



DIXON
HYDROGEOLOGY
LIMITED
Consulting Engineers

G04Q01

File No. H09Q01
January 11, 1992

ORO SEVENTH LINE AGGREGATE PITS
HYDROGEOLOGICAL STUDY

Distribution:

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1. INTRODUCTION

Six existing and proposed sand and aggregate extraction properties are located on the Seventh Line of the Township of Oro, between County Road 11 and Bass Lake Sideroad. The licensed properties include the Township of Oro, Seeley & Arnill Aggregates Ltd., Allan G. Cook Limited, and James Dick Construction Limited. Of these, all but the Township of Oro are seeking extensions to their licences. The proposed extraction properties are owned by Alfa Aggregates Limited and The Sarjeant Co. Ltd. Property areas and locations are listed in Table B-1 and shown on Drawing 1.

The six existing and proposed aggregate extraction properties are located almost entirely within an Aggregate Resource Area of Primary Significance (Figure 1), as designated by the Ministry of Natural Resources (MNR) (OGS, 1984).

This hydrogeological study has been undertaken to assess the impacts of the combined existing and proposed extraction pits on the groundwater system, local surface water flows, and private wells.

Locations of private wells are shown on Drawing 1, and the well records filed with the Ministry of the Environment (MOE) are contained in Appendix A.

This assessment includes a review of the individual site hydrogeological reports and on-site drilling and testing data. Estimates were made of stream flow at the Coldwater River tributaries and Hawkestone Creek on 26 November 1991.

2. PHYSICAL CONDITIONS

2.1 Precipitation and Water Surplus

The aggregate properties are located between the Orillia and Midhurst climate stations. Data recorded at the two Orillia climate stations is sequential and, since these stations are located in close proximity to each other, the data were combined to calculate average water budget values. Normal annual precipitation averaged from records for the two Orillia climate stations is 922 mm, as measured for the periods 1929 through 1961 and 1980 through 1989 (Canadian Climate Centre). At the Midhurst climate station, normal annual precipitation is 901 mm for the period 1953 through 1989 (Canadian Climate Centre). The average annual precipitation from the two locations is 911 mm (Table C-1).

The average annual water surplus calculated for the two Orillia climate stations is reported to be 427 mm, and for the Midhurst climate station it is reported to be 407 mm, for an average of 417 mm. The moisture surplus is the excess water after evaporation needs of the surface have been met and soil storage has been returned to its water-holding capacity. It is a measure of the water available for groundwater recharge and runoff. The water surplus calculations are based on a sandy soil with a water-holding capacity of 100 mm (Canadian Climate Centre). The water surplus is an indication of the maximum infiltration to recharge groundwater where there is no surface runoff.

2.2 Physiography and Drainage

The aggregate properties are located on the hummocky terrain of the Bass Lake Kame Moraine (also known as the Oro Sand Hills), where surface elevations range from 310 m above sea level (masl) at the Township of Oro property, to 378 masl at the Alfa Aggregates Limited property. The moraine, as defined by Chapman and Putnam (1984), extends approximately 25.6 km in a general south-west to north-east direction from

Midhurst to three kilometers north-east of Rugby. It is approximately 6590 m wide in a north-west to south-east direction in the vicinity of the Seventh Line of Oro Township.

The sandy surficial materials allow for a high infiltration rate. There is little evidence of surface runoff on the moraine proper, as determined from visual inspection of the properties, and therefore it appears that almost all of the water surplus infiltrates the sandy surficial deposits. On the flanks of the moraine, surficial material is finer grained, resulting in a decreased rate of infiltration and an increase in runoff.

The headwaters of right-bank tributaries of the Coldwater River are to the north on the flanks of the moraine. The Coldwater River flows northward to discharge to Matchedash Bay, Severn Sound. The Coulson Swamp is located on the north flanks of the moraine at an elevation of approximately 290 masl (Drawing 2). This swamp is at the headwaters of a tributary of the Coldwater River. The Coldwater River basin covers 17,700 Ha at Coldwater, where Stream Gauging Station No. 02ED007 is located.

The mean low flow monitored at Station No. 02ED007 is $1.073 \text{ m}^3/\text{s}$ for the period 1965 through 1988. The area of the drainage basin above this station is 177 km^2 . This low flow provides an indication of base flow to the stream. Estimates of the flow rate at locations closer to the pit properties were made on 26 November 1991. The estimates were made by measuring the cross-sectional area of the stream where it passes through a culvert and the stream velocity at the water surface. The mean stream velocity is equivalent to 90 to 95% of the surface velocity (Butler, 1951). Flow in the Coldwater River tributary, which follows the Seventh Line of Oro Township past Coulson (Drawing 1), was monitored at the culvert under Rowanwood Road. A flow rate of $0.027 \text{ m}^3/\text{s}$ was estimated (Table B-3). Flow in the east tributary, which follows the Eighth Line of Oro Township, was estimated at the culvert beneath the Eighth Line south of Coulson Swamp. A rate of $0.13 \text{ m}^3/\text{s}$ was estimated for this location.

To the south, a number of small creeks drain the flanks of the moraine and join the eastward-flowing Hawkestone Creek, which discharges to Lake Simcoe near Hawkestone. The Hawkestone Creek basin is estimated to cover approximately 3960 Ha (Young, 1992). The upstream reaches of the creek are within a wetland area identified as the Hawkestone Swamp, which covers approximately 832 Ha (Young, 1992).

There is no stream gauging station located on Hawkestone Creek. The stream flow beneath the Fifteenth/Sixteenth Sideroad bridge was estimated on 26 November 1991. A flow rate of 0.23 m³/s was estimated at this location.

2.3 Geology

Ordovician-age limestone of the Bobcaygeon Formation forms a relatively flat-lying bedrock surface in the Township of Oro (OGS, 1984). This surface is at an elevation of approximately 165 to 175 masl, which is at a depth of up to 200 m at the height of the moraine and 50 m at Lake Simcoe (Drawing 3).

The bedrock is overlain by Quaternary deposits. The upper layers of unconsolidated materials are deposits of the Wisconsinan ice sheet(s) (OGS, 1984).

The Ontario Geological Survey (1991) has drilled, cored, and visually logged three boreholes in the Township of Oro: one on the moraine and two to the south (Drawing 3). Another three boreholes were drilled in the Township of Medonte, north of the moraine (one of which is shown on Drawing 3). The logs of boreholes drilled north and south of the moraine indicate thick sequences of diamictic material (unstratified, poorly-sorted, fine- and coarse-grained material), probably till, and also indicate layers of sand. Six sand layers were intersected in wells south of the moraine, shown on Drawing 3. The surficial deposits on the lowlands surrounding the moraine are composed of the Lake

Simcoe till plain, which is described as a gritty clay loam which coarsens towards the north (OGS, 1984).

The moraine was deposited during a halt in the retreat of the glacier margin. The moraine deposits in the vicinity of the pits are described as ice contact stratified drift (OGS, 1984) which are comprised of glacial outwash, possibly from north-western and eastern sources (OGS, 1984). The kame deposits consist of discontinuous layers of sand, gravel, silt and clay, as shown in the logs of borehole OGS90-8, on-site test wells, and private wells shown on Drawing 3. The east-west orientation of the moraine suggests it may not be related to the last glacial advances during the Wisconsinan glaciation period, which is believed to have been from the north-east in the vicinity of the Township of Oro. It may be an interlobate moraine or an overridden deposit of an earlier glacial advance (Chapman and Putnam, 1984). Source material for the moraine is of Precambrian origin, with very little Ordovician material (Chapman and Putnam, 1984).

2.4 Hydrogeology

The aggregate properties are located on the Bass Lake Kame Moraine. The local hydrogeology is illustrated in Sections A-A through D-D. Regional hydrogeology is illustrated in Drawing 3.

Confined aquifers in the lowland areas appear to be relatively continuous, as shown in Drawing 3. They are composed predominantly of sand with lesser amounts of gravel. Where the aquifers have been completely intersected south of the moraine, thicknesses are generally less than 5 m. Thicker sequences of sand are indicated north of the moraine. The limestone bedrock aquifer is at a depth of 50 to 200 m between the Coldwater River and Lake Simcoe. Adequate water supplies are normally obtained from the overburden aquifers, and therefore few wells are drilled to the deep rock aquifer.

The moraine materials were deposited on top of the regional deposits, as discussed in Section 2.3. Moraine deposits are complex and irregular, and therefore distinct hydrogeological units cannot be traced to any extent. The moraine is a complex sequence of sand, gravel, silt and clay. Two aquifers have been intersected in the vicinity of the properties, as illustrated in the sections. An upper aquifer (A1) is at elevations of approximately 310 to 330 masl, and another aquifer (A2) is at the base of the moraine at elevations mainly below 280 masl. Other aquifers intersected in some locations appear to be more localized.

Upper aquifer A1, intersected in the central moraine area, is under water-table conditions within the Allan G. Cook Limited, The Sarjeant Co. Ltd., and James Dick Construction Limited properties. It is confined in some locations, as at OW1/91 on the Allan G. Cook Limited property, where there is a sandy silt confining layer above the water table. At some locations, upper aquifer A1 is under artesian conditions, as at Wells 11786 and 19519 (Section B-B). The aquifer is generally composed of sand combined with gravel in some locations. The hydraulic conductivity of the aquifer, estimated from grain-size analyses and single well response testing (discussed in Section 3), is estimated to be 4×10^{-3} to 10^{-2} cm/s. These estimates represent conditions where wells were tested. The hydraulic conductivity may vary from these estimates at other locations. Aquifer characteristics are summarized in Table B-2.

The intersected aquifer thickness varies from 5 to 12 m on the Allan G. Cook Limited, The Sarjeant Co. Ltd., and the James Dick Construction Limited properties. This aquifer thickness and calculated hydraulic conductivity indicate an aquifer transmissivity of 20 to 110 m²/day. A specific yield or storativity of 0.05 to 0.2 is reasonable for a water-table aquifer. For calculation purposes, a value of 0.1 has been assumed.

Beneath the crest of the moraine, confined aquifer A2 is present at approximately 280 masl. This aquifer is not present at all locations drilled to this depth, indicating that the aquifer is discontinuous. Towards the flanks of the moraine, aquifer A2 appears to

be interconnected with aquifer A1, as indicated by the log of Well 18049 (Section B-B). Pumping test data (discussed in Section 3) indicate a transmissivity of 22 to 92 m²/day and a storativity of 5.6×10^{-5} for this aquifer at The Sarjeant Co. Ltd. property, and a transmissivity of 500 m²/day at the flanks of the moraine on the Seeley & Arnill Aggregates Ltd. property. Testing at this location indicated leaky confining layer conditions. The aquifer material intersected at the Seeley & Arnill Aggregates Ltd. property is approximately 10.5 m thicker and coarser-grained than that intersected at The Sarjeant Co. Ltd. property, which accounts for the difference in transmissivity values.

2.5 Groundwater Recharge

The moraine is an upland area with a predominantly sandy surface, which provides a portion of the recharge to regional aquifers between the moraine and Lake Simcoe to the south, and Matchedash Bay to the north. Natural recharge on the moraine proper appears to be almost equivalent to the water surplus, because surface runoff is minimal. At the James Dick Construction Limited property, it was noted by Charlesworth & Associates (1991) that during the spring melt, surface runoff was minimal and infiltration was quick. Fine-grained deposits are more evident at the flanks of the moraine, and the headwaters of the Hawkestone Creek and Coldwater River tributaries and associated wetlands indicate groundwater discharge and surface runoff. A significant portion of the water surplus will be surface runoff in these wetland areas.

Natural infiltration rates at the height of the moraine are expected to be up to the average annual water surplus of 417 mm equivalent to an average of 11.4 m³/day/Ha. On the flanks of the moraine, where surface materials are finer-grained and surface runoff is evident, annual infiltration likely will be less than 200 mm.

2.6 Water Table

The water table beneath the aggregate properties is at a measured geodetic elevation of approximately 315 to 327 masl, which is 25 to 39 m below ground level within the aggregate properties depending mainly on ground elevation. The thickness of the aquifer, where fully intersected, varies from 5 to 12 m.

The water table is highest in the central part of the moraine. This location is the groundwater drainage divide between the Coldwater River and Hawkestone Creek drainage basins. The drainage divide and water table are shown on Drawing 2 by means of equipotential lines or lines of equal water-level elevation. It generally follows the axis of the SW-NE oriented moraine. The groundwater divide and groundwater flow directions are discussed in Section 2.7.

There is a potential for perched water-table conditions in a moraine environment. Observation well DC1 on the James Dick Construction Limited property intersected saturated conditions above a till layer, at a depth of 32 m. This appears to be a perched water-table condition. Perched water was not found at DC5 (north-west of DC1), nor is a confining layer indicated at this depth at Well 8728 (to the south-east). Therefore, this condition does not appear to be extensive.

Reports of a possible spring in Lot 9, Concession 8 (County Forest) indicated a possibility of a perched water table. This possibility was investigated by the drilling of an observation well on the Alfa Aggregates Ltd. property, to a depth below the potential perched water-table condition. No confining layer was intersected, and the observation well remained dry. No spring was located in the county forest during field investigations. Staff of the MNR have reported that this is the location of a cistern and not a dug well, the purpose of which is to store water for fire protection. Therefore, there is no evidence of perched water-table conditions in the vicinity of the County Forest in Lot 9, Concession 8.

2.7 Groundwater Flow

A gravity flow system is evident in the upper aquifers within the Bass Lake Kame Moraine complex. This is illustrated in Drawing 2 by means of equipotential lines, or lines of equal water-level elevation, and groundwater flow lines showing the direction of groundwater flow as inferred from the equipotential lines. The groundwater flow system depicted in Drawing 2 is based on water levels monitored at observation wells constructed within the aggregate properties, and in the records of private wells filed with the MOE (Appendix A). Local flow details will be complex because of the variations in the moraine deposits, as indicated in the sections (Figures 3 through 6).

