'$$

$$\text{shear strength} = (\text{compressive strength})/2$$

PROJECT: 1894152
 LOCATION: N 4879757.60; E 645890.30

RECORD OF BOREHOLE: MW18-01

SHEET 1 OF 2
 DATUM: Geodetic

BORING DATE: April 11, 2018

SPT/DCPT HAMMER: MASS, 64kg; DROP, 760mm

HAMMER TYPE: AUTOMATIC

DEPTH SCALE METRES	BORING METHOD	SOIL PROFILE		SAMPLES			DYNAMIC PENETRATION RESISTANCE, BLOWS/0.3m				HYDRAULIC CONDUCTIVITY, k, cm/s				ADDITIONAL LAB. TESTING	PIEZOMETER OR STANDPIPE INSTALLATION	
		DESCRIPTION	STRATA PLOT	ELEV. DEPTH (m)	NUMBER	TYPE	BLOWS/0.3m	SHEAR STRENGTH				WATER CONTENT PERCENT					
								20 40 60 80		nat V. + Q - rem V. ⊕ U - ⊙		10 ⁻¹⁰ 10 ⁻⁸ 10 ⁻⁶ 10 ⁻⁴		Wp ----- W ----- Wi			
0		GROUND SURFACE		343.74													
		TOPSOIL, trace rootlets; brown; non-cohesive, moist		0.00 343.28	1	DO									Casing		
		SAND and GRAVEL, coarse, trace silt; brown; non-cohesive, moist		0.46 342.67	2	DO									#2 Silica Sand		
		CLAYEY SILT, some sand, trace gravel; light brown; non-cohesive, moist		1.22 341.91	3	DO											
		CLAYEY SILT, trace sand, trace gravel; light brown; non-cohesive, moist		1.83	4	DO											
2		SILTY CLAY, trace sand; light brown; cohesive, w-PL		340.69	5	DO											
				3.05	6	DO											
		SAND and GRAVEL, coarse, trace silt; light brown; non-cohesive, moist		340.69	7	DO											
4					8	DO											
				338.25													
		SAND and GRAVEL, coarse, trace silt; dark brown to light brown; non-cohesive, moist		5.49 336.73	9	DO											
6					10	DO											
		SAND and GRAVEL, coarse; dark brown to light brown; non-cohesive, moist		7.01 335.21	10	DO											
8					11	DO											
		SAND and GRAVEL, coarse; dark brown; non-cohesive, moist		8.53 333.68	11	DO											
10					12	DO											
		SAND and GRAVEL, coarse; brown to light brown; non-cohesive, moist		10.06 332.16	12	DO									Bentonite		
12					13	DO											
		SAND, fine, trace silt; light brown; non-cohesive, moist		11.58 328.06	13	DO											
14					14	DO											
				17.68													
18		SAND, fine; light brown; non-cohesive, moist		17.68	14	DO											
20																	
		CONTINUED NEXT PAGE															

GTA-BHS 001 S:\CLIENTS\LA\FARGE\HOLCIMON_LUXBRIDGE_4TH\CONCESSION_4900\02_DATA\GINT\1894152.GPJ GAL-MIS.GDT 7/21/21

DEPTH SCALE
 1 : 100



LOGGED: JC
 CHECKED:

PROJECT: 1894152
 LOCATION: N 4879757.60; E 645890.30

RECORD OF BOREHOLE: MW18-01

SHEET 2 OF 2
 DATUM: Geodetic

BORING DATE: April 11, 2018

SPT/DCPT HAMMER: MASS, 64kg; DROP, 760mm

HAMMER TYPE: AUTOMATIC

DEPTH SCALE METRES	BORING METHOD	SOIL PROFILE		SAMPLES		DYNAMIC PENETRATION RESISTANCE, BLOWS/0.3m				HYDRAULIC CONDUCTIVITY, k, cm/s				ADDITIONAL LAB. TESTING	PIEZOMETER OR STANDPIPE INSTALLATION	
		DESCRIPTION	STRATA PLOT	ELEV. DEPTH (m)	NUMBER	TYPE	SHEAR STRENGTH Cu, kPa				WATER CONTENT PERCENT					
							20	40	60	80	nat V. rem V.	+ ⊕	- ⊖			Q - U
20		-- CONTINUED FROM PREVIOUS PAGE --														
		SAND, fine; light brown; non-cohesive, moist		323.01	14	DO										
		SAND, fine, trace silt; brown; non-cohesive, moist		20.73	15	DO										
		SAND, fine; light brown; non-cohesive, moist		322.40												
				21.34												
22					16	DO										
				319.97												
24		SAND and GRAVEL, coarse; brown; non-cohesive, moist to wet		23.77												
26																
					17	DO										
28																
30		SAND and GRAVEL, coarse; dark brown; non-cohesive, wet		313.87												
				29.87												
32																
34																
36																
					19	DO										
				306.86												
				36.88												
		END OF BOREHOLE														
		NOTES:														
38		1. Groundwater measured in open borehole at a depth of 24.5 m below ground surface upon completion of drilling.														
40		2. Groundwater measured in monitoring well at a depth of 23.6 m below ground surface on May 22, 2018.														

GTA-BHS 001 S:\CLIENTS\LAFARGE\HOLCIMON_UXBRIDGE_4THCONCESSION_4900\02_DATA\GINT\1894152.GPJ GAL-MIS.GDT 7/21/21

DEPTH SCALE
 1 : 100



LOGGED: JC
 CHECKED:

PROJECT: 1894152
 LOCATION: N 4879444.70; E 645868.30

RECORD OF BOREHOLE: MW18-02

SHEET 2 OF 3
 DATUM: Geodetic

BORING DATE: April 10, 2018

SPT/DCPT HAMMER: MASS, 64kg; DROP, 760mm

HAMMER TYPE: AUTOMATIC

DEPTH SCALE METRES	BORING METHOD	SOIL PROFILE		SAMPLES		DYNAMIC PENETRATION RESISTANCE, BLOWS/0.3m				HYDRAULIC CONDUCTIVITY, k, cm/s				ADDITIONAL LAB. TESTING	PIEZOMETER OR STANDPIPE INSTALLATION		
		DESCRIPTION	STRATA PLOT	ELEV. DEPTH (m)	NUMBER	TYPE	BLOWS/0.3m	SHEAR STRENGTH Cu, kPa				WATER CONTENT PERCENT					
								20	40	60	80	nat V. +	rem V. ⊕			Q - ●	U - ○
20		-- CONTINUED FROM PREVIOUS PAGE --															
		SAND, coarse; dark brown to light brown; non-cohesive, moist		325.70	14	DO											
		SAND and GRAVEL, coarse; dark brown to light brown and grey; non-cohesive, moist		20.73													
22					15	DO											
				322.66													
24		SAND, coarse; brown; non-cohesive, moist		23.77													
					16	DO											
26				319.61													
		SAND and GRAVEL, coarse; dark brown to brown; non-cohesive, moist		26.82													
28					17	DO											
				316.56													
30		GRAVEL, coarse, trace sand; brown; non-cohesive, moist		29.87													
				315.95													
		SAND, fine to coarse, trace gravel; dark brown to light brown; non-cohesive, moist to wet		30.48													
32					18	DO											
				310.46													
34				35.97													
		SAND to SAND and GRAVEL, coarse; brown to grey; non-cohesive, wet			19	DO											
36				308.33													
				38.10													
38		END OF BOREHOLE															
40		NOTES: 1. Groundwater measured in open borehole at a depth of 25.9 m below ground surface upon completion of drilling.															
		CONTINUED NEXT PAGE															

GTA-BHS 001 S:\CLIENTS\LA\FARGE\HOLCIMON_LXBRIDGE_4THCONCESSION_4900\02_DATA\GINT\1894152.GPJ GAL-MIS.GDT 7/21/21

DEPTH SCALE
1 : 100



LOGGED: JC
CHECKED:

PROJECT: 1894152
 LOCATION: N 4879444.70; E 645868.30

RECORD OF BOREHOLE: MW18-02

SHEET 3 OF 3
 DATUM: Geodetic

BORING DATE: April 10, 2018

SPT/DCPT HAMMER: MASS, 64kg; DROP, 760mm

HAMMER TYPE: AUTOMATIC

DEPTH SCALE METRES	BORING METHOD	SOIL PROFILE		SAMPLES		DYNAMIC PENETRATION RESISTANCE, BLOWS/0.3m				HYDRAULIC CONDUCTIVITY, k, cm/s				ADDITIONAL LAB. TESTING	PIEZOMETER OR STANDPIPE INSTALLATION
		DESCRIPTION	STRATA PLOT	ELEV. DEPTH (m)	NUMBER	TYPE	BLOWS/0.3m	SHEAR STRENGTH Cu, kPa		WATER CONTENT PERCENT		WATER CONTENT PERCENT			
								20	40	60	80	nat V. +	rem V. ⊕		
40		-- CONTINUED FROM PREVIOUS PAGE --													
42		2. Groundwater measured in monitoring well at a depth of 25.0 m below ground surface on May 22, 2018.													
44															
46															
48															
50															
52															
54															
56															
58															
60															

GTA-BHS 001 S:\CLIENTS\LA\FARGE\HOLCIMON_LXBRIDGE_4THCONCESSION_4900\02_DATA\GINT\1894152.GPJ GAL-MIS.GDT 7/21/21

PROJECT: 1894152
 LOCATION: N 4879384.90; E 645314.80

RECORD OF BOREHOLE: MW18-03

SHEET 1 OF 2
 DATUM: Geodetic

BORING DATE: April 12, 2018

SPT/DCPT HAMMER: MASS, 64kg; DROP, 760mm

HAMMER TYPE: AUTOMATIC

DEPTH SCALE METRES	BORING METHOD	SOIL PROFILE		SAMPLES			DYNAMIC PENETRATION RESISTANCE, BLOWS/0.3m		HYDRAULIC CONDUCTIVITY, k, cm/s				ADDITIONAL LAB. TESTING	PIEZOMETER OR STANDPIPE INSTALLATION	
		DESCRIPTION	STRATA PLOT	ELEV. DEPTH (m)	NUMBER	TYPE	BLOWS/0.3m	SHEAR STRENGTH Cu, kPa		WATER CONTENT PERCENT					
								20	40	60	80	nat V. +			rem V. ⊕
0		GROUND SURFACE		346.63											
		TOPSOIL, trace rootlets, trace organics; dark brown; moist		0.00	1	DO									Casing
		SAND, fine, trace silt; brown to light brown; non-cohesive, moist		0.30	2	DO									#2 Silica Sand
		SAND, fine; light brown; non-cohesive, moist		345.41											
2				1.22	3	DO									
					4	DO									
					5	DO									
6		SAND, fine, trace silt; light brown; non-cohesive, moist		341.14											
				5.49											
					6	DO									
12		SAND and GRAVEL, coarse, trace cobbles; dark brown; non-cohesive, moist		335.05											
		SAND and GRAVEL, coarse; dark brown to light brown; non-cohesive, moist		11.58	7	DO									
				11.89											
					8	DO									
18		SAND and GRAVEL, coarse; dark brown to light brown; non-cohesive, moist		328.95											
				17.68											
					9	DO									
20															
		CONTINUED NEXT PAGE													

GTA-BHS 001 S:\CLIENTS\LAFARGE\HOLCIMON_LUXBRIDGE_4THCONCESSION_4900\02 DATA\GINT\1894152.GPJ GAL-MIS.GDT 7/21/21

DEPTH SCALE
 1 : 100



LOGGED: JC
 CHECKED:

PROJECT: 1894152
 LOCATION: N 4879384.90; E 645314.80

RECORD OF BOREHOLE: MW18-03

SHEET 2 OF 2
 DATUM: Geodetic

BORING DATE: April 12, 2018

SPT/DCPT HAMMER: MASS, 64kg; DROP, 760mm

HAMMER TYPE: AUTOMATIC

DEPTH SCALE METRES	BORING METHOD	SOIL PROFILE		SAMPLES		DYNAMIC PENETRATION RESISTANCE, BLOWS/0.3m				HYDRAULIC CONDUCTIVITY, k, cm/s				ADDITIONAL LAB. TESTING	PIEZOMETER OR STANDPIPE INSTALLATION		
		DESCRIPTION	STRATA PLOT	ELEV. DEPTH (m)	NUMBER	TYPE	BLOWS/0.3m	SHEAR STRENGTH				WATER CONTENT PERCENT					
								20 40 60 80		nat V. + Q - rem V. ⊕ U - ⊙		10 ⁻¹⁰ 10 ⁻⁸ 10 ⁻⁶ 10 ⁻⁴				Wp	
20		-- CONTINUED FROM PREVIOUS PAGE --															
		SAND and GRAVEL, coarse; dark brown to light brown; non-cohesive, moist		325.90	9	DO											
		SAND and GRAVEL, coarse; dark brown; non-cohesive, moist		20.73	10	DO											
22		SAND and GRAVEL, coarse; light brown; non-cohesive, moist		324.38	11	DO											
		SAND, coarse, some gravel, trace silt; light brown; non-cohesive, moist		322.86	12	DO											
		SAND, coarse, trace gravel; brown to light brown and grey; non-cohesive, moist		319.81	13	DO											
		SAND, fine, trace gravel, trace silt; grey; non-cohesive, moist		317.67	14	DO											
30		No Sample Collected		316.76													
		No Sample Collected		29.87													
32					15	DO											
36		No Sample Collected		310.66													
		No Sample Collected		35.97													
				309.75	16	DO											
		END OF BOREHOLE		36.88													
38		NOTES: 1. Groundwater measured in open borehole at a depth of 26.1 m below ground surface upon completion of drilling. 2. Groundwater measured in monitoring well at a depth of 25.3 m below ground surface on May 22, 2018.															