The flow system, as depicted in Drawing 2, indicates that the groundwater flow divide generally follows the axis of the moraine, and that the moraine is a recharge centre. Groundwater flow north of the drainage divide is northward to the Coldwater River basin, and groundwater flow south of the drainage divide is southward towards the Hawkestone Creek and Lake Simcoe drainage basin. The groundwater drainage divide, which generally follows the summit of the moraine, is between the 320 m equipotential lines. It is a broad area about 300 to 1500 m wide.

The flow system is sustained by recharge from the infiltration of precipitation to the mainly sandy soils and the upper overburden deposits, which are mainly granular. The recharge is estimated to be up to the annual water surplus of 417 mm (Section 2.1). Of this, most of the infiltration will take place in the spring as the winter snow melts and the ground thaws.

Boundaries to the flow system include the termination of the upper aquifer at or near the flanks of the moraine, and the fine-grained deposits which mark the base of the upper aquifer system and retard the vertical seepage of groundwater.

Discharge takes place along the flanks of the moraine, where it sustains the base flow to the tributary streams of the Coldwater River to the north and Hawkestone Creek to the south (Section 2.2).

Vertical hydraulic gradients are downward within the area of the moraine. The differences in groundwater elevation are in the range of 2 to 12 m within the upper aquifer system, and 6 to 37 m between the upper aquifers and the lower aquifers. The equivalent downward hydraulic gradients are in the range of 0.2 to 0.7 m/m. The log of TW8/91, in the southern part of The Sarjeant Co. Ltd. property, indicates a confining layer approximately 20 m thick which is composed of clay and sandy clay and which probably has a vertical hydraulic conductivity of 10^{-7} cm/s or less. A hydraulic conductivity of 10^{-7} cm/s or less, together with a vertical hydraulic gradient of 0.4 m/m, indicates a potential seepage of recharge to the lower aquifers of up to 0.3 m³/day/Ha. This is about 2% of the estimated recharge to the upper aquifer system and indicates that, where the confining layer has a hydraulic conductivity of 10^{-7} cm/s or less, most of the flow in the upper aquifer system discharges to the surface along the flanks of the moraine.

Groundwater flow in the lower aquifer system is similar to that in the upper aquifers. This flow is sustained by seepage from the upper aquifers through the intervening confining layers. The amount of this recharge probably varies from place to place and will depend on the vertical hydraulic conductivity of the confining layer(s).

2.8 Local Wells

There are 34 house wells located on or between the Sixth, Seventh and Eighth Lines, between County Road 11 and Bass Lake Sideroad. Wells 3268 and 3302 could not be located in the field. It is possible that they have been abandoned or that the locations given in the records are incorrect.

Locations of local private wells are shown on Drawing 1. The wells have been located from records filed with the MOE and field verification of all wells within 500 m of the aggregate properties. All local wells are constructed in the overburden aquifers.

Private wells in the area have been drilled to a depth of between 11 and 121 m. Recorded yields are in the range of 20 to 136 L/s^{m³/s} (4 to 30 IGM).

All well records indicate fresh water.

Many wells constructed in the upper aquifers of the moraine are confined, although interconnection between the upper confined aquifer and the water-table aquifer is evident as indicated on Section B-B. Wells constructed in the lower aquifers and most of the wells located north and south of the moraine are constructed in confined aquifers.

3. SITE SPECIFIC HYDROGEOLOGICAL ASSESSMENTS

Individual site specific hydrogeological assessments have been undertaken for the five privately-owned existing and proposed aggregate extraction properties.

Seeley & Arnill Aggregates Ltd., Concession 7, Part East Half Lots 10 and 11.

A hydrogeological assessment of this property was completed by Henderson, Paddon and Associates (1990). The investigation included testing of a water-supply well for a washing operation. Seeley & Arnill Aggregates Ltd. has a Permit to Take Water from two on-site wells at a combined rate of up to 2618 m³/day (400 IGM). There is no record for one of the wells. The other, 1/89, was constructed in 1989 and is located in the south-east part of the property. It is a 200 mm diameter well constructed in a 15.5 m thick confined sand and gravel aquifer, at a depth of 44 m at this location. The well was tested at 22.7 L/s for 24 hours. Water levels were monitored at the production well, a nearby observation well, an existing on-site well, and the Musicco private dug well. Interference was monitored at the on-site wells and observation well, but not at the Musicco well. The drawdown data indicated leaky aquifer conditions. An average transmissivity of 5.8×10^{-3} m²/s and a storativity of 4.4×10^{-4} were estimated from the test data (Henderson, Paddon, 1990).

Alfa Aggregates Limited, proposed Roehner pit, Concession 7, Part East Half Lot 9.
A hydrogeological assessment was undertaken by Dixon Hydrogeology Limited (1991). It was based on existing information augmented by the drilling of a borehole, OW1/91, to determine whether a perched water-table aquifer was present on the east side of the property, as suggested by observations made by a local resident. Perched groundwater was not identified at this location and is not indicated in well records of the two house wells located on the property. The locations of the observation well and the two house wells are shown on Drawing 1. One well (13106) is constructed in the lower confined aquifer, and the other well (11628) is constructed in the upper water-table aquifer.

James Dick Construction Limited, Concession 7, West Half Lots 7 and 9, Part Lot 8, and East Half Lot 8.

Assessment of hydrogeological conditions on this property includes existing information augmented by the drilling and construction of five observation wells (Charlesworth & Associates, 1991). Grain-size analyses were conducted on three samples from one of the boreholes. An analysis of the data using the Hazen formula indicates a hydraulic conductivity of 10^{-2} to 10^{-4} cm/s for the sand at this location. A step test was run on the on-site house well (19519) constructed in the upper aquifer to assess its yield capabilities. The data suggested a specific yield of 0.7 L/s/m for this well (Charlesworth & Associates, 1991). Water levels were monitored during the spring, summer and fall of 1991. The monitoring data are contained in Table B-3.

The Sarjeant Co. Ltd., Concession 8, West Half Lot 8.

The site specific hydrogeologic assessment conducted for this property by Dixon Hydrogeology Limited (1992) included the drilling of eight locations to identify a well site for a washing operation. Four of these are test wells, two are observation wells, and two were abandoned. One well was pump tested at 4.5 L/s (60 IGM) for 24 hours. The production well, three on-site observation wells, and private Well 20959 were monitored during the test. A transmissivity value in the range of 22 to 92 m²/day was calculated for aquifer A2 using the test data. Single well response tests were conducted on two wells constructed in the upper aquifer. The calculated hydraulic conductivity, using the Hvorslev method, is approximately 4×10^{-3} cm/s. Water levels were monitored in the water table and lower confined aquifer (Table B-3).

Allan G. Cook Limited, Concession 8, West Half Lot 7.

A hydrogeologic assessment for this property, conducted by Dixon Hydrogeology Limited (1992), included the drilling and construction of one observation well, OW1/91, to assess conditions for a washing operation well and to identify the water-table elevation. A single well response test was conducted on this well using a PVC rod slug. A hydraulic conductivity of 5×10^{-3} to 7×10^{-3} cm/s was calculated using the Hvorslev method. The

water-table aquifer is approximately 5 m thick at this location. The Ontario Geological Survey drilled and logged a borehole (OGS90-8) on this property, the log of which is contained in Section B-B.

4. EXTRACTION AND ASSOCIATED OPERATIONS

4.1 Extraction

Extraction of the aggregate pits will involve the removal of existing vegetation and soil within the pit areas. Sixty-one percent of the existing cover is predominantly deciduous forest and pine plantation, 34% is agricultural pasture and early succession, and 5% is open pits (Young, 1992).

The proposed total extraction area is 320 Ha. Individual licence and extraction areas are given in Table B-1. The proposed licensed annual tonnages and maximum average extraction rates are included in Table B-1. The proposed licensed annual tonnage represents the maximum allowable extraction. The maximum average extraction rate is equivalent to 50% of the licensed rates. It is the maximum expected extraction averaged over the six pits.

Extraction will remain above the water table at all the properties, with the possible exception of the James Dick Construction Limited property. James Dick Construction Limited proposes to investigate the potential for extraction below the water table at a later date. If they do extract below the water table, it will be by dragline operation and there will be no dewatering. Extraction below the water table will require a site plan amendment and review by all appropriate authorities, including the municipality.

The estimated available resource is approximately 145.5 million tonnes. The annual operating period is expected to average 190 days, with some shipping in the winter. The pits are expected to be in operation for 15 to more than 75 years, depending on size and market conditions.

4.2 Crushing Operation

The crushing and sizing of product is a dry process, save and except for dust control by wetting of the product. These operations will have no impact on groundwater or surface water.

4.3 Washing Operation

Five of the six licensed and proposed aggregate operations plan to wash a portion of the aggregate they produce. The operators anticipate washing a maximum of 25% of the aggregate which they extract, with the exception of The Sarjeant Co. Ltd., which proposes to wash 90% of its production. Seeley & Arnill Aggregates Ltd. have two on-site production wells and a Permit to Take Water (PTTW) from the MOE, at rates up to 30.3 L/s (400 IGM). They will not be operating continuously at the permitted rate. The wells will be operated for short periods at rates up to that indicated in the PTTW, as discussed in Section 5.3. A Permit to Take Water will be required by each of the other four operators before they can set up washing operations.

Water requirements for aggregate washing operations vary depending on the type and size of operation. Water requirements consist of the filling of storage tanks or pond(s) at the start of the operating season, and then replenishment of water lost to the recirculation system during the operating season.

At the start of the operating season, the storage tanks or ponds will be filled. Some water stored in pond(s) may remain from the previous season, depending on permeability of the pond liner. This storage replenishment will likely require the greatest pumpage of water. In most cases, the beginning of the operating season will coincide with the spring high water table, which will offset some of the interference from washing operation wells.

Water flows per operator for washing operations is expected to be up to 4000 m³/day (1200 IGM) operating 12 hours per day, as used at the Seeley & Arnill Aggregates Ltd. operation in the Township of Medonte. Most of the washing operation water will be recirculated. Water lost in the flow system must be replenished. This makeup water is estimated to be 655 m³/day for the combined operations, assuming makeup water requirements of 131 m³/day per operation as indicated for the Seeley and Arnill Township of Medonte operation. Some of this lost water will return as seepage to the groundwater system. The only losses to the aquifer system will be water evaporated from pond surfaces and stockpiles, and water retained and exported in the product.

Evaporation from the pond surfaces will exceed replenishment by precipitation during the summer months (June through August). An estimate of the average evaporation from the pond surfaces has been made by subtracting the Potential Evapotranspiration from the precipitation indicated in the water budgets. Actual evaporation is expected to be slightly greater because the Potential Evapotranspiration is calculated for a vegetated surface. The average pond evaporation indicated from the three climate stations is 117 mm, equivalent to 12.8 m³/day/Ha for the summer months. Evaporation from the stockpiles of washed product will vary depending on how regularly aggregate is added. This evaporation is expected to be minimal. An assumed value of 32 m³/day for the combined operations has been used based on one hectare of surface area of washed product per operation, with a wet cover for half of the operating period.

The exported water is estimated to be 7 to 10% of the sand by volume (Todd, 1963), or 3 to 5 % by weight assuming a saturated sand density of 2100 kg/m³. Exported water is calculated in Table C-4. Exported water is expected to be 140 to 234 m³/day for all five pits operating at their expected average annual extraction rate. This would increase to 280 to 467 m³/day with all pits at their proposed licensed tonnage.

Water evaporated from wash pond surfaces is estimated to be 64 m³/day, assuming a one-hectare pond per property. Evaporation from stockpiles is estimated to be 32 m³/day.

The exported water from the combined operations is estimated to average 234 m³/day with a maximum of 460 m³/day (Table C-4). Therefore, the average loss to the aquifer from the washing operations is approximately 330 m³/day and the maximum loss is approximately 563 m³/day over the operating period of 190 days.

The capture or recharge area required to sustain the losses to the aquifer complex (equivalent to evaporated and exported water) would be 30 to 49 Ha, which is 9 to 15% of the licensed areas, assuming an infiltration rate equivalent to the natural water surplus of 417 mm/year. Losses do not include water which re-infiltrates and returns to the aquifer system.

The average amount of water which is lost to the system as infiltration from wash ponds and stockpiles is estimated to be 325 m³/day. This estimate is based on washing operations at Seeley & Arnill Aggregates Ltd. in the Township of Medonte. The water lost to the washing operations by infiltration from the wash ponds and stockpiles will recharge the aquifer and ultimately recirculate to the washing operation.

4.4 Stockpiles

Both washed and unwashed product will be stockpiled. There will be some drainage of water from the washed product which will re-infiltrate and return to the groundwater system. Some of the moisture near the surface of the pile will evaporate, resulting in a loss to the aquifer system. These losses have been discussed in Section 4.3.

4.5 Shipping

Most of the product shipped from the sites will be dry sand and gravel. The washed product shipped from the sites will contain water which adheres to the sand and gravel particles. This exported water is discussed in Section 4.3.

4.6 Dust Suppression

Dust suppression within the existing and proposed pits will be by the application of water or approved chemicals such as calcium chloride. Spraying of water or approved chemicals will occur on unpaved site roads and crushed product. Paving of the entrances to the site will reduce the need for dust suppression.

Application of calcium chloride would result in an increase of calcium and chloride in the underlying groundwater as the chemical is dissolved in infiltration. There is no water quality objective for calcium. Chloride is an aesthetic-related water quality parameter with a maximum desirable concentration of 250 mg/L, as indicated in the Ontario Drinking Water Objectives (ODWO). The MOE Central Region Barrie District office has advised that Policy No. 15-08 (MOE, 1986) would apply to the use of calcium chloride as a dust suppressant, which will limit the amount of calcium chloride or other chemicals used. This is discussed in Section 5.8.