GTA-BHS 001 S:\CLIENTS\LAFARGE\HOLCIMON_UXBRIDGE_4THCONCESSION_4900\02_DATA\GINT\1894152.GPJ GAL-MIS.GDT 7/21/21

DEPTH SCALE
 1 : 100



LOGGED: JC
 CHECKED:



Ministry
of the
Environment
Ontario

The Ontario Water Resources Act

WATER WELL RECORD

1. PRINT ONLY IN SPACES PROVIDED
2. CHECK CORRECT BOX WHERE APPLICABLE

11

1909612

MUNICIP 19,012

CON. CON

103

COUNTY OR DISTRICT Dufferin	TOWNSHIP, BOROUGH, CITY, TOWN, VILLAGE Uxbridge	CON. BLOCK TRACT SURVEY ETC 3	LOT 25-27 20
4900 CONC 4. 81 Goodwood, Ontario LOC 1A0		DATE COMPLETED DAY 20 MO 12 88	
RG 7,9,3,8,3	RC ELEVATION 345	RC BASIN CODE	II III IV

LOG OF OVERBURDEN AND BEDROCK MATERIALS (SEE INSTRUCTIONS)

GENERAL COLOUR	MOST COMMON MATERIAL	OTHER MATERIALS	GENERAL DESCRIPTION	DEPTH - FEET	
				FROM	TO
Brown	Clay	Sand	Layers	0	65
Brown	Gravel		Loose & Dry	65	85
Yellow	Clay	Strips of Sand	Packed	85	96
Brown	Sand		Clean & Fine	96	106

31

32

41 WATER RECORD

WATER FOUND AT - FEET	KIND OF WATER		
96'	1 <input checked="" type="checkbox"/> FRESH 2 <input checked="" type="checkbox"/> SALTY	3 <input type="checkbox"/> SULPHUR 4 <input type="checkbox"/> MINERALS 6 <input type="checkbox"/> GAS	
15-18	1 <input type="checkbox"/> FRESH 2 <input type="checkbox"/> SALTY	3 <input type="checkbox"/> SULPHUR 4 <input type="checkbox"/> MINERALS 6 <input type="checkbox"/> GAS	
20-23	1 <input type="checkbox"/> FRESH 2 <input type="checkbox"/> SALTY	3 <input type="checkbox"/> SULPHUR 4 <input type="checkbox"/> MINERALS 6 <input type="checkbox"/> GAS	
25-28	1 <input type="checkbox"/> FRESH 2 <input type="checkbox"/> SALTY	3 <input type="checkbox"/> SULPHUR 4 <input type="checkbox"/> MINERALS 6 <input type="checkbox"/> GAS	
30-33	1 <input type="checkbox"/> FRESH 2 <input type="checkbox"/> SALTY	3 <input type="checkbox"/> SULPHUR 4 <input type="checkbox"/> MINERALS 6 <input type="checkbox"/> GAS	

51 CASING & OPEN HOLE RECORD

INSIDE DIAM. INCHES	MATERIAL	WALL THICKNESS INCHES	DEPTH - FEET	
			FROM	TO
6 1/4"	STEEL GALVANIZED CONCRETE OPEN HOLE PLASTIC	.188	5'	96'
17-18	STEEL GALVANIZED CONCRETE OPEN HOLE PLASTIC			20-23
24-25	STEEL GALVANIZED CONCRETE OPEN HOLE PLASTIC			27-30

SCREEN

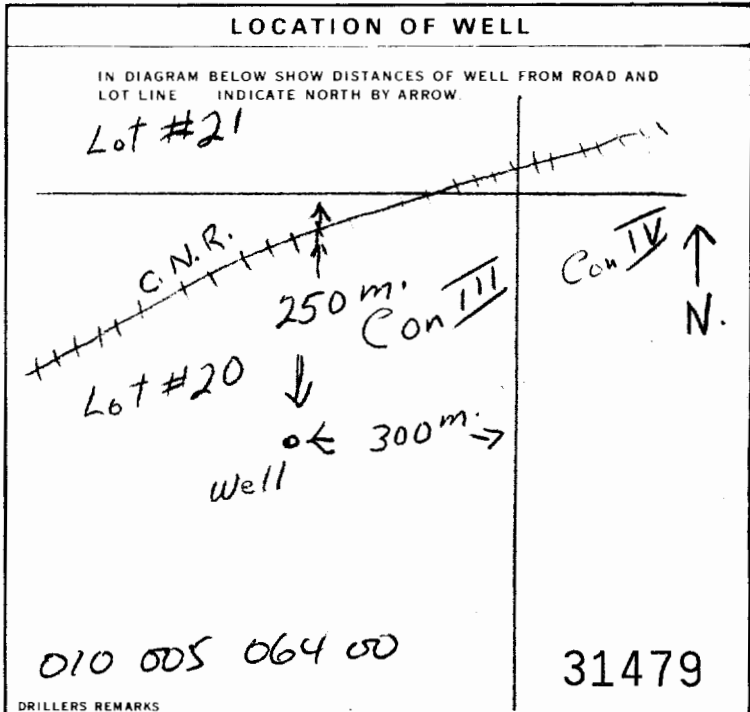
SIZE(S) OF OPENING (SLOT NO.) 8	DIAMETER 6 INCHES	LENGTH 10 FEET
MATERIAL AND TYPE S.S.		DEPTH TO TOP OF SCREEN 96 FEET

61 PLUGGING & SEALING RECORD

DEPTH SET AT - FEET	MATERIAL AND TYPE (CEMENT GROUT, LEAD PACKER, ETC.)
10-13	14-17
18-21	22-25
26-29	30-33

71 PUMPING TEST

PUMPING TEST METHOD 1 <input type="checkbox"/> PUMP 2 <input checked="" type="checkbox"/> BAILER	PUMPING RATE 12 GPM	DURATION OF PUMPING 2 HOURS 30 MINS
STATIC LEVEL 75 FEET	WATER LEVEL END OF PUMPING 86 FEET	WATER LEVELS DURING PUMPING 15 MINUTES: 86 FEET 30 MINUTES: 86 FEET 45 MINUTES: 86 FEET 60 MINUTES: 86 FEET
IF FLOWING, GIVE RATE	PUMP INTAKE SET AT 95 FEET	WATER AT END OF TEST 1 <input checked="" type="checkbox"/> CLEAR 2 <input type="checkbox"/> CLOUDY
RECOMMENDED PUMP TYPE 1 <input type="checkbox"/> SHALLOW 2 <input checked="" type="checkbox"/> DEEP	RECOMMENDED PUMP SETTING 95 FEET	RECOMMENDED PUMPING RATE 15 GPM



FINAL STATUS OF WELL

1 WATER SUPPLY
2 OBSERVATION WELL
3 TEST HOLE
4 RECHARGE WELL
5 ABANDONED - INSUFFICIENT SUPPLY
6 ABANDONED - POOR QUALITY
7 UNFINISHED
8 DEWATERING

WATER USE

1 DOMESTIC
2 STOCK
3 IRRIGATION
4 INDUSTRIAL
5 OTHER
6 COMMERCIAL
7 MUNICIPAL
8 PUBLIC SUPPLY
9 COOLING OR AIR CONDITIONING
10 NOT USED

METHOD OF CONSTRUCTION

1 CABLE TOOL
2 ROTARY (CONVENTIONAL)
3 ROTARY (REVERSE)
4 ROTARY (AIR)
5 AIR PERCUSSION
6 BORING
7 DIAMOND
8 JETTING
9 DRIVING
10 DIGGING
11 OTHER

CONTRACTOR

NAME OF WELL CONTRACTOR: **Sauder Well Drilling Ltd.**
WELL CONTRACTOR'S LICENCE NUMBER: **4743**
R.R. # 4 UXBRIDGE, Ontario LOC 1K0
NAME OF WELL TECHNICIAN: **Ab. Sauder**
WELL TECHNICIAN'S LICENCE NUMBER: **T-0241**
SIGNATURE OF TECHNICIAN/CONTRACTOR: *Ab. Sauder*
SUBMISSION DATE: DAY 30 MO 12 YR 88

OFFICE USE ONLY

DATA SOURCE: **4743**
CONTRACTOR: **4743**
DATE RECEIVED: **JAN 04 1989**
DATE OF INSPECTION: _____
INSPECTOR: _____
REMARKS: **WDE**