4.7 Fuelling and Equipment Maintenance

The properties which intend to store fuel on-site have included an impermeable based containment area on their respective site plans.

To ensure protection of the groundwater, the operators have prepared a spill contingency plan and will do everything practical to prevent, eliminate, and ameliorate spills which may have an adverse impact on the environment, in accordance with The Environmental Protection Act, Part IX. All waste oil from on-site equipment maintenance will be taken off-site and disposed of in accordance with the applicable regulations.

5. IMPACTS OF OPERATIONS

5.1 Water Budget

Components of a water budget include precipitation, evaporation from soils and transpiration from vegetation (combined as evapotranspiration), surface runoff, and infiltration to recharge groundwater. The runoff and infiltration make up the water surplus.

There will be changes in the water budget as a result of the extraction operations. The changes will be the amount of evapotranspiration and water surplus as the natural vegetation is removed. The water surplus is a combination of the amount of water available for infiltration to recharge groundwater, and for runoff (Section 2.1). The excavations resulting from the extraction of aggregate will intercept all the potential runoff within the area of the excavations, and therefore all of the available water surplus will infiltrate to recharge groundwater. There is very little evidence of significant surface runoff in the upland moraine area, and therefore an increase in the amount of groundwater recharge may not be significant as a result of the containment of all runoff within the excavation.

Various water budget models have been developed to provide water budgets for both regions and localized areas. The Canadian Climate Centre of Environment Canada uses the Thornthwaite model (1944) to provide water budgets based on data from climate stations and adjusted for various soil types. This model is not adjusted for different vegetation covers. A summary of the results of this model for the Orillia and Midhurst climate stations is included in Table C-1.

The Hargreaves model (1956) includes a coefficient for vegetation types. This model was used to provide an indication of the differences between potential evapotranspiration from a forest and from an open pasture. The coefficients used were calculated for a pasture

and a deciduous orchard in Davis, California. The data are for the frost-free season only and are contained in Table C-2.

Both the Thornthwaite and Hargreaves models can be solved using climate data collected at most climate stations. Other models, discussed below, require more detailed climate data which is not available for the Midhurst or Orillia stations.

Shiau and Davis (1973) used a modified Penman model (1948) to assess the potential evapotranspiration rate from a deciduous forest in New Brunswick. They compared their results with various models, including the Thornthwaite and Hargreaves models, and found that the modified Penman model agreed closely with the Thornthwaite model, and that the Hargreaves model indicated higher values of potential evapotranspiration.

Various studies have been undertaken to estimate potential evapotranspiration rates from forests and to assess changes to evapotranspiration rates as a result of deforestation. Johnston (1976) compared soil moisture losses over the growing season from a mature Aspen forest, a plot of grasses and annuals, and a bare plot located in Utah. Minimal runoff occurred from the plots, with the exception of the bare plot, where the measured runoff was discounted from precipitation. The study results indicated that the estimated evapotranspiration from the bare plot was approximately 53% of evapotranspiration from the mature Aspen plot, and evapotranspiration from the herbaceous plot was 73% of evapotranspiration from the mature Aspen plot.

Swift *et al* (1975) modelled evapotranspiration and drainage from a mature deciduous forest, a pine plantation, and a clear cut area. The model indicated greater evapotranspiration losses from the pine plantation than the deciduous forest, as a result of a greater rate of evapotranspiration during the winter season. Evapotranspiration losses were indicated to decrease as a result of clear cutting the deciduous forest. The model results agreed closely with measured stream flow changes that were produced by clear cutting hardwood forests and converting to pine plantation.

The climatological data available from the closest climate stations is insufficient to produce a detailed water budget model for the existing vegetation covers and an open pit. The differences in evapotranspiration for the various covers, as indicated in the References and calculated using the Hargreaves equation, have been used as a guide in assessing potential changes to the local water budget as a result of clearing and excavating the pits.

The Hargreaves model indicates that the difference in potential evapotranspiration rates between a deciduous orchard and a pasture averages approximately 208 mm per growing season (Table C-2). The Johnston (1976) study indicated an evapotranspiration reduction of 47% between the mature Aspen and bare soil plots. The Swift *et al* (1975) study noted a predicted and measured increase in stream flow of 36 to 41 cm as a result of clear cutting a mature deciduous forest.

The maximum indicated change in evapotranspiration is an approximate 50% reduction as a result of clear cutting, equivalent to 250 mm (50% of the average evapotranspiration indicated in Table C-1). This value has been used as a maximum expected decrease in evapotranspiration and equivalent increase in water surplus. Although the decrease in evapotranspiration may not be equivalent to this maximum impact, some decrease in evapotranspiration will occur as a result of removal of vegetation. A decrease in evapotranspiration and increase in water surplus of approximately 10% (50 mm/annum) has been assumed as a minimum impact, representing the difference between evapotranspiration from a herbaceous plot and evapotranspiration from a bare plot. An average value of 150 mm has been included in the calculations presented in Table C-3.

The operation of wells to provide water for washing operations will result in a removal of water from the aquifer system. Only the evaporated and exported water will be lost to the local groundwater. These losses will be realized over each operating season and are discussed in Section 4.3.

If aggregate is extracted below the water table as ultimately proposed by James Dick Construction Limited, a pond will form within the excavated area, the level of which will represent the water table. A localized change in the water budget will result. The change will be an increase in evaporation to the potential evaporation rate, which will be greater than in the naturally-vegetated areas and the pit areas which are above the water table.

The net impacts on the local water budget are summarized in Table C-3. The average net increase in groundwater recharge is expected to be 572 m³/day resulting from the combined properties.

Changes to the local water budget will be progressive as extraction of the pits begins and washing operations start. Additional infiltration or groundwater recharge, resulting from clearing and pit excavation, will increase as the size of the pits increases, until rehabilitation begins and the vegetation cover thickens. Water removal resulting from washing operations will remain steady throughout the life of the pits, assuming steady market conditions.

After pit operations are complete, the recharge gains will be limited to capture of all surplus water to infiltrate within the extraction areas. Evapotranspiration will return to pre-extraction rates, assuming the properties are rehabilitated approximately to their existing vegetated conditions; however, evaporation from ponds created by extraction below the water table will continue. Water losses resulting from washing operations will cease.

5.2 Water Tables

The removal of vegetation and soil and the excavation of pits will result in an increase in groundwater recharge, as discussed in Section 5.1. This increased recharge will be

partly offset by water losses resulting from washing operations, and therefore the average net impact on the local water budget is expected to be 572 m³/day.

An increase in the amount of groundwater recharge is expected to result in a mounding of the water table beneath the excavations and, to a lesser extent, in the surrounding areas. The mounding will apply to the main water table, as described in Section 2.6 and as illustrated in Drawing 2, and to local perched water tables if and where they exist (Section 2.6).

Mounding of a perched water table would occur until the excavation extends to depths below the perched water table and the underlying confining layer was removed. The perched water table in the area of the excavation would be removed, and drainage of at least part of the perched water table in adjacent areas would occur.

Perched water was intersected at DC1 on the James Dick Construction Limited property, as discussed in Section 2.6. There are no known wells constructed in this aquifer, and this perched water table is at too great a depth to influence local vegetation; therefore, a loss of part or all of this perched condition is not expected to have a detrimental impact on water supplies or the natural environment.

The potential mounding of the main water table has been assessed based on the anticipated increase in groundwater recharge (Section 5.1) and a range of transmissivity and storativity values representing the upper aquifer (Section 2.4).

The extent and degree of water table mounding has been assessed using the Theis and Darcy formulae. The Darcy formula is based on equilibrium conditions and indicates a potential mounding of up to 0.7 m beneath the pits (Table C-6), decreasing away from the pits to an extent of 7 to 30 m, assuming an average recharge increase. Theis calculations are based on non-equilibrium conditions, and therefore a period of one year

has been assumed to represent equilibrium conditions. The equations are included in Table C-5.

Theis calculation results agree with the Darcy results. The low transmissivity values indicated to represent the aquifer (Section 2.4) will limit the extent of mounding away from the excavations. After rehabilitation, the increased recharge will be reduced as evapotranspiration returns to natural levels, and therefore mounding will also decrease.

James Dick Construction Limited proposes to investigate the potential for extraction below the water table. Aggregate extraction below the water table will result in a small lowering of the water table as water replaces the extracted volume of aggregate. As extraction proceeds and the size of the pond increases, the effect on water levels will decrease. Pond level declines are expected to be minimal, and therefore water-table lowering in the surrounding area is expected to be negligible.

5.3 Water Supply Wells

Wells which are operated to provide water for washing operations will cause a decline in the water table or potentiometric surface as the wells are operated. The water levels will recover when the wells are not operating. The water level declines will be greatest at the wells, and will decline with increased distance from the wells.

Most of the time, the wells would be operated for short intervals to provide makeup water to replace the water not recirculated within the system. Makeup water replaces water which has evaporated, been exported with the product, and re-infiltrated the ground surface. Actual water losses to the area will be as evaporation from the wash ponds and stockpiles, and as water retained in the product. Makeup water requirements, including re-infiltrating water, are estimated to be 655 m³/day for the combined operations. The indicated losses are equivalent to average daily flows of 7.6 L/s (100 IGM). In practice,

the wells would be operated for only short periods, depending on their equipped yields. With efficient washing operations designed to conserve water, the amount of water which would re-infiltrate the ground would be minimized. The average cumulative water loss is approximately 37% of the average cumulative recharge increase resulting from vegetation clearing and pit excavations.

The most prolonged well operations will be in the spring or at the beginning of the operating season, when water is pumped into storage for the washing operations.

Assuming water for each washing operation is stored in a one-hectare, 0.2 m deep pond, the following time periods would be required to fill the storage at each operation at the start of the operating season:

Storage (2000 m ³)				
Pumping Rate (L/s) (IGM)	3.7 50	7.6 100	15.2 200	30.3 400
Time (days)	6	3	1.5	0.8

Seeley & Arnill Aggregates Ltd. have an on-site washing operation. Water use has been limited to less than 50,000 L/day due to limited production, but greater water use is anticipated by the operator when the permanent washing operation is set up. Seeley & Arnill Aggregates Ltd. anticipate water use to be similar to an operating pit they own in the Township of Medonte. This operation includes 227 m³ of storage in holding tanks, approximately 1575 m³ of storage in a wash pond, and some additional storage in a sludge pond, for a total of approximately 2000 m³. The recirculated flow is approximately 91 L/s (1200 IGM). Estimated makeup water requirements for this operation are 131 m³/day (20 IGM) during the summer months.

The extent of interference resulting from pumping washing operation wells has been estimated using the Theis equation. Aquifer transmissivity and storativity values indicated from pump test data for the Seeley & Arnill Aggregates Ltd. and The Sarjeant Co. Ltd. properties have been used for these calculations, which are contained in Table C-7. The low transmissivity values indicated from testing on The Sarjeant Co. Ltd. property will limit the extent of interference away from the wells. Water level drawdowns in the vicinity of extractions will be partially offset by water table mounding beneath the pits.

Interference in the range of 0.3 to 1.0 m is indicated at a distance of one kilometer from the well, for the pumpage to refill storage at the start of the season. The potential impacts on private wells is discussed in Section 5.4.

5.4 Private Wells

Five of the six aggregate properties propose to wash a portion of the aggregate they extract. Water to supply these washing operations will be obtained from wells, as discussed in Section 5.3. Pumping these wells will result in a drawdown of the water. The drawdown will be at a maximum at the well and will decrease at increased distance from the well. Water level drawdowns may interfere with water levels at nearby private wells. An interference of 0.8 m on Well 20959 (located as shown on Drawing 1) was indicated at the end of a 24-hour pumping test of The Sarjeant Co. Ltd. well TW8/91.

The potential interference with private wells from individual washing operations is addressed in the individual reports and/or will be evaluated when washing operation wells are drilled and tested.

The private wells located on the Seventh Line have the potential to be impacted by more than one operation. The potential combined interference on these private wells, from pumping at the five properties, is estimated in Table C-8. The estimates are for storage

refilling at the start of the season and replenishment of operating losses. Storage refilling at each property is anticipated to require a pumping period of one to six days, for rates of 3.7 to 30.3 L/s (Section 5.3), assuming 2000 m³ storage. Makeup water requirements vary depending on the type and size of the washing operation. Estimated makeup water requirements (including re-infiltrating water) for the five operations is 655 m³/day, based on requirements indicated at the Seeley & Arnill Aggregates Ltd. pit in the Township of Medonte. Pumpage of makeup water will likely take place for short periods throughout the day.

The maximum cumulative interference at the four closest wells is estimated to be 5 to 7 m (Table C-8) when storage is filled for the washing operations. This will be in the spring when natural water levels will be highest. The estimate is based on the assumption that all wells are operating at the same time, and on assumed pumping well locations, yields, and aquifer characteristics. The recorded static water levels for these wells is greater than 20 m above the screen levels. A loss of water supply is not anticipated. Actual impacts on private wells will have to be evaluated when the wash wells are drilled and tested.

The potential interference for each washing operation well will have to be assessed when the wells are drilled. These estimates should include impacts of other washing operation supply wells that have been constructed to date, to indicate the cumulative impact on private wells. A Permit to Take Water will be required by each of the operators proposing to set up a washing operation. Such Permits will require a pumping test to assess yield and interference with private wells. It will be necessary under each Permit to replace any private water supplies lost as a result of the operation of these wells.

5.5 Hawkestone Creek

Flow in Hawkestone Creek is sustained in part by drainage from the extraction area south of the drainage divide (Drawing 2).