APPENDIX C

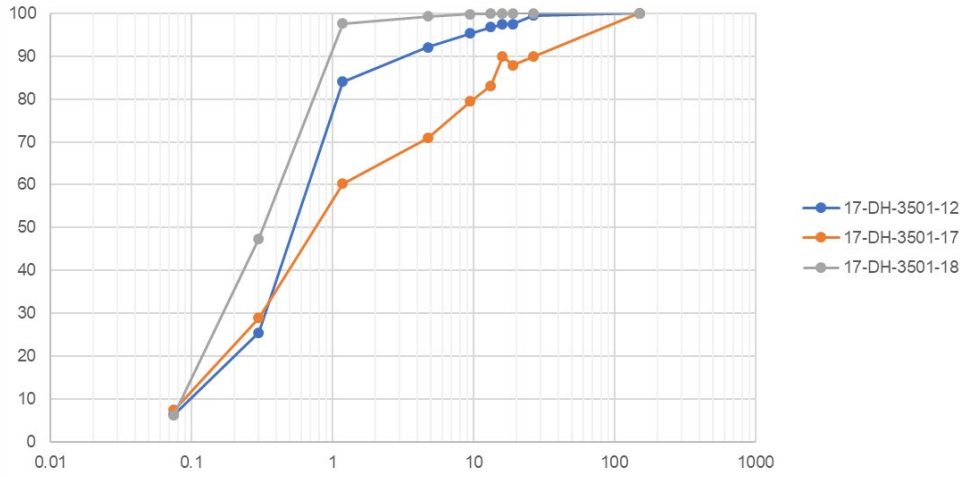
Grain Size Data

**21453907 LAFARGE GOODWOOD PIT EXTENSION: GRAIN SIZE ANALYSIS SUMMARY
BELOW WATER SAMPLES**

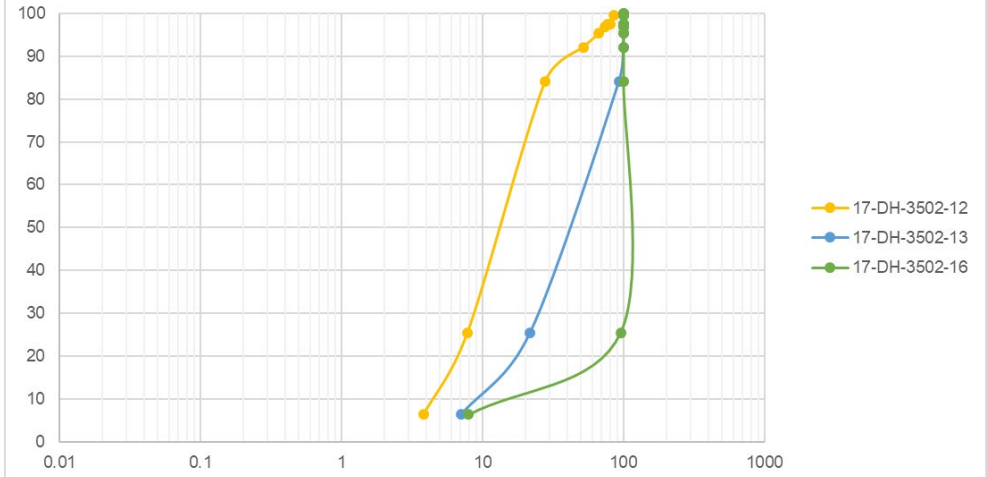
Sample	Depth To (mbgs)	d₁₀ (mm)	K (m/s)
17-DH-3501-12	19.51	0.1	1.E-04
17-DH-3501-17	32.92	0.09	8.E-05
17-DH-3501-18	35.05	0.085	7.E-05
17-DH-3502-12	21.95	0.38	1.E-03
17-DH-3502-13	23.17	0.13	2.E-04
17-DH-3502-16	28.65	0.079	6.E-05
17-DH-3502-19	33.22	0.12	1.E-04
17-DH-3503-12	27.13	0.15	2.E-04
17-DH-3503-13	28.04	0.28	8.E-04
17-DH-3503-14	30.18	0.11	1.E-04
17-DH-3503-15	31.39	0.19	4.E-04
17-DH-3503-16	33.22	0.10	1.E-04
17-DH-3504-14	22.25	0.26	7.E-04
17-DH-3504-16	24.99	0.25	6.E-04
17-DH-3504-17	27.13	0.22	5.E-04
17-DH-3505-13	27.13	0.09	8.E-05
17-DH-3505-14	28.35	0.09	8.E-05
17-DH-3505-15	30.18	0.16	3.E-04
17-DH-3505-16	31.7	0.15	2.E-04
17-DH-3507-15	28.35	0.11	1.E-04
17-DH-3507-16	30.18	0.16	3.E-04
17-DH-3507-17	33.22	0.12	1.E-04
17-DH-3507-20	-	0.09	8.E-05
17-DH-3508-12	25.91	0.19	4.E-04
17-DH-3508-13	26.82	0.36	1.E-03

<i>Geomean:</i>	<i>2E-04</i>
<i>Max:</i>	<i>1E-03</i>
<i>Min:</i>	<i>6E-05</i>

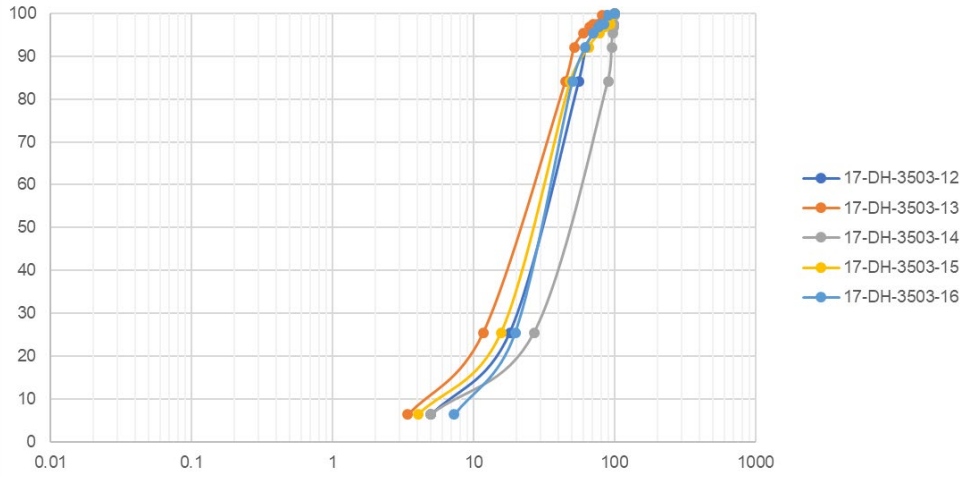
17-DH-3501 GRAIN SIZE SAMPLES BELOW WATER



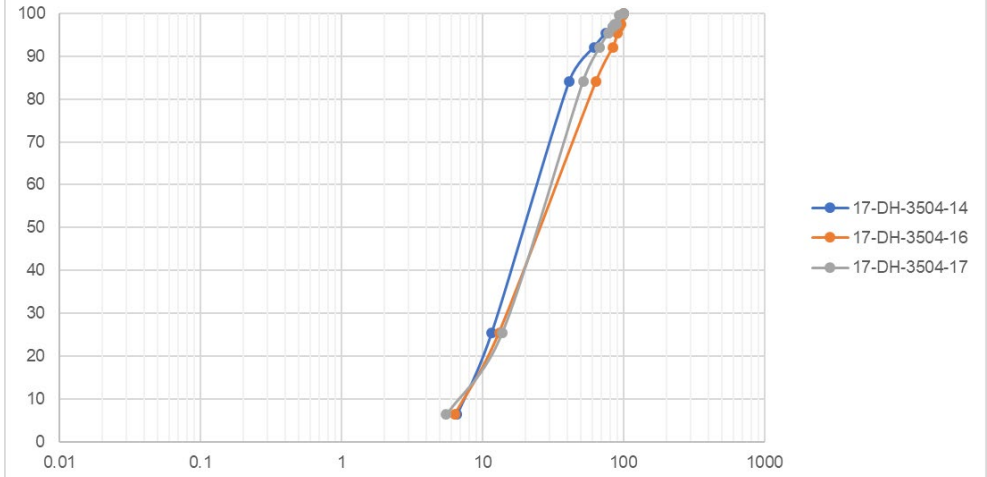
17-DH-3502 GRAIN SIZE SAMPLES BELOW WATER



17-DH-3503 GRAIN SIZE SAMPLES BELOW WATER



17-DH-3504 GRAIN SIZE SAMPLES BELOW WATER



LAFARGE GOODWOOD PIT EXTENSION: GRAIN SIZE ANALYSIS

GRAIN SIZE CURVES (07-DH-3501, 07-DH-3502, 07-DH-3503, 07-DH-3504)

SEPTEMBER 2019

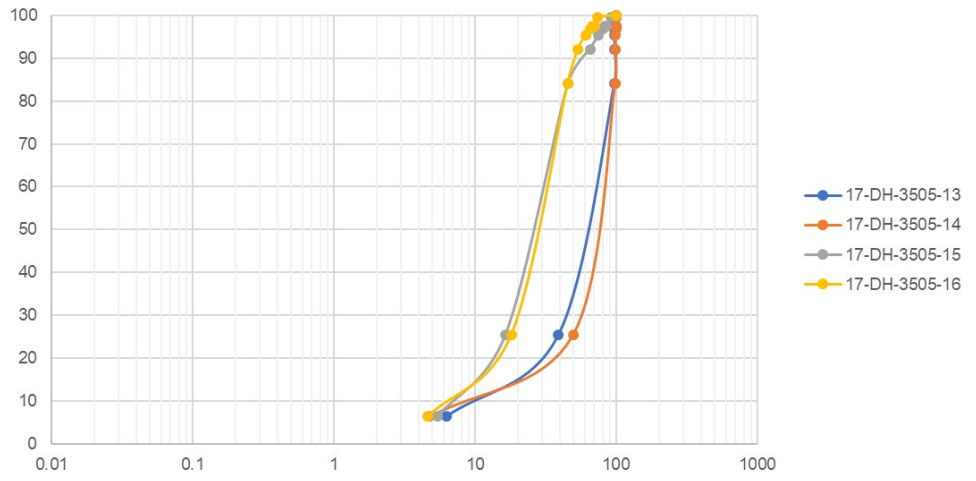
PROJECT: 1894152

FIGURE: C1

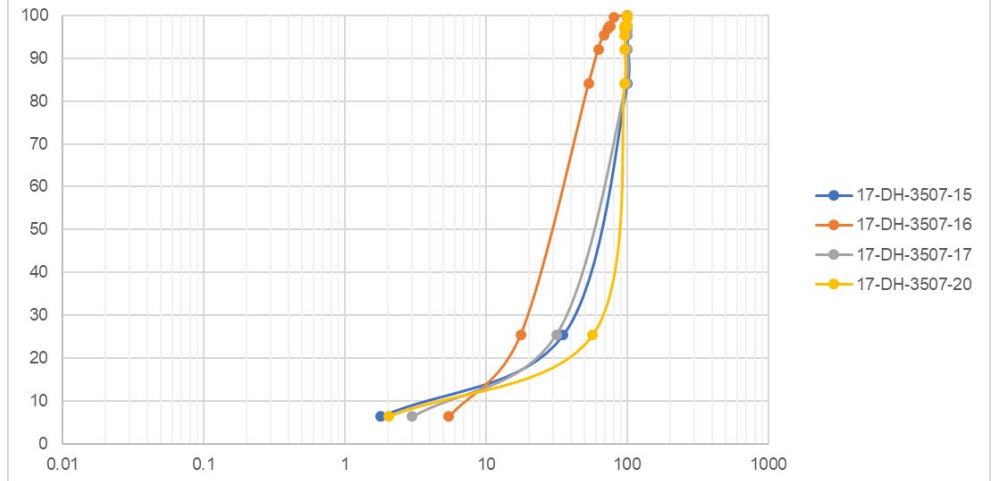


Golder Associates Ltd.
BARRIE, ONTARIO, CANADA

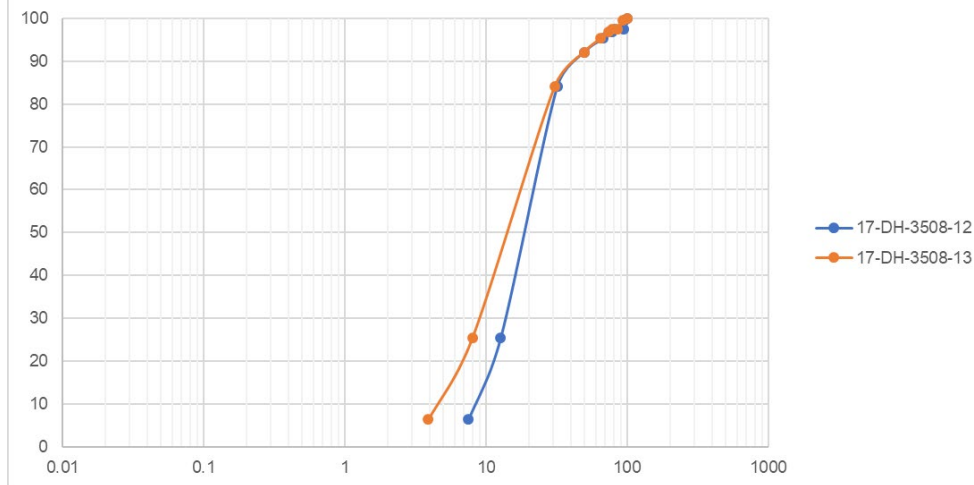
17-DH-3505 GRAIN SIZE SAMPLES BELOW WATER



17-DH-3507 GRAIN SIZE SAMPLES BELOW WATER



17-DH-3508 GRAIN SIZE SAMPLES BELOW WATER



LAFARGE GOODWOOD PIT EXTENSION: GRAIN SIZE ANALYSIS

GRAIN SIZE CURVES (07-DH-3505, 07-DH-3507, 07-DH-3508)

SEPTEMBER 2019

PROJECT: 1894152

FIGURE: C2



Golder Associates Ltd.
BARRIE, ONTARIO, CANADA

APPENDIX D

Water Quality Results

C.O.C.: G84846

REPORT No. B18-28353 (i)

Report To:

Golder Associates Ltd.
 121 Commerce Park Drive, Unit L,
 Barrie ON. L4N 8X1 Canada

Attention: Devin Hannan

Caduceon Environmental Laboratories

110 West Beaver Creek Rd Unit 14
 Richmond Hill ON L4B 1J9
 Tel: 289-475-5442
 Fax: 289-562-1963

DATE RECEIVED: 18-Sep-18

JOB/PROJECT NO.: Lafarge - Goodwood Pit

DATE REPORTED: 25-Sep-18

SAMPLE MATRIX: Groundwater

P.O. NUMBER: 1894152 (2000)

WATERWORKS NO.

Parameter	Qty	Site Analyzed	Analyst Initials	Date Analyzed	Lab Method	Reference Method
Alkalinity (as CaCO3)	3	Holly Lane	SYL	19-Sep-18	A-ALK-03 (o)	SM 2320B
Conductivity	3	Holly Lane	SYL	19-Sep-18	A-COND-02 (o)	SM 2510B
Anions	3	Holly Lane	VSC	19-Sep-18	A-IC-01 (o)	SM4110C
pH	3	Holly Lane	SYL	19-Sep-18	A-PH-01 (o)	SM 4500H
B - Bacteriological	3	Barrie	SAN	20-Sep-18	B-EC-001 (b)	SM9222B
Chromium (VI)	3	Holly Lane	JGC	21-Sep-18	D-CRVI-01 (o)	MOE E3056
Mercury	3	Holly Lane	PBK	24-Sep-18	D-HG-02 (o)	SM 3112 B
Metals - ICP-OES	3	Holly Lane	TPR	20-Sep-18	D-ICP-01 (o)	SM 3120
Metals - ICP-MS	3	Holly Lane	TPR	20-Sep-18	D-ICPMS-01 (o)	EPA 200.8
Calculation	3	Holly Lane	JGC	21-Sep-18	D-Ion Balance	Calc.

O. Reg. 153 - Soil, Ground Water and Sediment Standards
 Tbl. 2 - PGW - Table 2 - Potable Ground Water



R.L. = Reporting Limit

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Site Analyzed=K-Kingston,W-Windsor,O-Ottawa,R-Richmond Hill,B-Barrie

Christine Burke
 Lab Manager

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C.O.C.: G84846

REPORT No. B18-28353 (i)

Report To:

Golder Associates Ltd.
 121 Commerce Park Drive, Unit L,
 Barrie ON. L4N 8X1 Canada

Attention: Devin Hannan

Caduceon Environmental Laboratories

110 West Beaver Creek Rd Unit 14
 Richmond Hill ON L4B 1J9

Tel: 289-475-5442

Fax: 289-562-1963

DATE RECEIVED: 18-Sep-18

JOB/PROJECT NO.: Lafarge - Goodwood Pit

DATE REPORTED: 25-Sep-18

P.O. NUMBER: 1894152 (2000)

SAMPLE MATRIX: Groundwater

WATERWORKS NO.

Parameter	Client I.D.		MW 18-01	MW 18-02	PW	O. Reg. 153	
	Sample I.D.	Date Collected	B18-28353-1 17-Sep-18	B18-28353-2 17-Sep-18	B18-28353-3 17-Sep-18	Tbl. 2 - PGW	
	Units	R.L.					
E coli	cfu/100mL	1	2	< 2	< 2		
pH @25°C	pH Units		7.76	7.95	8.14		
Conductivity @25°C	mS/cm	0.001	1.05	0.515	0.46		
Alkalinity(CaCO3) to pH4.5	mg/L	5	233	179	168		
Carbonate (as CaCO3)	mg/L	5	< 5	< 5	< 5		
Bicarbonate(as CaCO3)	mg/L	5	233	179	168		
Hydroxide (as CaCO3)	mg/L	5	< 5	< 5	< 5		
Hardness (as CaCO3)	mg/L	1	359	248	223		
Chloride	µg/L	500	158000	25100	15100	790000	
Fluoride	µg/L	100	< 100	< 100	< 100		
Sodium	µg/L	200	84500	10900	7000	490000	
Antimony	µg/L	0.1	5.0	0.6	< 0.1	6	
Arsenic	µg/L	0.1	0.3	0.1	< 0.1	25	
Barium	µg/L	1	118	49	41	1000	
Beryllium	µg/L	0.1	< 0.1	< 0.1	< 0.1	4	
Boron	µg/L	5	27	7	< 5	5000	
Cadmium	µg/L	0.015	< 0.015	< 0.015	< 0.015	2.7	
Chromium	µg/L	2	< 2	< 2	< 2	50	
Chromium (VI)	µg/L	2	< 2	< 2	< 2	25	
Cobalt	µg/L	0.1	< 0.1	< 0.1	< 0.1	3.8	
Copper	µg/L	2	< 2	< 2	2	87	
Lead	µg/L	0.02	0.05	0.04	0.10	10	
Mercury	µg/L	0.02	< 0.02	< 0.02	< 0.02	0.29	

O. Reg. 153 - Soil, Ground Water and Sediment Standards
 Tbl. 2 - PGW - Table 2 - Potable Ground Water



Christine Burke
 Lab Manager

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Fax: 289-562-1963

DATE RECEIVED: 18-Sep-18

JOB/PROJECT NO.: Lafarge - Goodwood Pit

DATE REPORTED: 25-Sep-18

P.O. NUMBER: 1894152 (2000)

SAMPLE MATRIX: Groundwater

WATERWORKS NO.

Parameter	Client I.D. Sample I.D. Date Collected		MW 18-01 B18-28353-1 17-Sep-18	MW 18-02 B18-28353-2 17-Sep-18	PW B18-28353-3 17-Sep-18	O. Reg. 153 Tbl. 2 - PGW	
	Units	R.L.					
Molybdenum	µg/L	0.1	0.6	0.4	0.3	70	
Nickel	µg/L	10	< 10	< 10	< 10	100	
Selenium	µg/L	1	< 1	< 1	< 1	10	
Silver	µg/L	0.1	< 0.1	< 0.1	< 0.1	1.5	
Thallium	µg/L	0.05	< 0.05	< 0.05	< 0.05	2	
Uranium	µg/L	0.05	0.87	0.76	0.78	20	
Vanadium	µg/L	0.1	0.4	0.2	0.2	6.2	
Zinc	µg/L	5	< 5	< 5	12	1100	
Anion Sum	meq/L		10.2	5.08	4.56		
Cation Sum	meq/L		10.9	5.47	4.78		
% Difference	%		3.61	3.66	2.35		
Ion Ratio	AS/CS		0.930	0.929	0.954		
Sodium Adsorption Ratio	-		1.94	0.301	0.203		
TDS(ion sum calc.)	mg/L	1	570	277	246		
Conductivity (calc.)	µmho/cm		1050	516	456		
TDS(calc.)/EC(actual)	-		0.544	0.537	0.534		
EC(calc.)/EC(actual)	-		1.00	1.00	0.990		
Langelier Index(25°C)	S.I.		0.693	0.620	0.741		

1 Chromium (VI) result is based on total chromium

O. Reg. 153 - Soil, Ground Water and Sediment Standards
 Tbl. 2 - PGW - Table 2 - Potable Ground Water



Christine Burke
 Lab Manager

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Richmond Hill ON L4B 1J9

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DATE RECEIVED: 18-Sep-18

DATE REPORTED: 25-Sep-18

SAMPLE MATRIX: Groundwater

JOB/PROJECT NO.: Lafarge - Goodwood Pit

P.O. NUMBER: 1894152 (2000)

WATERWORKS NO.

Summary of Exceedances

O. Reg. 153 - Soil, Ground Water and Sediment Standards
Tbl. 2 - PGW - Table 2 - Potable Ground Water

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Christine Burke
Lab Manager

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 Fax: 289-562-1963

DATE RECEIVED: 18-Sep-18

JOB/PROJECT NO.: Lafarge - Goodwood Pit

DATE REPORTED: 25-Sep-18

SAMPLE MATRIX: Groundwater

P.O. NUMBER: 1894152 (2000)

WATERWORKS NO.

Parameter	Qty	Site Analyzed	Analyst Initials	Date Analyzed	Lab Method	Reference Method
PHC(F2-F4)	3	Kingston	KPR	20-Sep-18	C-PHC-W-001 (k)	MOE E3421
VOC's	3	Richmond Hill	FAL	19-Sep-18	C-VOC-02 (rh)	EPA 8260
PHC(F1)	3	Richmond Hill	FAL	19-Sep-18	C-VPHW-01 (rh)	MOE E3421

O. Reg. 153 - Soil, Ground Water and Sediment Standards
 Tbl. 2 - PGW - Table 2 - Potable Ground Water



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Christine Burke
 Lab Manager

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DATE RECEIVED: 18-Sep-18

JOB/PROJECT NO.: Lafarge - Goodwood Pit

DATE REPORTED: 25-Sep-18

P.O. NUMBER: 1894152 (2000)

SAMPLE MATRIX: Groundwater

WATERWORKS NO.

Parameter	Client I.D.		MW 18-01	MW 18-02	PW	O. Reg. 153	
	Sample I.D.	Date Collected	B18-28353-1 17-Sep-18	B18-28353-2 17-Sep-18	B18-28353-3 17-Sep-18	Tbl. 2 - PGW	
	Units	R.L.					
Acetone	µg/L	30	< 30	< 30	< 30	2700	
Benzene	µg/L	0.5	< 0.5	< 0.5	< 0.5	5	
Bromodichloromethane	µg/L	2	< 2	< 2	< 2	16	
Bromoform	µg/L	5	< 5	< 5	< 5	25	
Bromomethane	µg/L	0.5	< 0.5	< 0.5	< 0.5	0.89	
Carbon Tetrachloride	µg/L	0.2	< 0.2	< 0.2	< 0.2	0.79	
Monochlorobenzene (Chlorobenzene)	µg/L	0.5	< 0.5	< 0.5	< 0.5	30	
Chloroform	µg/L	1	< 1	< 1	< 1	2.4	
Dibromochloromethane	µg/L	2	< 2	< 2	< 2	25	
Dichlorobenzene,1,2-	µg/L	0.5	< 0.5	< 0.5	< 0.5	3	
Dichlorobenzene,1,3-	µg/L	0.5	< 0.5	< 0.5	< 0.5	59	
Dichlorobenzene,1,4-	µg/L	0.5	< 0.5	< 0.5	< 0.5	1	
Dichlorodifluoromethane	µg/L	2	< 2	< 2	< 2	590	
Dichloroethane,1,1-	µg/L	0.5	< 0.5	< 0.5	< 0.5	5	
Dichloroethane,1,2-	µg/L	0.5	< 0.5	< 0.5	< 0.5	1.6	
Dichloroethylene,1,1-	µg/L	0.5	< 0.5	< 0.5	< 0.5	1.6	
Dichloroethene, cis-1,2-	µg/L	0.5	< 0.5	< 0.5	< 0.5	1.6	
Dichloroethene, trans-1,2-	µg/L	0.5	< 0.5	< 0.5	< 0.5	1.6	
Dichloropropane,1,2-	µg/L	0.5	< 0.5	< 0.5	< 0.5	5	
Dichloropropene, cis-1,3-	µg/L	0.5	< 0.5	< 0.5	< 0.5		
Dichloropropene, trans-1,3-	µg/L	0.5	< 0.5	< 0.5	< 0.5		

O. Reg. 153 - Soil, Ground Water and Sediment Standards
 Tbl. 2 - PGW - Table 2 - Potable Ground Water



Christine Burke
 Lab Manager

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Caduceon Environmental Laboratories

110 West Beaver Creek Rd Unit 14
 Richmond Hill ON L4B 1J9

Tel: 289-475-5442

Fax: 289-562-1963

DATE RECEIVED: 18-Sep-18

JOB/PROJECT NO.: Lafarge - Goodwood Pit

DATE REPORTED: 25-Sep-18

P.O. NUMBER: 1894152 (2000)

SAMPLE MATRIX: Groundwater

WATERWORKS NO.