The aggregate properties within this drainage basin are those of the Township of Oro, Seeley & Arnill Aggregates Ltd., Alfa Aggregates Limited, and the southern approximately 35 Ha of the James Dick Construction Limited property.

These properties are approximately 17% of the kame moraine area which drains to Hawkestone Creek.

There are no defined tributary streams of Hawkestone Creek which originate on or adjacent to these properties, and discharge to the creek is mainly groundwater providing base flow.

Hawkestone Creek
Expected increases in groundwater recharge are described in Section 5.1. The expected increases in recharge are in part realized at the Oro Township and Seeley & Arnill Aggregates Ltd. properties, and expected at the Alfa Aggregates Limited and part of the James Dick Construction Limited property, are estimated to be 422 m³/day as an average and 781 m³/day as a maximum. This will be a measure of a direct increase in the base flow to Hawkestone Creek and increased recharge to underlying aquifers. This is 2 to 4% of flow monitored at the Fifteenth/Sixteenth Sideroad (Section 2.2). Actual increased recharge is expected to be less because of progressive rehabilitation. The estimates provide an approximate indication of increases in the flow of the stream. Precise estimates are not possible with the information. Impacts on the swamp are not expected to be significant and would only be noticeable during the dry season, if at all.

5.6 Coulson Swamp and Coldwater River Basin

Flow within the Coldwater River basin is sustained in part by infiltration within the aggregate properties north of the drainage divide (Drawing 2). The aggregate properties within this drainage basin include The Sarjeant Co. Ltd., Allan G. Cook Limited, and the majority of the James Dick Construction Limited property (approximately 135 Ha). These properties comprise approximately 5% of the kame moraine area which drains to the confluence of the tributaries which follow the Seventh and Eighth Lines of the Township of Oro (Drawing 2). These tributaries have their headwaters approximately 600 m north of the properties. Coulson Swamp forms part of the headwaters to the Eighth Line tributary.

A stream gauge station is located on the Coldwater River in Coldwater, and data are available for the period 1965 to 1988. Spot flow monitoring at the bridge on the Eighth Line tributary to the Coldwater River and at the culvert beneath Rowanwood Road, provide an indication of flow near the headwaters of these tributaries (Section 2.2). The average potential increase to base flow for this basin is estimated to be 310 m³/day. The expected maximum increase is 640 m³/day, equivalent to less than 1% of base flow at the Coldwater River gauging station. Flow in the Seventh Line tributary to the Coldwater River was measured at 3076 m³/day on 26 November 1991. Flow in the Eighth Line tributary was measured at 14221 m³/day on the same date. Increased recharge to the west tributary is estimated to average 124 m³/day with a maximum increase of 256 m³/day or 8% of the flow monitored on 26 November 1991. The average increased recharge to the east tributary is estimated to be 186 m³/day with a maximum increase of 384 m³/day or 3% of the monitored flow.

5.7 Soil Moisture

The excavations are in areas where the water table (and intersected perched water table) are 25 to 39 m below ground level and below the expected root zones of the vegetation which relies on retained soil water for growth.

The soil moisture will be depleted immediately adjacent to the excavations. The excavation setbacks will provide buffer zones between the excavations and the property lines, and maintenance of vegetation within the buffer zone will limit the extent of drying from the pit edge.

5.8 Water Quality

The water quality has been analyzed at the Seeley & Arnill Aggregates Ltd. and The Sarjeant Co. Ltd. properties as shown in Table B-4. The water was within Ontario Drinking Water Objective (ODWO) standards for all parameters tested.

The water-table aquifer is unconfined over most of the moraine and is therefore susceptible to contamination. Servicing of equipment will involve portable oil collection systems. Waste oil will be taken off-site and disposed of according to applicable regulations. If fuels are stored on-site, they will be on concrete containment pads.

Dust suppression will be achieved by the spraying of water, or MNR approved chemicals such as calcium chloride. The applications of calcium chloride must be limited to comply with the MOE Reasonable Use Policy 15-08, which states that the application of a contaminant must be limited to ensure the water quality is not deteriorated by more than 50% of the difference between the ODWO and the background concentration. Under the Reasonable Use Policy, the maximum allowable application of calcium chloride is 808 kg/Ha, assuming an annual precipitation infiltration rate of 417 mm (Table C-9),

which is the existing water surplus. Increased recharge within the pits will further dilute the chloride concentration in the infiltration.

Salting of the Seventh Line will also introduce an increase in chloride to the surface and shallow groundwater, similar to salting of other roads in Simcoe County. The Township minimizes salting on the roads by limiting the application to 95% sand and 5% salt.

The water surplus within the pits will infiltrate the pit floor quickly during periods when the ground is thawed, and therefore the water will not be at a greater temperature than the existing infiltration. The temperature of groundwater discharging to the Coldwater River and Hawkestone Creek basins will not change as a result of the pit excavations.

CONCLUSIONS

1. The six existing and proposed aggregate properties are mostly within an Aggregate Resource Area of Primary Significance.
2. The estimated reserves for the six properties are 145.5 million tonnes. Depending on market conditions and property size, the operations have a life expectancy of 15 to more than 75 years.
3. The total proposed extraction area is 320 Ha. The extraction properties range in size from 25 Ha to 149 Ha.
4. Extraction will remain above the water table, with the possible exception of the James Dick Construction Limited property, which will investigate the potential of a dragline extraction below the water table. Dewatering will not occur.
5. The on-site operations will include clearing and extraction, crushing and sizing, stockpiling and shipping, and washing at five of the properties.
6. Removal of vegetation and soil and the creation of pits will decrease evapotranspiration and contain all surface runoff, resulting in an increase in infiltration.
7. Most of the water used in the washing operations will be recirculated or will re-infiltrate to the groundwater system. Some water will evaporate from the wash ponds and washed product and will be retained in the product exported from the site. This water will be lost to the local groundwater flow system.

8. The net impact on the local water budget will be an increase in infiltration. As operations start up and cleared and extracted areas are limited, water losses from the washing operation may exceed recharge increases resulting from clearing.
9. A slight increase in base flow to the Hawkestone Creek and Coldwater River basins is expected as a result of increased recharge within the aggregate pits. The extractions will not result in a change in temperature of groundwater discharging to the streams.
10. Five properties propose to wash a portion of the aggregate extracted. Seeley & Arnill Aggregates Ltd. has a Permit To Take Water from two wells for their washing operation. The other four operators will have to apply for a Permit To Take Water, and three of the operators will have to drill and test a well or wells. A condition included in Permits to Take Water requires that private water supplies be replaced if lost as a result of the permitted operation.
11. Water losses resulting from the washing operations are expected to remain relatively constant throughout the operating period.
12. Operation of wells to provide water for washing operations will result in a drawdown in water levels in the vicinity of the wells. This may result in a drawdown in water levels at local wells. Anticipated interference resulting from five washing operations is estimated to be up to 7 m at private wells on the Seventh Line, assuming simultaneous storage refilling at all properties. This will have to be verified by testing of wells when they are drilled. Interference will be limited to short periods, and all storage filling will not necessarily be simultaneous. The recorded static level in the wells on the Seventh Line are greater than 20 m above the screen levels. The estimated interference is not expected to result in a loss of water supplies. It will be necessary to obtain Permits to Take

Water from the Ministry of the Environment in order to operate these washing systems. A condition of the Permits will protect private water supplies.

13. Application of dust suppressants, such as calcium chloride, will result in an increase in calcium and chloride concentrations in the infiltrating water. The applications should be limited to comply with Policy 15-08 of the Ministry of the Environment.

RECOMMENDATIONS

1. The water-table elevation should be monitored at each property. Initially, measurements should be taken bi-monthly to establish seasonal fluctuations, after which at least quarterly measurements should be taken to assess/verify impacts on the water levels. If impacts warrant, the monitoring schedule should be altered. Observation wells should be located at the upgradient and downgradient sides of the property. Existing wells constructed on some of the properties would be used in a monitoring network.
2. At properties with a washing operation, water levels should be monitored at an observation well constructed in the same aquifer as the production well. Monitoring should be monthly during the operating season.
3. Extractions should remain at least 1.5 m above the high water table elevation in the spring plus an allowance for the anticipated water-table mounding which is estimated to be up to 1 m and should be verified by monitoring water levels.
4. The cumulative impact on pertinent private wells should be assessed when washing operation wells are tested, prior to applying for a Permit To Take Water.
5. Water samples should be obtained from existing and proposed wells and observation wells on each property and analyzed to document background water quality. The analysis should include general chemical parameters and phenols. Water quality monitoring should be conducted on a yearly basis.
B-H
6. With the permission of the owners, background water quality should be established for private wells located within 300 m of the extraction properties, and additionally for private wells on the Sixth, Seventh and Eighth Lines between County Road 11 and Bass Lake Sideroad. The analyses should include general

chemical parameters, phenols, fuels, lubricants, and solvents. If on-site water quality monitoring indicates an impact on the water quality, downgradient private wells should be monitored, again with the permission of the owners.

7. If calcium chloride is used as a dust suppressant, its annual application should be limited to 808 kg/Ha to meet the Ministry of the Environment Policy 15-08. Chloride concentrations should be analyzed annually at the downgradient property boundary where calcium chloride is being used as a dust suppressant. This would be included in the annual monitoring discussed in Recommendation 5 above.

DIXON HYDROGEOLOGY LIMITED



per: Sharon J. Lee, B.Sc.



per: V. R. Dixon, P.Eng.



January 11, 1992.

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ORO SEVENTH LINE AGGREGATE PITS
HYDROGEOLOGICAL STUDY

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ORO SEVENTH LINE AGGREGATE PITS

HYDROGEOLOGICAL STUDY

GLOSSARY OF TECHNICAL TERMS

AESTHETIC

Refers to those aspects of drinking water quality that are perceptible by the senses, namely taste, odour, colour and clarity.

AQUIFER

An aquifer is a formation, group of formations, or part of a formation that contains sufficient saturated permeable material to yield significant quantities of water to wells and springs.

AQUIFER, CONFINED

A confined aquifer is under pressure significantly greater than atmospheric and its upper limit is the bottom of a bed of distinctly lower hydraulic conductivity (aquitard) than that of the material in which the confined water occurs.

AQUIFER, PERCHED

A perched aquifer is unconfined groundwater separated from an underlying body of groundwater by an unsaturated zone. Its water table is a perched water table. It is held up by a perching bed (aquitard) whose permeability is so low that the water percolating downward through it is not able to bring water in the underlying unsaturated zone above atmospheric pressure.

AQUIFER, UNCONFINED

An unconfined aquifer is bounded by a water table at its upper surface.

AQUITARD

An aquitard is a body of "impermeable" material stratigraphically adjacent to one or more aquifers. Its hydraulic conductivity, relative to that of the aquifer it confines, should be specified such as relatively impermeable or moderately permeable.

CONFINING LAYER

A layer or strata of rock or soil of low hydraulic conductivity which is stratigraphically adjacent to one or more aquifers.

DIAMICTIC

Unconsolidated, unstratified, poorly-sorted material of variable soil particle sizes.

HEAD, STATIC (h)

The static head is the height above a standard datum of the surface of a column of water than can be supported by the static pressure at a given point. The static head is the sum of the elevation head, h_e ; and the pressure head, h_p ; that is,
$$h = h_e + h_p$$
. Head is usually measured in metres.

HYDRAULIC CONDUCTIVITY (K)

Ratio of flow velocity to driving force (hydraulic gradient) for viscous flow under saturated conditions of a specified liquid in a porous medium. Hydraulic conductivity is commonly measured in units of cm/s or ft/day.

HYDRAULIC GRADIENT (i)

Maximum increase in hydraulic head per unit length of flow path.

HYDROGEOLOGIC UNIT

An aquifer, an aquiclude, or a combination of aquifers and aquiclude that compose a framework for a reasonably distinct hydraulic system.

PERMEABILITY

The ability of a porous medium (i.e. soil, rock) to transmit water.

PIEZOMETER

A tube or pipe placed into water-bearing zones in soil or rock to enable measurement of water level.

POROSITY (n)

The porosity of a rock or soil is its property of containing interstices, or voids. With respect to the movement of water, only the system of interconnected interstices is significant. Porosity is the ratio of void volume to total rock volume and is expressed as a percentage.

POTENIOMETRIC SURFACE

The potentiometric surface, replacing the term piezometric surface, is a surface which represents the static head or groundwater level. As related to an aquifer, it is defined by the levels to which water will rise in sealed piezometers or observation wells. The water table is a particular potentiometric surface.

SATURATED ZONE

In the saturated zone, all voids are ideally filled with water. The water table is the upper limit of this zone and the water in it is under pressure greater than atmospheric.

SINGLE WELL RESPONSE TEST

A test designed to determine the hydraulic conductivity of formation materials adjacent to a piezometer screen. The method typically is initiated by causing an instantaneous change in piezometer water levels and measuring the subsequent change in water level with time.

SPECIFIC YIELD (S_y)

The specific yield of a rock or soil is the ratio of (1) the volume of water which, after being saturated, will yield by gravity, to (2) its own volume. The definition implies that gravity drainage is complete. In the natural environment, specific yield is generally observed as the change in the amount of water in storage per unit area of unconfined aquifer that occurs as the result of a unit change in head.

STORAGE COEFFICIENT (S)

The storage coefficient is the volume of water an aquifer releases from or takes into storage per unit surface area of the aquifer per unit change in head. In a confined water body, the water derived from storage with decline in head comes from expansion of water and compression of the aquifer. Similarly, water added to storage with a rise in head is accommodated partly by compression of the water and partly by expansion of the aquifer. In an unconfined water body, the amount of water derived from or added to the aquifer generally is negligible compared to that involved in gravity drainage or filling of pores. In an unconfined water body, the storage coefficient equals specific yield. The storage coefficient is usually regarded as a dimensionless characteristic of aquifers.