Parameter	Client I.D. Sample I.D. Date Collected		MW 18-01 B18-28353-1 17-Sep-18	MW 18-02 B18-28353-2 17-Sep-18	PW B18-28353-3 17-Sep-18	O. Reg. 153 Tbl. 2 - PGW	
	Units	R.L.					
Dichloropropene 1,3- cis+trans	µg/L	0.5	< 0.5	< 0.5	< 0.5	0.5	
Ethylbenzene	µg/L	0.5	< 0.5	< 0.5	< 0.5	2.4	
Dibromoethane,1,2- (Ethylene Dibromide)	µg/L	0.2	< 0.2	< 0.2	< 0.2	0.2	
Hexane	µg/L	5	< 5	< 5	< 5	51	
Methyl Ethyl Ketone	µg/L	20	< 20	< 20	< 20	1800	
Methyl Isobutyl Ketone	µg/L	20	< 20	< 20	< 20	640	
Methyl-t-butyl Ether	µg/L	2	< 2	< 2	< 2	15	
Dichloromethane (Methylene Chloride)	µg/L	0.3	< 0.3	< 0.3	< 0.3	50	
Styrene	µg/L	0.5	< 0.5	< 0.5	< 0.5	5.4	
Tetrachloroethane,1,1,1,2 -	µg/L	0.5	< 0.5	< 0.5	< 0.5	1.1	
Tetrachloroethane,1,1,2,2 -	µg/L	0.5	< 0.5	< 0.5	< 0.5	1	
Tetrachloroethylene	µg/L	0.5	< 0.5	< 0.5	< 0.5	1.6	
Toluene	µg/L	0.5	< 0.5	< 0.5	< 0.5	24	
Trichloroethane,1,1,1-	µg/L	0.5	< 0.5	< 0.5	< 0.5	200	
Trichloroethane,1,1,2-	µg/L	0.5	< 0.5	< 0.5	< 0.5	4.7	
Trichloroethylene	µg/L	0.5	< 0.5	< 0.5	< 0.5	1.6	
Trichlorofluoromethane	µg/L	5	< 5	< 5	< 5	150	
Vinyl Chloride	µg/L	0.5	< 0.5	< 0.5	< 0.5	0.5	
Xylene, m,p-	µg/L	1.0	< 1.0	< 1.0	< 1.0		
Xylene, o-	µg/L	0.5	< 0.5	< 0.5	< 0.5		

O. Reg. 153 - Soil, Ground Water and Sediment Standards
 Tbl. 2 - PGW - Table 2 - Potable Ground Water



Christine Burke
 Lab Manager

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 Fax: 289-562-1963

DATE RECEIVED: 18-Sep-18

JOB/PROJECT NO.: Lafarge - Goodwood Pit

DATE REPORTED: 25-Sep-18

P.O. NUMBER: 1894152 (2000)

SAMPLE MATRIX: Groundwater

WATERWORKS NO.

Parameter	Units	R.L.	Client I.D.	MW 18-01	MW 18-02	PW	O. Reg. 153	
			Sample I.D.	B18-28353-1	B18-28353-2	B18-28353-3	Tbl. 2 -	PGW
			Date Collected	17-Sep-18	17-Sep-18	17-Sep-18		
Xylene, m,p,o-	µg/L	1.1		< 1.1	< 1.1	< 1.1	300	
PHC F1 (C6-C10)	µg/L	50		< 50	< 50	< 50	750	
PHC F2 (>C10-C16)	µg/L	50		< 50	< 50	< 50	150	
PHC F3 (>C16-C34)	µg/L	400		2400	< 400	< 400	500	
PHC F4 (>C34-C50)	µg/L	400		< 400	< 400	< 400	500	

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 Tbl. 2 - PGW - Table 2 - Potable Ground Water



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DATE REPORTED: 25-Sep-18

P.O. NUMBER: 1894152 (2000)

SAMPLE MATRIX: Groundwater

WATERWORKS NO.

Summary of Exceedances

Table 2 - Potable Ground Water		
MW 18-01	Found Value	Limit
PHC F3 (>C16-C34) (µg/L)	2400	500

O. Reg. 153 - Soil, Ground Water and Sediment Standards
 Tbl. 2 - PGW - Table 2 - Potable Ground Water



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DATE RECEIVED: 18-Sep-18

JOB/PROJECT NO.: Lafarge - Goodwood Pit

DATE REPORTED: 25-Sep-18

SAMPLE MATRIX: Groundwater

P.O. NUMBER: 1894152 (2000)

WATERWORKS NO.

Parameter	Qty	Site Analyzed	Analyst Initials	Date Analyzed	Lab Method	Reference Method
SVOC	1	Kingston	sge	24-Sep-18	C-NAB-W-001 (k)	EPA 8270

O. Reg. 153 - Soil, Ground Water and Sediment Standards
 Tbl. 2 - PGW - Table 2 - Potable Ground Water



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DATE RECEIVED: 18-Sep-18

JOB/PROJECT NO.: Lafarge - Goodwood Pit

DATE REPORTED: 25-Sep-18

P.O. NUMBER: 1894152 (2000)

SAMPLE MATRIX: Groundwater

WATERWORKS NO.

Parameter	Units	R.L.	Client I.D. Sample I.D. Date Collected	MW 18-02 B18-28353-2 17-Sep-18	O. Reg. 153 Tbl. 2 - PGW	
Acenaphthene	µg/L	0.05	< 0.05		4.1	
Acenaphthylene	µg/L	0.05	< 0.05		1	
Anthracene	µg/L	0.05	< 0.05		2.4	
Benzo(a)anthracene	µg/L	0.05	< 0.05		1	
Benzo(a)pyrene	µg/L	0.01	< 0.01		0.01	
Benzo(b)fluoranthene	µg/L	0.05	< 0.05		0.1	
Benzo(b+k)fluoranthene	µg/L	0.1	< 0.1			
Benzo(g,h,i)perylene	µg/L	0.05	< 0.05		0.2	
Benzo(k)fluoranthene	µg/L	0.05	< 0.05		0.1	
Chrysene	µg/L	0.05	< 0.05		0.1	
Dibenzo(a,h)anthracene	µg/L	0.05	< 0.05		0.2	
Fluoranthene	µg/L	0.05	< 0.05		0.41	
Fluorene	µg/L	0.05	< 0.05		120	
Indeno(1,2,3,-cd)pyrene	µg/L	0.05	< 0.05		0.2	
Methylnaphthalene,1-	µg/L	0.05	< 0.05		3.2	
Methylnaphthalene,2-	µg/L	0.05	< 0.05		3.2	
Methylnaphthalene 2-(1-)	µg/L	0.07	< 0.07		3.2	
Naphthalene	µg/L	0.05	< 0.05		11	
Phenanthrene	µg/L	0.05	< 0.05		1	
Pyrene	µg/L	0.05	< 0.05		4.1	

O. Reg. 153 - Soil, Ground Water and Sediment Standards
 Tbl. 2 - PGW - Table 2 - Potable Ground Water



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Fax: 289-562-1963

DATE RECEIVED: 18-Sep-18

DATE REPORTED: 25-Sep-18

SAMPLE MATRIX: Groundwater

JOB/PROJECT NO.: Lafarge - Goodwood Pit

P.O. NUMBER: 1894152 (2000)

WATERWORKS NO.

Summary of Exceedances

O. Reg. 153 - Soil, Ground Water and Sediment Standards
Tbl. 2 - PGW - Table 2 - Potable Ground Water



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C.O.C.: G75461

REPORT No. B18-32587 (i)

Report To:

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 121 Commerce Park Drive, Unit L,
 Barrie ON. L4N 8X1 Canada

Attention: Devin Hannan

Caduceon Environmental Laboratories

112 Commerce Park Drive
 Barrie ON L4N 8W8
 Tel: 705-252-5743
 Fax: 705-252-5746

DATE RECEIVED: 22-Oct-18

JOB/PROJECT NO.: Lafarge - Goodwood Pit

DATE REPORTED: 02-Nov-18

P.O. NUMBER: 1894152/2000/2030

SAMPLE MATRIX: Groundwater

WATERWORKS NO.

Client I.D.	MW 18-03		
Sample I.D.	B18-32587-1		
Date Collected	22-Oct-18		

Parameter	Units	R.L.	Reference Method	Date/Site Analyzed			
Total Coliform	cfu/100mL	1	SM9222B	22-Oct-18/B	10		
E coli	cfu/100mL	1	SM9222B	22-Oct-18/B	0		
pH @25°C	pH Units		SM 4500H	26-Oct-18/O	8.09		
Conductivity @25°C	µmho/cm	1	SM 2510B	26-Oct-18/O	497		
Alkalinity(CaCO3) to pH4.5	mg/L	5	SM 2320B	26-Oct-18/O	190		
Carbonate (as CaCO3)	mg/L	5	SM 2320B	26-Oct-18/O	< 5		
Bicarbonate(as CaCO3)	mg/L	5	SM 2320B	26-Oct-18/O	190		
Hydroxide (as CaCO3)	mg/L	5	SM 2320B	26-Oct-18/O	< 5		
Hardness (as CaCO3)	mg/L	1	SM 3120	02-Nov-18/O	239		
Chloride	mg/L	0.5	SM4110C	23-Oct-18/O	11.8		
Fluoride	mg/L	0.1	SM4110C	23-Oct-18/O	< 0.1		
Nitrate (N)	mg/L	0.1	SM4110C	23-Oct-18/O	4.7		
Nitrite (N)	mg/L	0.1	SM4110C	23-Oct-18/O	< 0.1		
Sodium	mg/L	0.2	SM 3120	02-Nov-18/O	3.1		
Antimony	mg/L	0.0001	EPA 200.8	23-Oct-18/O	< 0.0001		
Arsenic	mg/L	0.0001	EPA 200.8	23-Oct-18/O	0.0003		
Barium	mg/L	0.001	SM 3120	02-Nov-18/O	0.053		
Beryllium	mg/L	0.0001	EPA 200.8	23-Oct-18/O	< 0.0001		
Boron	mg/L	0.005	SM 3120	02-Nov-18/O	0.009		
Cadmium	mg/L	0.000015	EPA 200.8	23-Oct-18/O	< 0.000015		
Chromium	mg/L	0.002	SM 3120	02-Nov-18/O	< 0.002		
Chromium (VI)	mg/L	0.002	MOE E3056	24-Oct-18/O	< 0.002		
Cobalt	mg/L	0.0001	EPA 200.8	23-Oct-18/O	< 0.0001		
Copper	mg/L	0.002	SM 3120	02-Nov-18/O	< 0.002		
Lead	mg/L	0.00002	EPA 200.8	23-Oct-18/O	0.00003		



Christine Burke
 Lab Manager

R.L. = Reporting Limit

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C.O.C.: G75461

REPORT No. B18-32587 (i)

Report To:

Golder Associates Ltd.
 121 Commerce Park Drive, Unit L,
 Barrie ON. L4N 8X1 Canada

Attention: Devin Hannan

Caduceon Environmental Laboratories

112 Commerce Park Drive
 Barrie ON L4N 8W8
 Tel: 705-252-5743
 Fax: 705-252-5746

DATE RECEIVED: 22-Oct-18

JOB/PROJECT NO.: Lafarge - Goodwood Pit

DATE REPORTED: 02-Nov-18

P.O. NUMBER: 1894152/2000/2030

SAMPLE MATRIX: Groundwater

WATERWORKS NO.

Client I.D.	MW 18-03		
Sample I.D.	B18-32587-1		
Date Collected	22-Oct-18		

Parameter	Units	R.L.	Reference Method	Date/Site Analyzed			
Mercury	mg/L	0.00002	SM 3112 B	30-Oct-18/O	< 0.00002		
Molybdenum	mg/L	0.0001	EPA 200.8	23-Oct-18/O	0.0141		
Nickel	mg/L	0.01	SM 3120	02-Nov-18/O	< 0.01		
Selenium	mg/L	0.001	EPA 200.8	23-Oct-18/O	< 0.001		
Silver	mg/L	0.0001	EPA 200.8	23-Oct-18/O	< 0.0001		
Thallium	mg/L	0.00005	EPA 200.8	23-Oct-18/O	< 0.00005		
Uranium	mg/L	0.00005	EPA 200.8	23-Oct-18/O	0.00164		
Vanadium	mg/L	0.0001	EPA 200.8	23-Oct-18/O	0.0003		
Zinc	mg/L	0.005	SM 3120	02-Nov-18/O	< 0.005		
Anion Sum	meq/L		Calc.	02-Nov-18/O	5.13		
Cation Sum	meq/L		Calc.	02-Nov-18/O	4.94		
% Difference	%		Calc.	02-Nov-18/O	1.89		
Ion Ratio	AS/CS		Calc.	02-Nov-18/O	1.04		
Sodium Adsorption Ratio	-		Calc.	02-Nov-18/O	0.0872		
TDS(ion sum calc.)	mg/L	1	Calc.	02-Nov-18/O	266		
Conductivity (calc.)	µmho/cm		Calc.	02-Nov-18/O	484		
TDS(calc.)/EC(actual)	-		Calc.	02-Nov-18/O	0.536		
EC(calc.)/EC(actual)	-		Calc.	02-Nov-18/O	0.975		
Langelier Index(25°C)	S.I.		Calc.	02-Nov-18/O	0.745		



Christine Burke
 Lab Manager

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C.O.C.: G75461

REPORT No. B18-32587 (ii)

Report To:

Golder Associates Ltd.
 121 Commerce Park Drive, Unit L,
 Barrie ON. L4N 8X1 Canada

Attention: Devin Hannan

Caduceon Environmental Laboratories

112 Commerce Park Drive
 Barrie ON L4N 8W8
 Tel: 705-252-5743
 Fax: 705-252-5746

DATE RECEIVED: 22-Oct-18

JOB/PROJECT NO.: Lafarge - Goodwood Pit

DATE REPORTED: 02-Nov-18

P.O. NUMBER: 1894152/2000/2030

SAMPLE MATRIX: Groundwater

WATERWORKS NO.

Client I.D.	MW 18-03			
Sample I.D.	B18-32587-1			
Date Collected	22-Oct-18			

Parameter	Units	R.L.	Reference Method	Date/Site Analyzed				
Acetone	µg/L	30	EPA 8260	24-Oct-18/R	< 30			
Benzene	µg/L	0.5	EPA 8260	24-Oct-18/R	< 0.5			
Bromodichloromethane	µg/L	2	EPA 8260	24-Oct-18/R	< 2			
Bromoform	µg/L	5	EPA 8260	24-Oct-18/R	< 5			
Bromomethane	µg/L	0.5	EPA 8260	24-Oct-18/R	< 0.5			
Carbon Tetrachloride	µg/L	0.2	EPA 8260	24-Oct-18/R	< 0.2			
Monochlorobenzene (Chlorobenzene)	µg/L	0.5	EPA 8260	24-Oct-18/R	< 0.5			
Chloroform	µg/L	1	EPA 8260	24-Oct-18/R	< 1			
Dibromochloromethane	µg/L	2	EPA 8260	24-Oct-18/R	< 2			
Dichlorobenzene, 1,2-	µg/L	0.5	EPA 8260	24-Oct-18/R	< 0.5			
Dichlorobenzene, 1,3-	µg/L	0.5	EPA 8260	24-Oct-18/R	< 0.5			
Dichlorobenzene, 1,4-	µg/L	0.5	EPA 8260	24-Oct-18/R	< 0.5			
Dichlorodifluoromethane	µg/L	2	EPA 8260	24-Oct-18/R	< 2			
Dichloroethane, 1,1-	µg/L	0.5	EPA 8260	24-Oct-18/R	< 0.5			
Dichloroethane, 1,2-	µg/L	0.5	EPA 8260	24-Oct-18/R	< 0.5			
Dichloroethylene, 1,1-	µg/L	0.5	EPA 8260	24-Oct-18/R	< 0.5			
Dichloroethene, cis-1,2-	µg/L	0.5	EPA 8260	24-Oct-18/R	< 0.5			
Dichloroethene, trans-1,2-	µg/L	0.5	EPA 8260	24-Oct-18/R	< 0.5			
Dichloropropane, 1,2-	µg/L	0.5	EPA 8260	24-Oct-18/R	< 0.5			
Dichloropropene, cis-1,3-	µg/L	0.5	EPA 8260	24-Oct-18/R	< 0.5			
Dichloropropene, trans-1,3-	µg/L	0.5	EPA 8260	24-Oct-18/R	< 0.5			
Dichloropropene 1,3-cis+trans	µg/L	0.5	EPA 8260	24-Oct-18/R	< 0.5			
Ethylbenzene	µg/L	0.5	EPA 8260	24-Oct-18/R	< 0.5			



Christine Burke
 Lab Manager

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C.O.C.: G75461

REPORT No. B18-32587 (ii)

Report To:

Golder Associates Ltd.
 121 Commerce Park Drive, Unit L,
 Barrie ON. L4N 8X1 Canada
Attention: Devin Hannan

Caduceon Environmental Laboratories

112 Commerce Park Drive
 Barrie ON L4N 8W8
 Tel: 705-252-5743
 Fax: 705-252-5746

DATE RECEIVED: 22-Oct-18
 DATE REPORTED: 02-Nov-18
 SAMPLE MATRIX: Groundwater

JOB/PROJECT NO.: Lafarge - Goodwood Pit
 P.O. NUMBER: 1894152/2000/2030
 WATERWORKS NO.

Client I.D.	MW 18-03			
Sample I.D.	B18-32587-1			
Date Collected	22-Oct-18			

Parameter	Units	R.L.	Reference Method	Date/Site Analyzed				
Dibromoethane,1,2-(Ethylene Dibromide)	µg/L	0.2	EPA 8260	24-Oct-18/R	< 0.2			
Hexane	µg/L	5	EPA 8260	24-Oct-18/R	< 5			
Methyl Ethyl Ketone	µg/L	20	EPA 8260	24-Oct-18/R	< 20			
Methyl Isobutyl Ketone	µg/L	20	EPA 8260	24-Oct-18/R	< 20			
Methyl-t-butyl Ether	µg/L	2	EPA 8260	24-Oct-18/R	< 2			
Dichloromethane (Methylene Chloride)	µg/L	0.3	EPA 8260	24-Oct-18/R	< 0.3			
Styrene	µg/L	0.5	EPA 8260	24-Oct-18/R	< 0.5			
Tetrachloroethane,1,1,1,2-	µg/L	0.5	EPA 8260	24-Oct-18/R	< 0.5			
Tetrachloroethane,1,1,2,2-	µg/L	0.5	EPA 8260	24-Oct-18/R	< 0.5			
Tetrachloroethylene	µg/L	0.5	EPA 8260	24-Oct-18/R	< 0.5			
Toluene	µg/L	0.5	EPA 8260	24-Oct-18/R	< 0.5			
Trichloroethane,1,1,1-	µg/L	0.5	EPA 8260	24-Oct-18/R	< 0.5			
Trichloroethane,1,1,2-	µg/L	0.5	EPA 8260	24-Oct-18/R	< 0.5			
Trichloroethylene	µg/L	0.5	EPA 8260	24-Oct-18/R	< 0.5			
Trichlorofluoromethane	µg/L	5	EPA 8260	24-Oct-18/R	< 5			
Vinyl Chloride	µg/L	0.5	EPA 8260	24-Oct-18/R	< 0.5			
Xylene, m,p-	µg/L	1.0	EPA 8260	24-Oct-18/R	< 1.0			
Xylene, o-	µg/L	0.5	EPA 8260	24-Oct-18/R	< 0.5			
Xylene, m,p,o-	µg/L	1.1	EPA 8260	24-Oct-18/R	< 1.1			
PHC F1 (C6-C10)	µg/L	50	MOE E3421	24-Oct-18/R	< 50			
PHC F2 (>C10-C16)	µg/L	50	MOE E3421	24-Oct-18/K	80			
PHC F3 (>C16-C34)	µg/L	400	MOE E3421	24-Oct-18/K	400			
PHC F4 (>C34-C50)	µg/L	400	MOE E3421	24-Oct-18/K	< 400			



Christine Burke
 Lab Manager

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C.O.C.: G75461

REPORT No. B18-32587 (iii)

Report To:

Golder Associates Ltd.
 121 Commerce Park Drive, Unit L,
 Barrie ON. L4N 8X1 Canada
Attention: Devin Hannan

Caduceon Environmental Laboratories

112 Commerce Park Drive
 Barrie ON L4N 8W8
 Tel: 705-252-5743
 Fax: 705-252-5746

DATE RECEIVED: 22-Oct-18
 DATE REPORTED: 02-Nov-18
 SAMPLE MATRIX: Groundwater

JOB/PROJECT NO.: Lafarge - Goodwood Pit
 P.O. NUMBER: 1894152/2000/2030
 WATERWORKS NO.

Client I.D.	MW 18-03			
Sample I.D.	B18-32587-1			
Date Collected	22-Oct-18			

Parameter	Units	R.L.	Reference Method	Date/Site Analyzed				
Acenaphthene	µg/L	0.05	EPA 8270	25-Oct-18/K	< 0.05			
Acenaphthylene	µg/L	0.05	EPA 8270	25-Oct-18/K	< 0.05			
Anthracene	µg/L	0.05	EPA 8270	25-Oct-18/K	< 0.05			
Benzo(a)anthracene	µg/L	0.05	EPA 8270	25-Oct-18/K	< 0.05			
Benzo(a)pyrene	µg/L	0.01	EPA 8270	25-Oct-18/K	< 0.01			
Benzo(b)fluoranthene	µg/L	0.05	EPA 8270	25-Oct-18/K	< 0.05			
Benzo(b+k)fluoranthene	µg/L	0.1	EPA 8270	25-Oct-18/K	< 0.1			
Benzo(g,h,i)perylene	µg/L	0.05	EPA 8270	25-Oct-18/K	< 0.05			
Benzo(k)fluoranthene	µg/L	0.05	EPA 8270	25-Oct-18/K	< 0.05			
Chrysene	µg/L	0.05	EPA 8270	25-Oct-18/K	< 0.05			
Dibenzo(a,h)anthracene	µg/L	0.05	EPA 8270	25-Oct-18/K	< 0.05			
Fluoranthene	µg/L	0.05	EPA 8270	25-Oct-18/K	< 0.05			
Fluorene	µg/L	0.05	EPA 8270	25-Oct-18/K	< 0.05			
Indeno(1,2,3,-cd)pyrene	µg/L	0.05	EPA 8270	25-Oct-18/K	< 0.05			
Methylnaphthalene,1-	µg/L	0.05	EPA 8270	25-Oct-18/K	< 0.05			
Methylnaphthalene,2-	µg/L	0.05	EPA 8270	25-Oct-18/K	< 0.05			
Methylnaphthalene 2-(1-)	µg/L	0.07	EPA 8270	25-Oct-18/K	< 0.07			
Naphthalene	µg/L	0.05	EPA 8270	25-Oct-18/K	< 0.05			
Phenanthrene	µg/L	0.05	EPA 8270	25-Oct-18/K	< 0.05			
Pyrene	µg/L	0.05	EPA 8270	25-Oct-18/K	< 0.05			



Christine Burke
 Lab Manager

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CLIENT NAME: GOLDER ASSOCIATES LTD.
121 COMMERCE PARK DRIVE, UNIT L
BARRIE, ON L4N8X1
(705) 722-4492

ATTENTION TO: Devin Hannan

PROJECT: 1894152(2110)

AGAT WORK ORDER: 19T460288

TRACE ORGANICS REVIEWED BY: Pinkal Patel, Report Reviewer

DATE REPORTED: May 01, 2019

PAGES (INCLUDING COVER): 5

VERSION*: 1

Should you require any information regarding this analysis please contact your client services representative at (905) 712-5100

*NOTES

All samples will be disposed of within 30 days following analysis. Please contact the lab if you require additional sample storage time.



Certificate of Analysis

AGAT WORK ORDER: 19T460288

PROJECT: 1894152(2110)

5835 COOPERS AVENUE
MISSISSAUGA, ONTARIO
CANADA L4Z 1Y2
TEL (905)712-5100
FAX (905)712-5122
<http://www.agatlabs.com>

CLIENT NAME: GOLDER ASSOCIATES LTD.