TILL

Heterogeneous, non-stratified sediment deposited by a glacier. It is characterized by its wide spectrum and variable ratio of soil particle sizes and by its high density.

TRANSMISSIVITY (T)

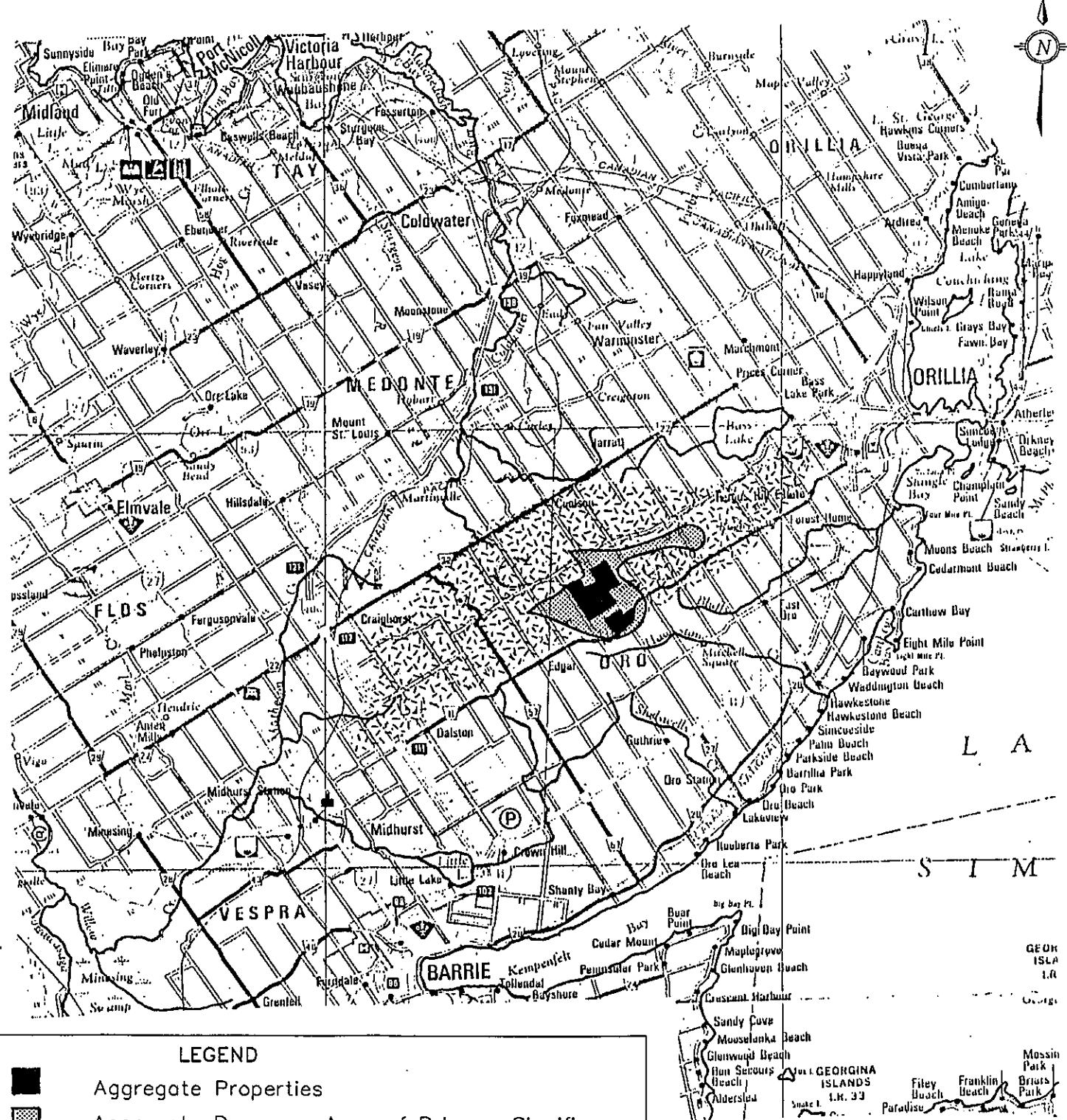
Transmissivity is the rate at which water is transmitted through a unit width of the aquifer under a unit hydraulic gradient. It is the product of hydraulic conductivity and aquifer thickness. The units of transmissivity are gallons/day/foot, ft²/day, or m²/second.

UNSATURATED ZONE

The unsaturated zone is the zone between the land surface and the water table. It includes the capillary fringe. This zone contains liquid water under less than atmospheric pressure and gases usually at atmospheric pressure. The term "unsaturated zone" replaced the terms "zone of aeration" and "vadose zone".

WATER TABLE

The water table is that surface in an unconfined water body at which the pressure is atmospheric. It is defined by the levels at which water stands in wells that penetrate the water body just far enough to hold standing water.



metres
0 2500 5000 10000 20000
SCALE 1:250000

SOURCES: Base Map from Ministry of Transportation, ONTARIO TRANSPORTATION MAP SERIES; Map 5, 1984.
Aggregate Resources from Ontario Geological Survey, AGGREGATE RESOURCES INVENTORY, ORO TWP., Map 2.
Physiography from THE PHYSIOGRAPHY OF SOUTHERN ONTARIO, L.J. Chapman & D.F. Putnam, 1984.

Drawn R. Draper	Job No. H09 Q01	Date: 30 Dec 91
Approved	ACAD: H09PI RFG	Dwg No. 91314

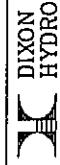
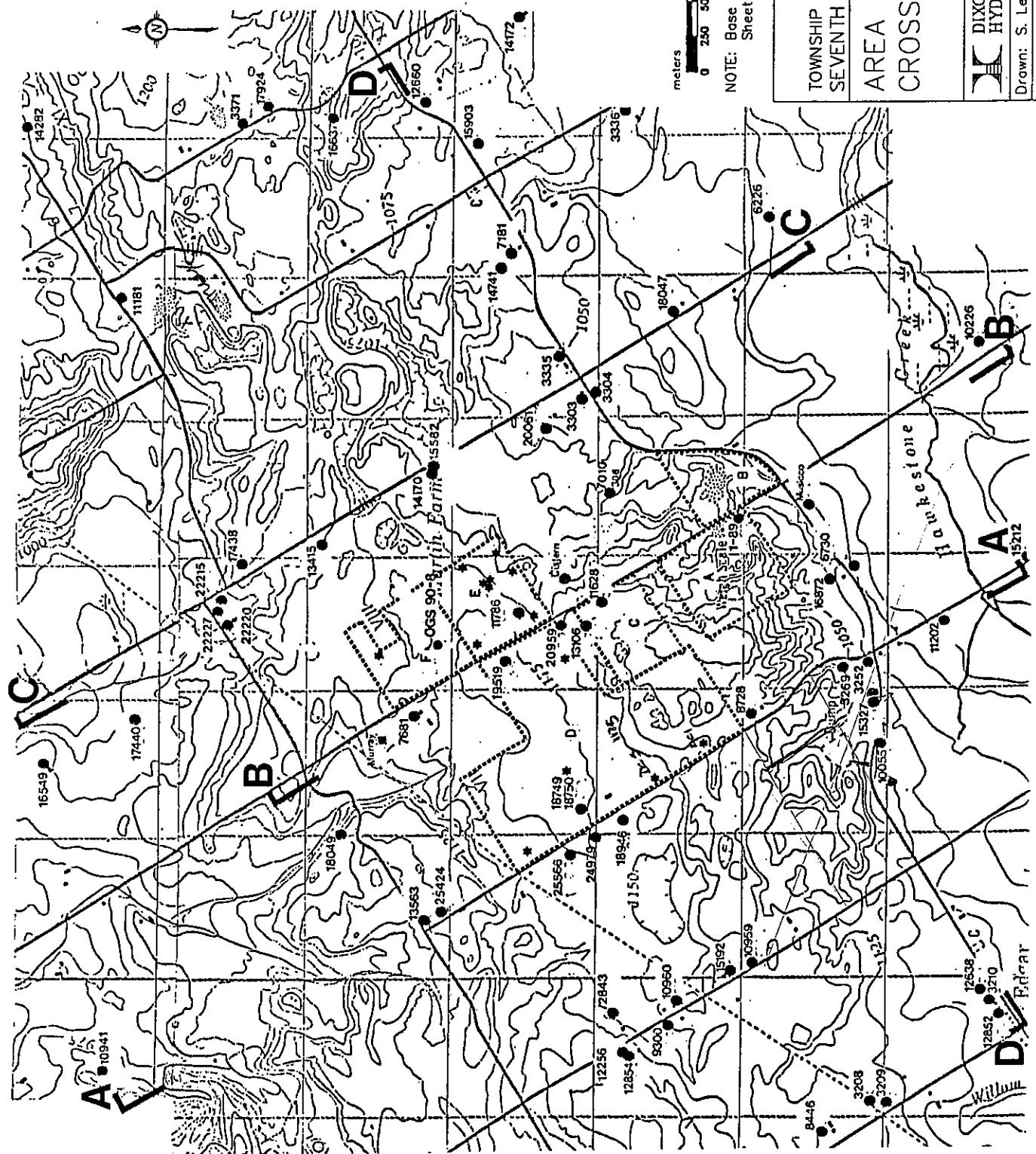
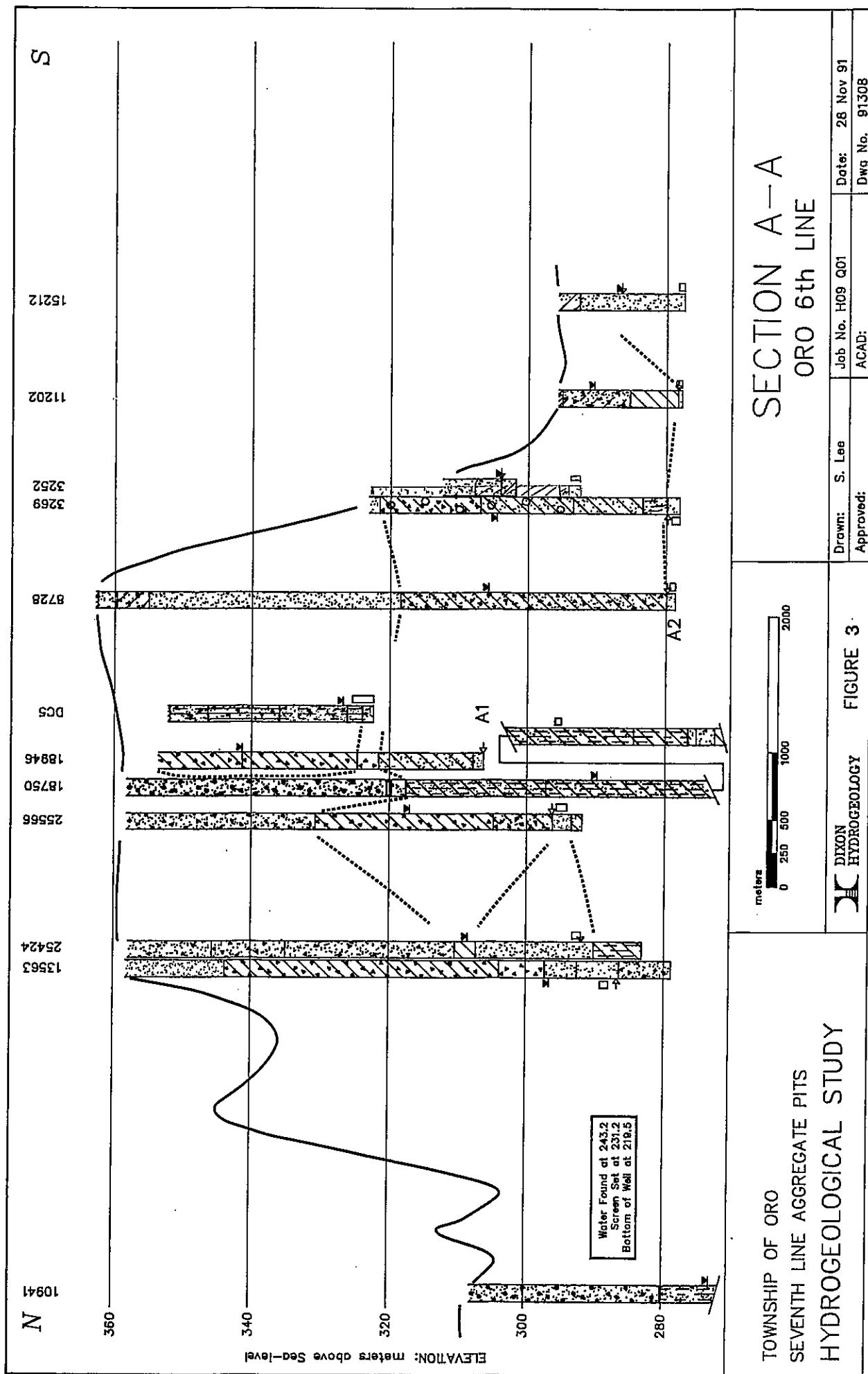
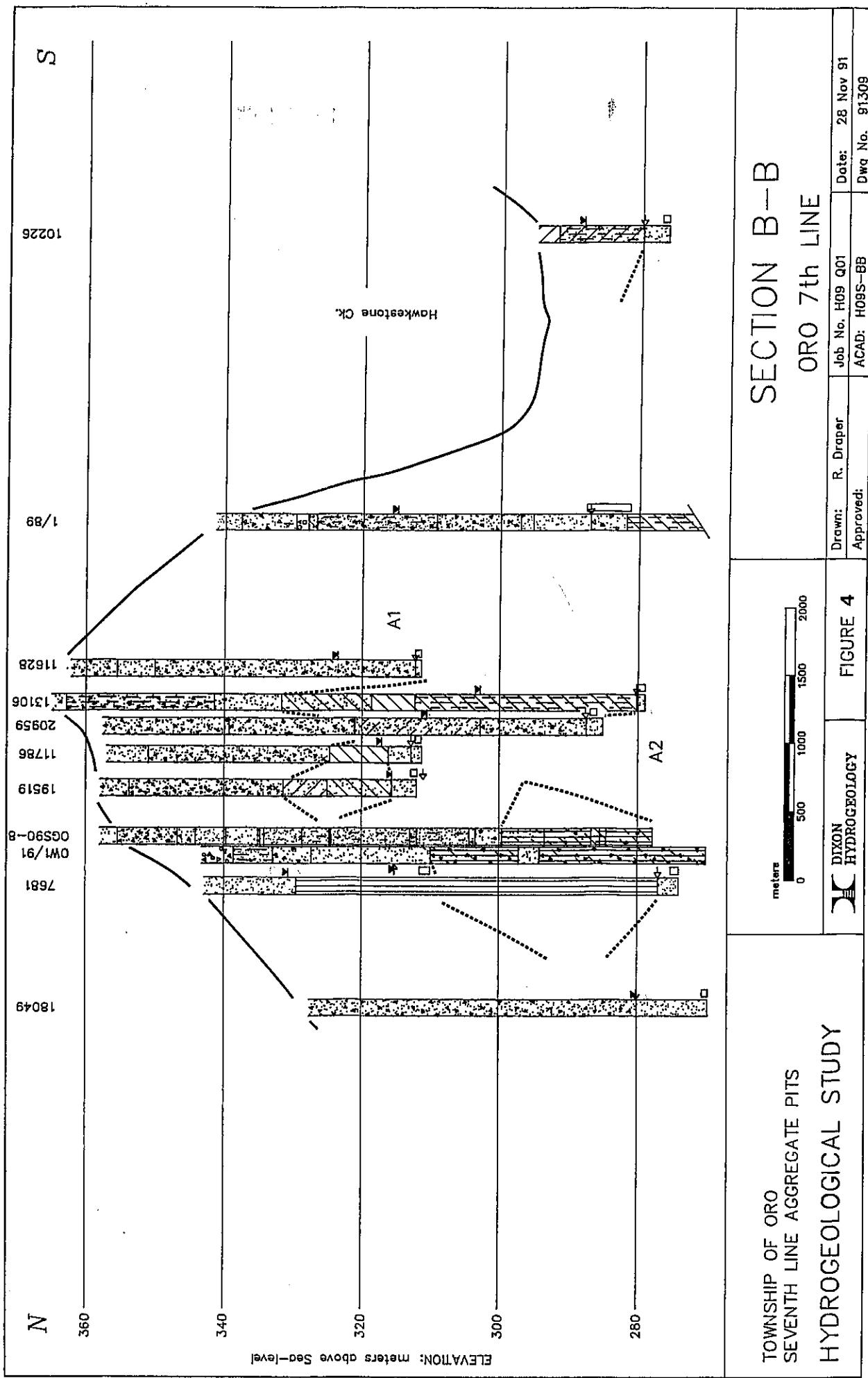
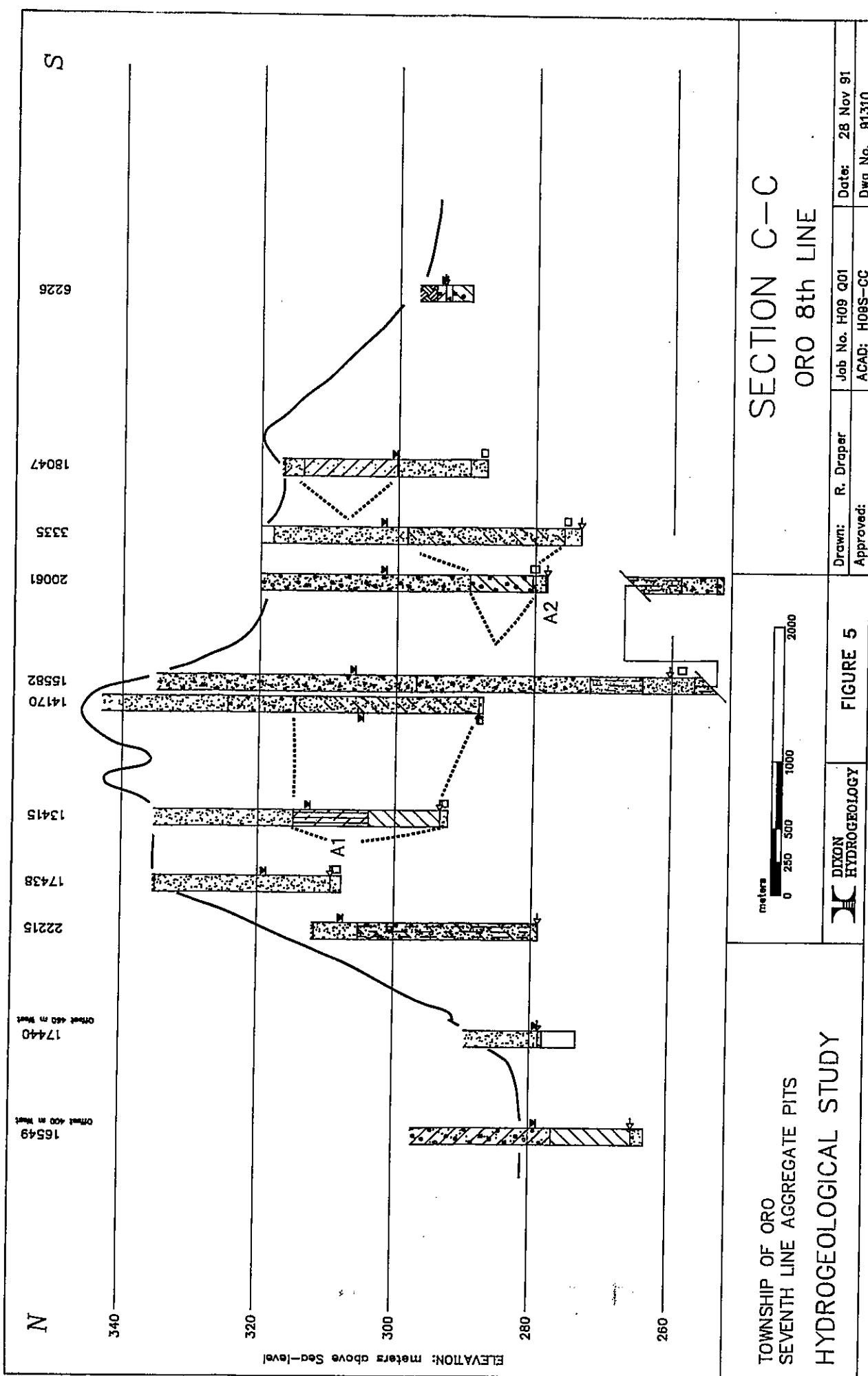