ATTENTION TO: Devin Hannan

SAMPLING SITE:

SAMPLED BY:

O. Reg. 153(511) - PHCs F1 - F4 (Water)

DATE RECEIVED: 2019-04-25

DATE REPORTED: 2019-05-01

Parameter	Unit	SAMPLE DESCRIPTION:		MW18-01	MW18-02	MW18-03	PW	DUP
		G / S	RDL	154946	154947	154948	154949	154950
Benzene	µg/L	5.0	0.20	<0.20	<0.20	<0.20	<0.20	<0.20
Toluene	µg/L	24	0.20	<0.20	<0.20	<0.20	<0.20	<0.20
Ethylbenzene	µg/L	2.4	0.10	<0.10	<0.10	<0.10	<0.10	<0.10
Xylene Mixture	µg/L	300	0.20	<0.20	<0.20	<0.20	<0.20	<0.20
F1 (C6 - C10)	µg/L	750	25	<25	<25	<25	<25	<25
F1 (C6 to C10) minus BTEX	µg/L	750	25	<25	<25	<25	<25	<25
F2 (C10 to C16)	µg/L	150	100	<100	<100	<100	<100	<100
F3 (C16 to C34)	µg/L	500	100	<100	<100	110	<100	120
F4 (C34 to C50)	µg/L	500	100	<100	<100	<100	<100	<100
Gravimetric Heavy Hydrocarbons	µg/L	500	500	NA	NA	NA	NA	NA
Surrogate	Unit	Acceptable Limits						
Terphenyl	%	60-140		60	62	112	61	72

Comments: RDL - Reported Detection Limit; G / S - Guideline / Standard: Refers to Table 2: Full Depth Generic Site Condition Standards in a Potable Ground Water Condition - Potable Ground Water - All Types of Property Uses - Coarse Textured Soils

Guideline values are for general reference only. The guidelines provided may or may not be relevant for the intended use. Refer directly to the applicable standard for regulatory interpretation.

154946-154950 The C6-C10 fraction is calculated using Toluene response factor.
Xylenes total is a calculated parameter. The calculated value is the sum of m&p-Xylene and o-Xylene.
C6-C10 (F1 minus BTEX) is a calculated parameter. The calculated value is F1 minus BTEX.
The C10 - C16, C16 - C34, and C34 - C50 fractions are calculated using the average response factor for n-C10, n-C16, and nC34.
Gravimetric Heavy Hydrocarbons are not included in the Total C16 - C50 and are only determined if the chromatogram of the C34 - C50 Hydrocarbons indicated that hydrocarbons >C50 are present.
The chromatogram has returned to baseline by the retention time of nC50.
Total C6-C50 results are corrected for BTEX contribution.
This method complies with the Reference Method for the CWS PHC and is validated for use in the laboratory.
nC6 and nC10 response factors are within 30% of Toluene response factor.
nC10, nC16 and nC34 response factors are within 10% of their average.
C50 response factor is within 70% of nC10 + nC16 nC34 average.
Linearity is within 15%.
Extraction and holding times were met for this sample.
Fractions 1-4 are quantified with the contribution of PAHs. Under Ontario Regulation 153/04, results are considered valid without determining the PAH contribution if not requested by the client.
NA = Not Applicable

Analysis performed at AGAT Toronto (unless marked by *)

Certified By:

Quality Assurance

 CLIENT NAME: GOLDER ASSOCIATES LTD.
 PROJECT: 1894152(2110)
 SAMPLING SITE:

 AGAT WORK ORDER: 19T460288
 ATTENTION TO: Devin Hannan
 SAMPLED BY:

Trace Organics Analysis

RPT Date: May 01, 2019			DUPLICATE			Method Blank	REFERENCE MATERIAL			METHOD BLANK SPIKE			MATRIX SPIKE		
PARAMETER	Batch	Sample Id	Dup #1	Dup #2	RPD		Measured Value	Acceptable Limits		Recovery	Acceptable Limits		Recovery	Acceptable Limits	
								Lower	Upper		Lower	Upper		Lower	Upper
O. Reg. 153(511) - PHCs F1 - F4 (Water)															
Benzene	146693		< 0.20	< 0.20	NA	< 0.20	105%	50%	140%	85%	60%	130%	119%	50%	140%
Toluene	146693		< 0.20	< 0.20	NA	< 0.20	108%	50%	140%	80%	60%	130%	114%	50%	140%
Ethylbenzene	146693		< 0.10	< 0.10	NA	< 0.10	104%	50%	140%	75%	60%	130%	106%	50%	140%
Xylene Mixture	146693		< 0.20	< 0.20	NA	< 0.20	89%	50%	140%	76%	60%	130%	103%	50%	140%
F1 (C6 - C10)	146693		< 25	< 25	NA	< 25	98%	60%	140%	89%	60%	140%	81%	60%	140%
F2 (C10 to C16)		TW	< 100	< 100	NA	< 100	94%	60%	140%	72%	60%	140%	62%	60%	140%
F3 (C16 to C34)		TW	< 100	< 100	NA	< 100	98%	60%	140%	85%	60%	140%	87%	60%	140%
F4 (C34 to C50)		TW	< 100	< 100	NA	< 100	96%	60%	140%	96%	60%	140%	92%	60%	140%

Comments: Tap water analysis has been performed as QC sample testing for duplicate and matrix spike due to insufficient sample volume. When the average of the sample and duplicate results is less than 5x the RDL, the Relative Percent Difference (RPD) will be indicated as Not Applicable (NA).

Certified By: _____



Method Summary

CLIENT NAME: GOLDER ASSOCIATES LTD.

AGAT WORK ORDER: 19T460288

PROJECT: 1894152(2110)

ATTENTION TO: Devin Hannan

SAMPLING SITE:

SAMPLED BY:

PARAMETER	AGAT S.O.P	LITERATURE REFERENCE	ANALYTICAL TECHNIQUE
Trace Organics Analysis			
Benzene	VOL-91-5010	MOE PHC-E3421	P&T GC/MS
Toluene	VOL-91-5010	MOE PHC-E3421	P&T GC/MS
Ethylbenzene	VOL-91-5010	MOE PHC-E3421	P&T GC/MS
Xylene Mixture	VOL-91-5010	MOE PHC-E3421	P&T GC/MS
F1 (C6 - C10)	VOL-91- 5010	MOE PHC-E3421	P&T GC/FID
F1 (C6 to C10) minus BTEX	VOL-91-5010	MOE PHC-E3421	P&T GC/FID
F2 (C10 to C16)	VOL-91-5010	MOE PHC-E3421	GC/FID
F3 (C16 to C34)	VOL-91-5010	MOE PHC-E3421	GC/FID
F4 (C34 to C50)	VOL-91-5010	MOE PHC-E3421	GC/FID
Gravimetric Heavy Hydrocarbons	VOL-91-5010	MOE PHC-E3421	BALANCE
Terphenyl	VOL-91-5010	MOE PHC-E3421	GC/FID



Your Project #: 21453907
 Site Location: GOODWOOL
 Your C.O.C. #: 827225-01-01

Attention: Dawn Hoyle

Golder Associates Ltd
 121 Commerce Park Drive
 Unit L
 Barrie, ON
 CANADA L4N 8X1

Report Date: 2021/05/28
 Report #: R6651649
 Version: 1 - Final

CERTIFICATE OF ANALYSIS

BV LABS JOB #: C1D2546

Received: 2021/05/17, 14:44

Sample Matrix: Water
 # Samples Received: 6

Analyses	Quantity	Date	Date	Laboratory Method	Analytical Method
		Extracted	Analyzed		
Petroleum Hydro. CCME F1 & BTEX in Water	6	N/A	2021/05/20	CAM SOP-00315	CCME PHC-CWS m
Petroleum Hydrocarbons F2-F4 in Water (1)	5	2021/05/19	2021/05/20	CAM SOP-00316	CCME PHC-CWS m

Remarks:

Bureau Veritas is accredited to ISO/IEC 17025 for specific parameters on scopes of accreditation. Unless otherwise noted, procedures used by Bureau Veritas are based upon recognized Provincial, Federal or US method compendia such as CCME, MELCC, EPA, APHA.

All work recorded herein has been done in accordance with procedures and practices ordinarily exercised by professionals in Bureau Veritas' profession using accepted testing methodologies, quality assurance and quality control procedures (except where otherwise agreed by the client and Bureau Veritas in writing). All data is in statistical control and has met quality control and method performance criteria unless otherwise noted. All method blanks are reported; unless indicated otherwise, associated sample data are not blank corrected. Where applicable, unless otherwise noted, Measurement Uncertainty has not been accounted for when stating conformity to the referenced standard.

Bureau Veritas liability is limited to the actual cost of the requested analyses, unless otherwise agreed in writing. There is no other warranty expressed or implied. Bureau Veritas has been retained to provide analysis of samples provided by the Client using the testing methodology referenced in this report. Interpretation and use of test results are the sole responsibility of the Client and are not within the scope of services provided by Bureau Veritas, unless otherwise agreed in writing. Bureau Veritas is not responsible for the accuracy or any data impacts, that result from the information provided by the customer or their agent.

Solid sample results, except biota, are based on dry weight unless otherwise indicated. Organic analyses are not recovery corrected except for isotope dilution methods.

Results relate to samples tested. When sampling is not conducted by Bureau Veritas, results relate to the supplied samples tested.

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Reference Method suffix "m" indicates test methods incorporate validated modifications from specific reference methods to improve performance.

* RPDs calculated using raw data. The rounding of final results may result in the apparent difference.

(1) All CCME PHC results met required criteria unless otherwise stated in the report. The CWS PHC methods employed by Bureau Veritas Laboratories conform to all prescribed elements of the reference method and performance based elements have been validated. All modifications have been validated and proven equivalent following "Alberta Environment's Interpretation of the Reference Method for the Canada-Wide Standard for Petroleum Hydrocarbons in Soil Validation of Performance-Based Alternative Methods September 2003". Documentation is available upon request. Modifications from Reference Method for the Canada-wide Standard for Petroleum Hydrocarbons in Soil-Tier 1 Method: F2/F3/F4 data reported using validated cold solvent extraction instead of Soxhlet extraction.



Your Project #: 21453907
Site Location: GOODWOOL
Your C.O.C. #: 827225-01-01

Attention: Dawn Hoyle

Golder Associates Ltd
121 Commerce Park Drive
Unit L
Barrie, ON
CANADA L4N 8X1

Report Date: 2021/05/28
Report #: R6651649
Version: 1 - Final

CERTIFICATE OF ANALYSIS

BV LABS JOB #: C1D2546
Received: 2021/05/17, 14:44

Encryption Key

Please direct all questions regarding this Certificate of Analysis to your Project Manager.

Ema Gitej, Senior Project Manager
Email: emese.gitej@bureauveritas.com
Phone# (905)817-5829

=====

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BUREAU
VERITAS

BV Labs Job #: C1D2546
Report Date: 2021/05/28

Golder Associates Ltd
Client Project #: 21453907
Site Location: GOODWOOL
Sampler Initials: D.S

PETROLEUM HYDROCARBONS (CCME)

BV Labs ID		POX994		
Sampling Date		2021/05/17 08:00		
COC Number		827225-01-01		
	UNITS	TRIP BLANK	RDL	QC Batch
BTEX & F1 Hydrocarbons				
F1 (C6-C10)	ug/L	<25	25	7361905
F1 (C6-C10) - BTEX	ug/L	<25	25	7361905
Surrogate Recovery (%)				
1,4-Difluorobenzene	%	105		7361905
4-Bromofluorobenzene	%	87		7361905
D10-o-Xylene	%	102		7361905
D4-1,2-Dichloroethane	%	103		7361905
RDL = Reportable Detection Limit QC Batch = Quality Control Batch				



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VERITAS

BV Labs Job #: C1D2546
Report Date: 2021/05/28

Golder Associates Ltd
Client Project #: 21453907
Site Location: GOODWOOL
Sampler Initials: D.S

O.REG 153 PHCS, BTEX/F1-F4 (WATER)

BV Labs ID		POX989	POX990	POX991	POX992	POX993		
Sampling Date		2021/05/17 09:25	2021/05/17 09:25	2021/05/17 10:25	2021/05/17 11:30	2021/05/17 12:15		
COC Number		827225-01-01	827225-01-01	827225-01-01	827225-01-01	827225-01-01		
	UNITS	MW18-03	DUPE-1	PW	MW18-02	MW18-01	RDL	QC Batch
BTEX & F1 Hydrocarbons								
F1 (C6-C10)	ug/L	<25	<25	<25	<25	<25	25	7361905
F1 (C6-C10) - BTEX	ug/L	<25	<25	<25	<25	<25	25	7361905
F2-F4 Hydrocarbons								
F2 (C10-C16 Hydrocarbons)	ug/L	<100	<100	<100	<100	<100	100	7360004
F3 (C16-C34 Hydrocarbons)	ug/L	<200	<200	<200	<200	<200	200	7360004
F4 (C34-C50 Hydrocarbons)	ug/L	<200	<200	<200	<200	<200	200	7360004
Reached Baseline at C50	ug/L	Yes	Yes	Yes	Yes	Yes		7360004
Surrogate Recovery (%)								
1,4-Difluorobenzene	%	103	102	104	103	103		7361905
4-Bromofluorobenzene	%	95	81	90	93	95		7361905
D10-o-Xylene	%	99	105	105	99	106		7361905
D4-1,2-Dichloroethane	%	105	108	104	106	105		7361905
o-Terphenyl	%	81	88	87	89	88		7360004
RDL = Reportable Detection Limit QC Batch = Quality Control Batch								