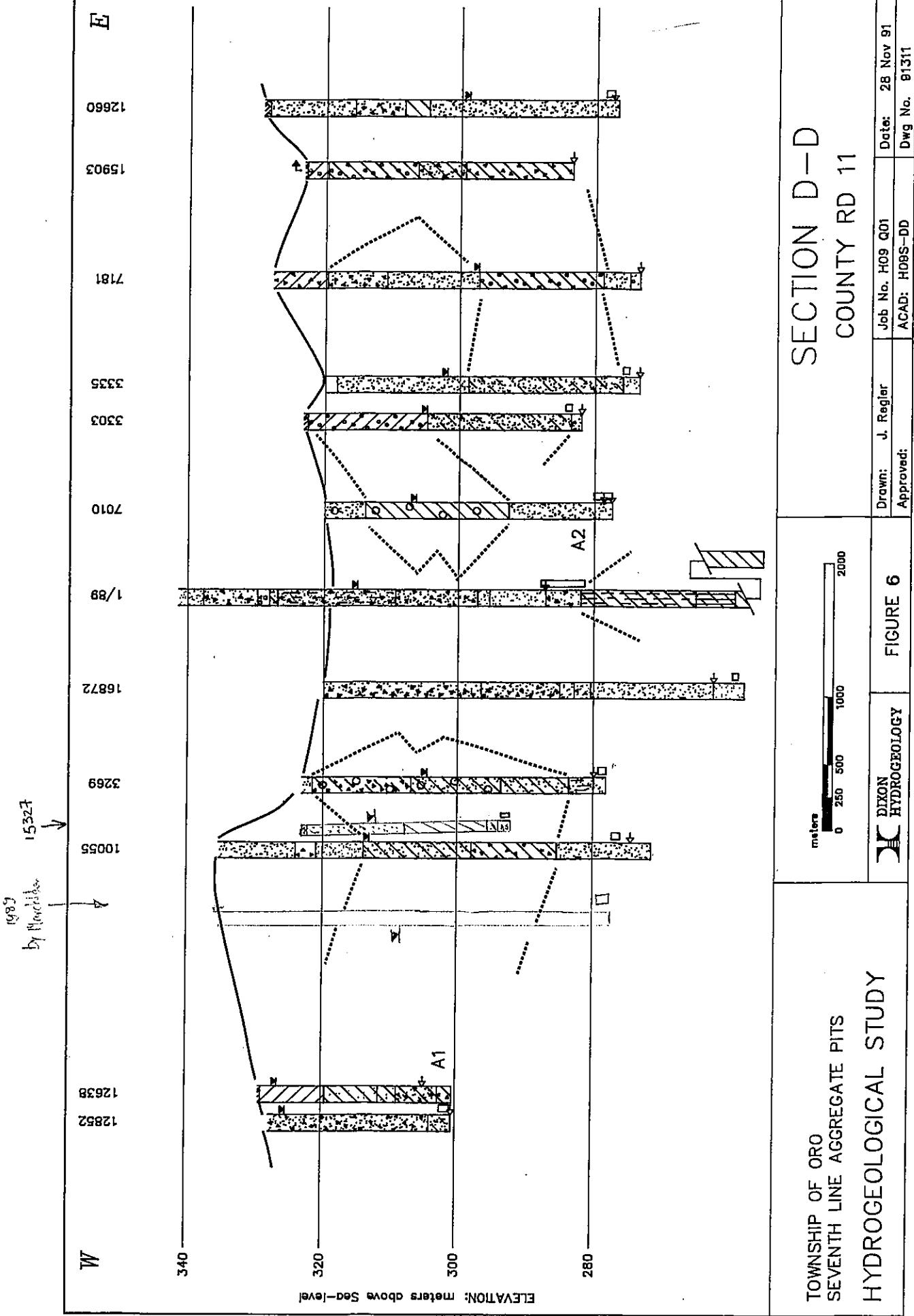
FIGURE 2

Drawn: S. Lee	Job No. H09 Q01	Date: 31 Dec 91
Approved:	ACAD: H09SECT	Dwg No. 91315









 Unoxidized CLAY, Blue, Grey,
White, or Undefined CLAY
 Oxidized CLAY, Brown, Red, Yellow
 SILT
 SAND
 GRAVEL
 STONES, Rounded GRAVEL, PEBBLES

 BOULDER
 SHALE
 SANDSTONE
 DOLOSTONE
 LIMESTONE
 CRYSTALLINE ROCK

- Screen
- Recorded Static Water Level
- ← Water Producing Zone
- Q Test Pumping Rate
- ↗ Flowing Well
- Water Table
- Inferred Geologic Contact

WATER QUALITY
 Fr Fresh
 Mn Mineral
 Sa Salty
 Su Sulphur

HYDROGEOLOGICAL SYMBOLS

NOTE: On all sections, boundaries between soil strata have been determined only at well and test well locations. Between the wells and test wells, boundaries are not proven, but are assumed from geological evidence.



Dixon Hydrogeology Limited

Drawn J P R	Job No.	Date: 19 Jan 89
Approved	ACAD: HYDSYM	Dwg No. 88999



APPENDIX A
WATER WELL RECORDS

CONCESSION	UTM ZONE	WELL EASTING	NORTHING	ELEV FEET	DATE DRILLED
ETC	LOT	NO	NO		

CRUCIAL WATER BULLETIN REPORT
DRAINS/LOG/SCREEN
IN FEET TO WHICH
PARTICLES EXTEND

COMMERCIALITY CODE 57010 (CONTINUED)

DIA OF INS	WATER FEET	LVL FEET	TEST TIME	WATER USE	DEPTHS IN FEET TO WHICH FORMATICS SYSTEM CIRCUIT LOG/SCREEN
------------------	---------------	-------------	--------------	--------------	---

IN FEET TO MILEAGE
CARTES/SCHEMEN

卷之三

CONCESSION ETC. LOT NO. NO. ETC.

CSG	KIND	WATER	STAT	PUMP	TEST	TEST
DIA	OF	FOUND	LVL	LVL	RATE	TIME
INS	INS	WATER	FEET	FEET	GPM	HRS/MIN

DEPTHS IN FEET TO WHICH
ECHOES WERE RECEIVED
CHARTS LOG/SCREEN

MINISTRY 2003 E2010 (CONTINUE)

CONCESSION ETC	LOT	WELL NO	UTM EASTING	ELEV FEET	DATE DRILLED	DIA IN	CSG FOUND	WATER LVL FEET	TEST LVL FEET	PUMP RATE GPM	TEST TIME HR/MN	WATER USE	OWNER/LOG/SCREEN DESCRIPTS	DEPTH IN FEET TO WHICH FORMATION EXTENDS
CON	5 28	516400 17490	750 06/61	3660	5 FR	73	10	58	5 1/00	DO		CLAY GRYL 0045	0018 GREY	

MUNICIPALITY CODE 57010 (CONTINUED...)

CON	6 7	57- 12843	609750 4931875	1170 06/75	4816	6 FR	365	212	30	3/00	DO	BLACK TPSL 0001	BROWN CLAY BLDR 0018 GREY	
CON	6 8	57- 10950	609844 4931437	1175 04/74	3660	5 FR	219	193	5	2/00	DO	CLAY GRYL 0030	CLAY GRYL 0045	
CON	6 8	57- 15192	610050 4931050	1190 05/78	4816	6 FR	200	161	195	4	2/00	DO	PRDG 0045	GREY CGYL CSND 0045
CON	6 9	57- 10955	610106 4930920	1190 04/74	3660	5 FR	255	206	239	3	2/00	DO	CHTD 0073	YLWH SAND WSPS 0076 (S 0673)
CON	6 11	57- 3252	612183 4930144	1025 01/65	4008	30 FR	28	28	2			SAND GRYL	0045	
CON	6 11	57- 10055	611603 4930054	1100 06/73	4816	6 FR	200	73	30	2/00	DO	CHTD 0247	CLAY DME 0506 SAND CHTD 0576	
CON	6 11	57- 15327	611900 4930100	1060 05/78	3660	5 FR	97	39	10	2/00	DO	CHTD 0395	(S 0577 04)	
CON	6 12	57- 11202	612511 4929618	970 03/74	3203	5 FR	57	17	52	2	2/30	DO	SUTHERLAND ED	
CON	6 14	57- 16191	612880 4928800	1000 10/72	4608	30 FR	27	27	30	2	1/00	DO	CHTD 0190	
CON	6 14	57- 3253	612868 4927229	1000 07/79	3660	5 FR	67	31	55	4	2/00	DO	HALLINSON J	
CON	6 16	57- 16191	612880 4928800	1000 07/79	3660	5 FR	195	65	160	6	3/00	ST DO	BROWN TPSL 0002	
CON	6 16	57- 16191	612868 4927229	950 08/64	2514	6 FR	195	65	160	6	3/00	TERRIEN L	BROWN SAND 0056	
CON	6 16	57- 16191	612868 4927229	950 05/70	4713	6 FR	157	45	50	20	2/30	DO	FILL 0005	BROWN CLAY HSND 0055 BLUE HSND
												SPYL 0055	BLUE CLAY FNSD 0194 YLLN HSND	
												0196 (S 0195 03)	HECTOR MARY	

WATER WELL DATA SYSTEM

7 APR 87

GROUND WATER BULLETIN REPORT

CONCESSION ETC	LOT	WELL EASTING FEET	ELEV NG NORTHING FEET	DATE DRILLER	TEST TIME	WATER RATE GPM	TEST FEET	TEST FEET	HR/MIN USE	OWNER/LOG/SCREEN DEPTHS IN FEET TO WHICH FORMATIONS EXTEND

MUNICIPALITY CODE 57010 (CONTINUED...)

CSG KIND DIA OF WATER FLOW	STAT FOUND LVL WATER FEET	PUMP TEST LVL RATE FEET GPM	TEST TIME	WATER DEPTH	DEPTHS IN FEET TO WHICH FORMATIONS EXTEND
CON 6 17 57- 613241 3254 4926099	975 11/64 608	30 FR	25	32	2 ST DO SCHLAIB MSND 0157
CON 6 17 57- 614120 56e7 4926870	975 12/68 2514	6 FR	123	30 118	15 36/00 ST DO BEERS J D PRDG 0028 GRVL HSND CLAY 0045 GREY CLAY
CON 6 19 57- 614676 3255 4925893	965 12/60 4713	6 FR	177	56 100	5 8/00 ST DO CUNNINGTON J PRDG 0065 BLUE CLAY 0071 GREY CLAY 0123 MSND GRVL STNS 0127 (S 0124 03)
CON 6 19 57- 614795 11210 4925806	975 05/74 3203	5 FR	90	40 80	3 1/45 DO STESLING R TPSL 0002 CLAY STNS 0060 MSND 0090 CLAY
CON 6 19 57- 614650 12849 4925958	965 09/75 4816	6 FR	185	34 133	12 2/00 DQ HARRINGTON PAT CLAY SAND GRVL 0092 CLAY 0167 SAND CMTD
CON 6 20 57- 614239 3256 4924492	955 08/62 1614	4 FR	78	34 39	20 2/00 DQ 0178 MSND 0185 (S 0178 03)
CON 6 20 57- 615053 10853 4925374	940 10/73 4715	4 FR	206	17 160	2 4/00 DQ JAMIESON M HPAN BLDR 0078 GRVL 0079 CUNNINGTON R
CON 6 20 57- 614200 14181 4924500	950 05/76 3203	5 FR	114	27 65	10 1/30 DQ SILT 0238 TERMINAL HOLD
CON 6 21 57- 614339 4789 4924448	935 09/66 2514	FR	77	22 49	10 2/00 CO BENT T PRDG 0015 MSND CLAY BLDR 0077 MSND GRVL
CON 6 21 57- 614180 7169 4924370	950 05/70 4608	30 FR	18	10 18	SILT 0060 (S 0077 03) STEPHENSON R W
CON 6 21 57- 614521 10595 4923881	895 08/73 1204	5 FR	40	1 40	DO BROWN CLAY STNS 0018 GREY CLAY MSND 0025 FORSYTHE J
CON 6 21 57- 614200 18089 4924450	950 05/82 3602	6 FR	102	20 85	8 5/00 DQ LOHRIDES M BROWN CLAY 0010 GREY GRVL 0020 BROWN SAND 0030 BROWN SILT SAND 0040 GREY SAND GRVL
CON 6 22 57- 614550 9687 4923700	890 08/72 4713	6 FR	76 FLW	20 2/30 ST DO RATTLE JEAN	0050 (S 0044 03) BROWN CLAY LOOS 0010 BROWN BLDR CLAY DNSE 0014 BLUE CLAY GRYL STNS 0074 BROWN GRYL
CON 6 23 57- 615086 860 07/64 2514					SAND LOOS 0076 - HCARTHUR M

- CONTINUED -

CONCESSION UTM
LOT WELL EASTING ELEV
ETC NO NORTHING FEET DATE DRILLER INS HATER FEET

7 APR 87

CSC KIND WATER STAT PUMP TEST TEST
DIA OF FOUND LVL LVL RATE TIME WATER DEPTHS IN FEET TO WHICH
INS HATER FEET FEET FEET GPM HR/MIN USE FORMATIONS EXTEND

MUNICIPALITY CODE 57010 (CONTINUED...)

CON	6	28	57-	616700	725	10/79	2514	6	FR	32		16	20	1/00	DO	0120 GREY BLDR GRNT LHSN 0134 GREY CLAY SAND DRSE 0234 GREY LHSN DRSE LYRD 0260 SHOCK J
• CON	7	2	57-	609321	1010	12/73	4816	6	FR	212	114	125	50	5/00	HU	BIRN SAND STNS 0002 BIRN SILT SAND BLER 0007 GREY SILT CLAY SAND 0032 GREY SAND ABASCO INC
• CON	7	6	57-	610400	1175	07/76	4816	6	FR	234	202		10	2/00	DO	SANG GRVL STNS 0091 SAND SILT 0122 GREY CLAY SAND 0212 HSND 0265 (S 0247 09) CLAY 0290
• CON	7	6	57-	611000	1075	06/81	3135	5	FR	156	156	180	5	3/00	DO	HAIDLE'S ANTIQUES FSDH 0047 CLAY GRVL 0176 GRVL 0200 HSND 0215 CSND 0235 (S 0226 04) SAND GRVL CHID 0260
• CON	7	7	57-	611820	1125	07/70	5206	5	FR	216	40	200	6	6/00	ST	HOLMES B SAND GRVL DRY 0156 SAND 0190 (S 0187 03)
• CON	7	9	57-	612523	1125	07/55	5510	4	FR	149	130	133	1	/30	DO	BOLTON W BRN HSND 0064 SILT 0216 FSND 0226 (S 0222 04)
• CON	7	9	57-	612688	1125	09/86	3203	6	FR	164	127	153	2	1/00	DO	HCIVIEN W TPSL 0002 HSND STNS 0014 CLAY HSND STNS 0031 CLAY GRVL 0101 CLAY HSND 0142 HSND 0149
• CON	7	9	57-	612650	1190	09/74	3203	6	FR	164	127	153	2	1/00	DO	PORTER MARS BRN SAND GRVL STNS 0022 BRAN SAND LOUS OCAD BEKN SAND GRVL PCKD 0164 ERAN GRVL SAND SBPS 0157 (S 0164 03)
• CON	7	9	57-	612600	1125	05/75	2514	6	FR	278	204	265	6	24/00	DO	ROCHNER GORDEN GRVL SAND 0067 GRVL SAND SILT 0077 YLLW SAND 0109 BRAN CLAY SAND 0152 GREY CLAY 0173 GREY CLAY SILT 0276 SAND LYRD PCKD 0282 (S 0278 04)
• CON	7	10	57-	611620	1190	06/71	4715	4	FR	272	187	190	4	2/00	DO	PARKER D TPSL 0001 HSND 0010 CLAY HSND GRVL 0025 HSND 0145 CLAY HSND GRVL 0272 FSND 0276 (S 0273 03)
• CON	7	11	57-	612158	1060	10/65	4715	4	FR	142	61	62	10	2/30	DO	NICHOLS J HSND 0005 GRYL CLAY BLDR 0053 CLAY HSND BLDR 0057 CLAY HSND 0130 HSND SILT 0142 HSND 0143 (S 0144 04)
• CON	7	11	57-	612200	1050	05/60	4616	6	FR	190			30	3/90	DO	SPAR T CGYL SAND 0077 S4HD GRYL LYRD 0115 HSND 0122 FSND GRYL 0150 HSND 0190 HSND 0265 (S 0139 03)
• CON	7	12	57-	612950	1015	09/69	4603	30	FR	46	44	48	2	1/00	DO	SPAR T E GREY GRYL 0054
• CON	7	13	57-	612950	970	04/77	3203	5	FR	30	30	40	5	1/10	DO	YOUSDAN D ERAN CLAY SAND 0010 BRAN SAND 0060 (S

- CONTINUED -

CONCESSION UTM WELL EASTING ELEV DIA OF WATER TEST PUMP TEST TIME WATER DEPTHS IN FEET TO WHICH
ETC LOT NO NORTHING FEET DATE DRILLER IN' S WATER FEET FEET FEET FEET HR/MIN USE FORMATIONS EXTEND

MUNICIPALITY CODE 57010 (CONTINUED...)

● CON 7 15 57- 614659 1000 11/64 4715 4 FR 46 30 42 5 2/00 ST DO BRIGGS R	0057 03)
● CON 7 15 57- 613461 1000 03/73 4315	TPSL 0001 TPSL HSND 0003 BRWN CLAY MSND GRYL 0046 HSND GRYL 0050 (S 0046 04) BRWN CLAY HSND GRYL 0055
● CON 7 16 57- 614156 990 09/65 4715	YANDERHAZET J SAND 0016 CLAY GRYL 0044 CLAY 0101 CLAY SILT 0471 LHSN 0475
● CON 7 16 57- 614156 990 09/65 4715	Goss A PROG COJO CLAY MSND 0047 GREY CLAY 0077 CLAY SILT 0113 GREY CLAY 0118 FSND 0123 GREY CLAY 0130 MSND SILT 0150 HSND 0155 (S 0151 04)
CON 7 16 57- 615050 985 10/58 4608	ROBINSON D
CON 7 16 57- 614650 990 05/71 3293	BRWN CLAY STNS 0008 HSND GRYL 0015 CLAY STNS SAND 0027 GREY SAND 0046 BRWN CLAY SAND 0048 BRWN CSND 0053 (S 0049 03)
CON 7 16 57- 614450 1000 09/71 3203	THOMSON I BRWN TPSL 0001 BRWN CLAY GRYL STNS 0028 CLAY GRYL SAND 0069 SAND 0072 (S 0069 03)
CON 7 16 57- 615154 985 04/73 4603	ROBINSON E BRWN TPSL 0001 BRWN CLAY GRYL 0009 GREY CLAY STNS SAND 0027 GREY SAND 0046 BRWN CLAY SAND 0048 BRWN CSND 0053 (S 0049 03)
CON 7 18 57- 614700 950 03/78 2514	GRAY J BRWN CLAY 0010 SAND 0022 LECH-ED V
CON 7 18 57- 614700 950 03/78 2514	TPSL DKCL 0001 BRWN SAND CLAY 0015 GREY SAND GRYL SILT 0081 GREY GRYL SILT 0084 BRWN GRYL CSND 0084
CON 7 20 57- 616421 935 04/56 4515	CARIBOO HOTEL PROS 0018 BLDR CLAY 0037 MSND 0052 HPAN 0075 MSND GRYL 0080 CLAY STNS 0125 CLAY
CON 7 20 57- 616421 935 04/56 4515	0225 GRYL 0232 CARIBOO HOTEL
CON 7 20 57- 615079 930 03/63 1312	MARSHAL T BLDR CLAY 0035 MSND CLAY 0655 GRYL 0060
CON 7 20 57- 616200 925734	BRWN CLAY 0010 GREY CLAY HPAN 0035 GREY CLAY HPAN BLDR 0045 GREY CLAY 0056 GRYL MSND 0057 GREY CLAY HPAN 0062
CON 7 20 57- 616460 935 04/56 4515	MC GUIRE R BRWN CLAY STNS 0018 GREY CLAY STNS 0028
CON 7 20 57- 616200 930 07/72 4608	HOTEL CARIBOU GREY SAND STNS 0025
CON 7 21 57- 616232 927 10/55 4526	BRITISH PETROLEUM CLAY MSND BLDR 0035 GRYL 0049 GREY 0045 SHIE 0049 GRYL 0052
CON 7 21 57- 616057 925 10/60 3512	TOMPKINSON V

- CONTINUED -

CONCESSION ETC	LOT	WELL NO	EASTING	ELEV FEET	DATE	DRILLER	HWS	TEST TIME	WATER USE	DEPTHS IN FEET TO WHICH FORMATIONS EXTEND

MUNICIPALITY CODE 57010 (CONTINUED...)