BUREAU
VERITAS

BV Labs Job #: C1D2546
Report Date: 2021/05/28

Golder Associates Ltd
Client Project #: 21453907
Site Location: GOODWOOL
Sampler Initials: D.S

TEST SUMMARY

BV Labs ID: POX989
Sample ID: MW18-03
Matrix: Water

Collected: 2021/05/17
Shipped:
Received: 2021/05/17

Test Description	Instrumentation	Batch	Extracted	Date Analyzed	Analyst
Petroleum Hydro. CCME F1 & BTEX in Water	HSGC/MSFD	7361905	N/A	2021/05/20	Abdikarim Ali
Petroleum Hydrocarbons F2-F4 in Water	GC/FID	7360004	2021/05/19	2021/05/20	Ravinder Gaidhu

BV Labs ID: POX990
Sample ID: DUPE-1
Matrix: Water

Collected: 2021/05/17
Shipped:
Received: 2021/05/17

Test Description	Instrumentation	Batch	Extracted	Date Analyzed	Analyst
Petroleum Hydro. CCME F1 & BTEX in Water	HSGC/MSFD	7361905	N/A	2021/05/20	Abdikarim Ali
Petroleum Hydrocarbons F2-F4 in Water	GC/FID	7360004	2021/05/19	2021/05/20	Ravinder Gaidhu

BV Labs ID: POX991
Sample ID: PW
Matrix: Water

Collected: 2021/05/17
Shipped:
Received: 2021/05/17

Test Description	Instrumentation	Batch	Extracted	Date Analyzed	Analyst
Petroleum Hydro. CCME F1 & BTEX in Water	HSGC/MSFD	7361905	N/A	2021/05/20	Abdikarim Ali
Petroleum Hydrocarbons F2-F4 in Water	GC/FID	7360004	2021/05/19	2021/05/20	Ravinder Gaidhu

BV Labs ID: POX992
Sample ID: MW18-02
Matrix: Water

Collected: 2021/05/17
Shipped:
Received: 2021/05/17

Test Description	Instrumentation	Batch	Extracted	Date Analyzed	Analyst
Petroleum Hydro. CCME F1 & BTEX in Water	HSGC/MSFD	7361905	N/A	2021/05/20	Abdikarim Ali
Petroleum Hydrocarbons F2-F4 in Water	GC/FID	7360004	2021/05/19	2021/05/20	Ravinder Gaidhu

BV Labs ID: POX993
Sample ID: MW18-01
Matrix: Water

Collected: 2021/05/17
Shipped:
Received: 2021/05/17

Test Description	Instrumentation	Batch	Extracted	Date Analyzed	Analyst
Petroleum Hydro. CCME F1 & BTEX in Water	HSGC/MSFD	7361905	N/A	2021/05/20	Abdikarim Ali
Petroleum Hydrocarbons F2-F4 in Water	GC/FID	7360004	2021/05/19	2021/05/20	Ravinder Gaidhu

BV Labs ID: POX994
Sample ID: TRIP BLANK
Matrix: Water

Collected: 2021/05/17
Shipped:
Received: 2021/05/17

Test Description	Instrumentation	Batch	Extracted	Date Analyzed	Analyst
Petroleum Hydro. CCME F1 & BTEX in Water	HSGC/MSFD	7361905	N/A	2021/05/20	Abdikarim Ali



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VERITAS

BV Labs Job #: C1D2546
Report Date: 2021/05/28

Golder Associates Ltd
Client Project #: 21453907
Site Location: GOODWOOL
Sampler Initials: D.S

GENERAL COMMENTS

Each temperature is the average of up to three cooler temperatures taken at receipt

Package 1	11.0°C
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Sample POX994 [TRIP BLANK] : Sample analyzed for F1/BTEX as per client request.

Results relate only to the items tested.



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BV Labs Job #: C1D2546

Report Date: 2021/05/28

Golder Associates Ltd

Client Project #: 21453907

Site Location: GOODWOOL

Sampler Initials: D.S

QUALITY ASSURANCE REPORT

QA/QC Batch	Init	QC Type	Parameter	Date Analyzed	Value	Recovery	UNITS	QC Limits
7360004	RGA	Matrix Spike	o-Terphenyl	2021/05/19		91	%	60 - 130
			F2 (C10-C16 Hydrocarbons)	2021/05/19		98	%	60 - 130
			F3 (C16-C34 Hydrocarbons)	2021/05/19		95	%	60 - 130
			F4 (C34-C50 Hydrocarbons)	2021/05/19		97	%	60 - 130
7360004	RGA	Spiked Blank	o-Terphenyl	2021/05/19		91	%	60 - 130
			F2 (C10-C16 Hydrocarbons)	2021/05/19		101	%	60 - 130
			F3 (C16-C34 Hydrocarbons)	2021/05/19		99	%	60 - 130
			F4 (C34-C50 Hydrocarbons)	2021/05/19		100	%	60 - 130
7360004	RGA	Method Blank	o-Terphenyl	2021/05/19		89	%	60 - 130
			F2 (C10-C16 Hydrocarbons)	2021/05/19	<100		ug/L	
			F3 (C16-C34 Hydrocarbons)	2021/05/19	<200		ug/L	
			F4 (C34-C50 Hydrocarbons)	2021/05/19	<200		ug/L	
7360004	RGA	RPD	F2 (C10-C16 Hydrocarbons)	2021/05/19	NC		%	30
			F3 (C16-C34 Hydrocarbons)	2021/05/19	NC		%	30
			F4 (C34-C50 Hydrocarbons)	2021/05/19	NC		%	30
7361905	AAI	Matrix Spike	1,4-Difluorobenzene	2021/05/19		100	%	70 - 130
			4-Bromofluorobenzene	2021/05/19		99	%	70 - 130
			D10-o-Xylene	2021/05/19		102	%	70 - 130
			D4-1,2-Dichloroethane	2021/05/19		101	%	70 - 130
			F1 (C6-C10)	2021/05/19		82	%	60 - 140
7361905	AAI	Spiked Blank	1,4-Difluorobenzene	2021/05/19		101	%	70 - 130
			4-Bromofluorobenzene	2021/05/19		98	%	70 - 130
			D10-o-Xylene	2021/05/19		103	%	70 - 130
			D4-1,2-Dichloroethane	2021/05/19		99	%	70 - 130
			F1 (C6-C10)	2021/05/19		82	%	60 - 140
7361905	AAI	Method Blank	1,4-Difluorobenzene	2021/05/19		103	%	70 - 130
			4-Bromofluorobenzene	2021/05/19		91	%	70 - 130
			D10-o-Xylene	2021/05/19		99	%	70 - 130
			D4-1,2-Dichloroethane	2021/05/19		105	%	70 - 130
			F1 (C6-C10)	2021/05/19	<25		ug/L	
7361905	AAI	RPD	F1 (C6-C10) - BTEX	2021/05/19	<25		ug/L	
			F1 (C6-C10)	2021/05/19	NC		%	30
			F1 (C6-C10) - BTEX	2021/05/19	NC		%	30

Duplicate: Paired analysis of a separate portion of the same sample. Used to evaluate the variance in the measurement.

Matrix Spike: A sample to which a known amount of the analyte of interest has been added. Used to evaluate sample matrix interference.

Spiked Blank: A blank matrix sample to which a known amount of the analyte, usually from a second source, has been added. Used to evaluate method accuracy.

Method Blank: A blank matrix containing all reagents used in the analytical procedure. Used to identify laboratory contamination.

Surrogate: A pure or isotopically labeled compound whose behavior mirrors the analytes of interest. Used to evaluate extraction efficiency.

NC (Duplicate RPD): The duplicate RPD was not calculated. The concentration in the sample and/or duplicate was too low to permit a reliable RPD calculation (absolute difference <= 2x RDL).



BUREAU
VERITAS

BV Labs Job #: C1D2546

Report Date: 2021/05/28

Golder Associates Ltd

Client Project #: 21453907

Site Location: GOODWOOL

Sampler Initials: D.S

VALIDATION SIGNATURE PAGE

The analytical data and all QC contained in this report were reviewed and validated by:

Eva Pranjić

Ewa Pranjić, M.Sc., C.Chem, Scientific Specialist

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APPENDIX E

**Groundwater Zone of Influence
Calculations**

**Inflow to the Open Pit (1)
Based on Equations by Marinelli and Niccoli (2000)**

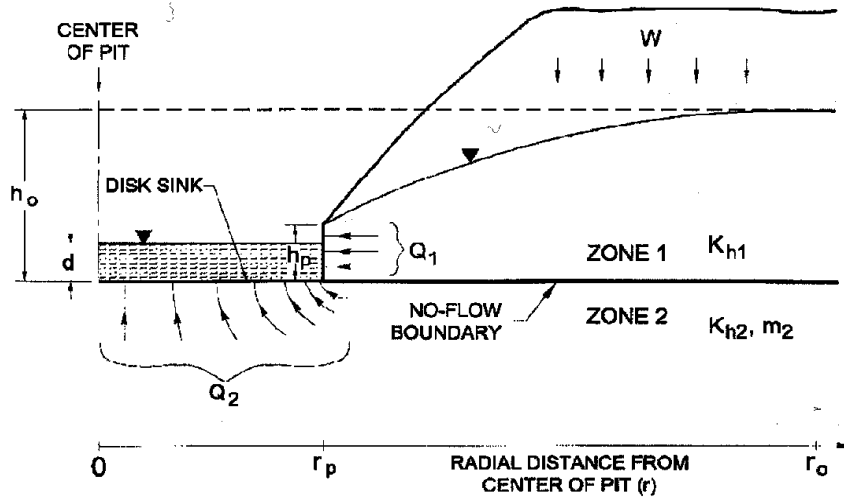


Figure 2. Pit inflow analytical model.

Marinelli, F., and W. L. Niccoli. 2000. Simple analytical equations for estimating ground water inflow to a mine pit. *Ground Water* 38, no. 2: 311-314.

$$(A) \quad h_o = \sqrt{h_p^2 + \frac{W}{K_{h1}} \left[r_o^2 \ln \left(\frac{r_o}{r_p} \right) - \frac{(r_o^2 - r_p^2)}{2} \right]}$$

$$(B) \quad Q_1 = W \pi (r_o^2 - r_p^2)$$

$$(C) \quad Q_2 = 4 r_p \left(\frac{K_{h2}}{m_2} \right) (h_o - d)$$

$$(D) \quad m_2 = \sqrt{\frac{K_{h2}}{K_{v2}}}$$

Input Parameters

W (m/s)	6.8E-09	recharge flux
Kh1 (m/s)	2.0E-04	horizontal hydraulic conductivity in Zone 1
Kh2 (m/s)	2.0E-04	horizontal hydraulic conductivity in Zone 2
Kv2 (m/s)	2.0E-04	vertical hydraulic conductivity in Zone 2
ho (m)	12.25	initial saturated thickness above the base of Zone 1
hp (m)	12.0	saturated thickness at the pit wall
rp (m)	135	effective pit radius
d (m)	12.00	depth of the pit lake

Inflow to the Open Pit (2)
Based on Equations by Marinelli and Niccoli (2000)

Inflow from Zone 1

ro (m)	474.0	radius of influence calculated by iterating equation A / goalseek
(known ho)		(ho calculated using eq. A)
12.2500	=	12.251

**Drawdown with Distance
Based on Equations by Marinelli and Niccoli (2000)**

r (m)	=	450.0	selected distance from pit centre
H (m) at r	=	12.25	head at selected distance from pit centre
Drawdown (m) at r	=	0.001	drawdown at selected distance from pit centre

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