CONCESSION ETC	LOT	WELL NO	EASTING	ELEV FEET	DATE	DRILLER	HWS	TEST TIME	WATER USE	DEPTHS IN FEET TO WHICH FORMATIONS EXTEND
CCN	7 27	57- 15699	618200 4922050	725 06/78	3135	FR	215	FLW	3 7 6/00 D0	SOFT 0223 (S 0222 05 J BRWN SAND CLAY 0226 GREY CLAY SOFT 0226
CON	7 27	57- 15905	616350 4922350	750 09/78	3135	S FR	215		22 5 18/00 D0	KUCK K TPSL 0001 BRN CLAY STNS 0011 GREY CLAY CLAY 0215 GREY GRYL SAND 0115 GREY MARCH C
CCN	7 27	57- 16547	617500 4921600	750 11/79	3203	6 FR	169		6 19 8 1/30 D0	BRWN CLAY STNS 0010 GREY CLAY STNS 0033 GREY CLAY 0190 GREY CLAY SILT SAND 0213 GREY SAND GRYL 0215 GREY GRYL 0215 BOORMAN V
CON	8 1	57- 11183	609915 4936550	920 07/74	3203	6 FR	60		40 52 8 1/00 D0	PROS 0015 BRWN CLAY SAND 0019 GREY CLAY 0123 GREY CLAY GRYL 0142 GREY CLAY 0189 GREY SAND 0197 (S 0194 03) GREY FSD 0199
CON	8 1	57- 17597	609900 4935600	925 03/78	3413	30 FR	25		22 32 5 4/00 D0	NEWELL NORMAN BRWN CLAY 0015 BRWN SAND STNS CLAY 0040 BRWN SAND HBRG 0070 (S 0067 03)
CON	8 2	57- 8506	611400 4936640	950 10/71	3203	5 FR	75		40 63 2 1/30 D0	ORR F BRWN CLAY 0025 CGYL 0030 BRWN CLAY 0048 HARE J BRWN TPSL 0001 CLAY STNS 0031 SAND 0677 (S 0074 03)
CON	8 2	57- 14203	616250 4936630	925 06/76	3203	5 FR	150		47 55 15 2/00 D0	TORPEY LARRY BRWN SAND 0017 BRWN CLAY 0020 GREY CLAY 0121 BRWN CLAY SAND 0126 GREY CLAY 0150 GREY CSND 0156 (S 0153 03)
CON	8 3	57- 16549	611550 4935630	975 11/79	3203	6 FR	105		60 78 15 1/25 D0	MCNIVEN E BRWN CLAY STNS SAND 0067 GREY CLAY 0105 BRWN SAND GRYL 0111
CON	8 4	57- 17440	611650 4935250	950 05/80	3135	5 FR	35		35 42 5 1/30 D0	ATHWOOD K BRWN SAND DRY 0035 SAND 0037 UNKN 0053 (S SAND 04)
CON	8 7	57- 3302	612485 4933245	1160 04/57	1510	2 FR	135		125 5 ST DO GRAVES W	MIDDLETON ROBERT PRDR 0090 CLAY STNS 0135 HSND 0145 (S 0141 04)
CON	8 7	57- 13415	613090 4833900	1100 01/75	3203	5 FR	137		75 120 7 1/30 D0	WALSH PETER BRWN SAND DRY 0067 BRWN SILT CLAY 0103 GREY CLAY 0137 BRWN SAND 0141 (S 0128 03) BRWN SAND CLAY 0141
CON	8 8	57- 11766	612450 4932625	1170 12/74	3660	5 FR	145		131 140 4 2/00 ST EO	MIDDLETON ROBERT RED SAND 0020 BRWN SAND GRYL LYRD 0106 BRWN CLAY 0134 BRWN SAND CHTD 0145 YLLH SAND 0150 (S 0147 03)
CON	8 9	57- 14170	613640 4933100	1060 03/76	3203	5 FR	180		125 3 2/00 D0	CLIFFE DARRELL BRWN SAND DRY 0060 EPBN SAND GRYL 0092 SAND 0182 (S 0179 03) BRWN ASIMI A

- CONTINUED -

GEOGRAPHICAL SETTING 800

MUNICIPALITY CODE 5700 | CERTIFIED

115522 4933100

SECRET CLR 0060

WATER WELL DATA SYSTEM 7 APR 87 PAGE 650

CONCESSION WELL EASTING ELEV
ETC LOT NO NORTHING FEET DATE DRILLER INS WATER FEET

MUNICIPALITY CODE 57010 (CONTINUED...)

	CGS KIND DIA OF FOUND LYL FEET	WATER STAT LVL FEET	PUMP TEST RATE TIME FEET	TEST TIME	WATER HR/MN USE	OWNER/LOG/SCREEN DEPTHS IN FEET TO WHICH FORMATION EXTEND
CON 8 27 57- 15700 4921700	FR 155	FR 98	4 12	10 21/00 DO	CLAY GRVL LYRD 0034 CLAY STNS HARD 0059 CLAY 0110 CLAY GRVL 0134 CSND 0155 (S 0144 03) CLAY SAND 0185	
CON 8 27 57- 15700 4922600	FR 1452	FR 212	15 175	5 48/00 DO	CANCILLA F BLCK TPSL 0001 GREY SAND STNS CHTD 0067 GREY GRVL CGRD 0062 GREY SAND GRVL CMTD 0104 BRWN SAND STNS 0105	
CON 8 27 57- 16192 4923100	FR 3660	SU 242			CRAWFORD W BLCK TPSL 0002 BRWN CLAY 0015 GREY CLAY 0093 BRWN SAND 0096 GREY CLAY GRVL BLDR 0206 GREY FSND CLAY 0208 YLLW LMSN FCRD 0212 GREY LMSN ROCK 0242	
CON 8 27 57- 16712 4923200	FR 2514	6			WINCH E TPSL DKCL 0001 BRWN SAND SILT BLDR 0027 GREY SILT SAND GRVL 0149	
CON 8 27 57- 16715 4923200	FR 2514	6	29	12 25	KINCH E TPSL DKCL 0001 GREY SAND CLAY SILT 0022 GREY SILT SAND BLDR 0029 GREY GRVL SAND SILT 0032	
CON 8 27 57- 17439 4922750	FR 3135	5	115	18 100	FEATHERSTONE J PRDG 0029 GREY CLAY STNS 0115 GREY SAND 0118 (S 0115 03)	
CON 8 27 57- 17464 4922750	FR 3660	5	203	FLW 150	HELMKAY W BLCK TPSL 0001 GREY CLAY BLDR 0063 BLUE CLAY GRVL LYRD 0195 BRWN FSND 0203 BRWN MSND MCVL 0209 (S 0204 03) GREY GRVL	
CON 8 27 57- 17542 4923250	FR 3660	5	227	6	YANIN L CHTD 0212 GREY LMSN ROCK 0212	
CON 8 27 57- 17992 4923250	FR 3203	6	212	10	BRWN CLAY 0015 GREY CLAY GRVL BLDR 0227 GREY SAND GRVL 0230 (S 0227 02) GREY LMSN ROCK 0230	
CON 8 27 57- 18046 4922700	FR 3203	6	221	3	DOKARD W BRWN SAND 0009 BRWN CLAY STNS 0048 BRWN SAND CLAY STNS 0062 GREY CLAY STNS GRVL 0078 GREY CLAY STNS PCKD 0148 GREY CLAY SILT LYRD 0205 GREY CLAY GRVL FC KD 0210 GREY LMSN 0211 GREY CGVL 0212	
CON 8 27 57- 18057 4922600	FR 3203	6	148	10 70	DUNN D TPSL 0002 BRWN CLAY SAND 0006 GREY CLAY HARD 0042 GREY CLAY SAND SLTY 0182 GREY CLAY 0210 GREY CLAY SAND 0221 GREY GRVL CLAY 0224 GREY GRVL 0226 GREY CLAY 0226	
CON 9 1 57- 611500	FR 4816	6	170	44	GREER E BLCK TPSL 0002 BRWN CLAY SHDY 0018 GREY CLAY STNS 0029 GREY CLAY SAND 0101 GREY CLAY STNS 0119 GREY CLAY 0148 GREY GRVL CLAY 0159 GREY GRVL 0166 GREY CLAY GRVL STNS 0166	

CONCESSION ETC	LOT	UTM NO	HELL EASTING NORTHING FEET	ELEV FEET	DATE DRILLER	TEST WATER	STAT OF	PUMP FR	TEST LYL	TEST RATE TIME	WATER FEET	USE	DEPTHS IN FEET TO WHICH FORMATION EXTEND	CHIERS/LOGS/SCREEN

MUNICIPALITY CODE 57910 (CONTINUED...)

CON	9	12	57-	616191	1015 12/64	4603	30	FR	28	28	1	ST DO	KOZAK P	BLCK FILL 0008 BRYN CLAY STNS 0012 GRYL	
CON	9	13	57-	615420	975 03/63	4608	30	FR	12	13		SI DO	STRACHAN GEORGE	BRYN CLAY PKD 0002 YLW CLAY BLR LOGS 0004 BRYN CLAY BLR PKD 0009 GREY CLAY BLR PKD 0017 GREY CLAY GRYL STKY 0066 Gey CLAY STKY 0072 GREY CLAY GRYL BLR 0121 GREY CLAY GRYL HARD 0125 GREY GRYL SAND HARR 0128	
CON	9	17	57-	617500	975 04/30	2104	6	FR	413	25	60	30 15/00 IN	SHERIDAN MURSERIES	- CONTINUED -	
CON	9	10	57-	614415	1050 05/66	4713	6	FR	153	60	90	15	2/00 ST DO	WILSON J	
CON	9	10	57-	615050	1070 09/77	2517	6	FR	150	168	100	10	2/00 ST DO	HULLIS STEEVES	
CON	9	11	57-	615163	1075 08/69	4713	6	FR	178	100	120	10	2/00 ST DO	BRYN CLAY GRYL 0026 BRYN GRYL HSND 0055 0064 04 J BRYN CLAY 0147 GREY GRYL CMTD 0164 GREY SAND 0168 GREY CLAY GRYL 0175	
CON	9	12	57-	616191	1015 12/64	4603	30	FR	28	28	1	ST DO	MEKAY B	BRYN CLAY GRYL 0028 BRYN CLAY STNS 0160 BRYN HSND 0173 BRYN CSND STNS 0173	
CON	9	12	57-	614750	1040 09/81	3135	5		55	78	12	1/30 ST DO	KOZAK P	TPLS 0001 SAND CLAY 0010 CLAY SNDY 0055 SAND HARD PKD 0090 SAND 0098 (S 0075 03)	
CON	9	13	57-	615285	970 10/74	2514	6	FR	125	12	20	25	1/30 DO	LUCAS G	BRYN CLAY PKD 0002 YLW CLAY BLR LOGS 0004 BRYN CLAY BLR PKD 0009 GREY CLAY BLR PKD 0017 GREY CLAY GRYL STKY 0066 Gey CLAY STKY 0072 GREY CLAY GRYL BLR 0121 GREY CLAY GRYL HARD 0125 GREY GRYL
CON	9	17	57-	61665	49328575										
CON	9	17	57-	617500	975 04/30	2104	6	FR	413	25	60	30 15/00 IN			

CONCESSION UTM
LOT WELL EASTING ELEV
ETC NO NORTHING FEET DATE DRILLER INS WATER FEET FEET

OWNER/LOG/SCREEN
DEPTHS IN FEET TO WHICH
FORMATION EXTEND

MUNICIPALITY CODE 57010 (CONTINUED...)

16711 4929400

CONCESSION	LOT	WELL	EASTING	ELEV	DATE	DRILLER	INS	WATER	FEET	TEST	TEST	WATER	DEPTH
									FEET	HR/MIN	USE		IN FEET TO WHICH
CON	9 20	57-	617926	930 07/67	4608	30	FR	46	39	2	D9	BWN	TPSL SAND LOOS 0001 BWN CLAY BLDR
CON	9 20	57-	33337 4926907	925 02/73	4713	6	FR	72	15	30	40	BWN	SAND SILT DNE 0040 BWN SAND GRVL DNE
CON	9 20	10264	618700	925 02/73	4713	6	FR	72	15	30	40	BWN	SILT DNE 0227 BLUE CLAY DNE 0418 BWN GRVL SAND
CON	9 20	57-	617600	950 02/76	3660	5	FR	44	17	36	4	BWN	LOOS 0419 GREY LHSN POES 0420
CON	9 21	12962	4927600									MCPHERSON H	MCPHERSON H
CON	9 21	57-	618954	900 12/61	4713	6	FR	170	6	100	10	HORNACO S	BWN CLAY STNS 0026 GREY CLAY STNS 0064
CON	9 21	33338	4926889									BWN	TPSL 0002 CLAY STNS 0027 BLUE CLAY 0168
CON	9 23	57-	617854	910 01/62	3715	30	FR	35	5	35	2	JOHNSTON R	CLAY GRVL 0170
CON	9 23	33340	4925413									BWN	CLAY 0035 GRVL 0042
CON	9 23	57-	619834	810 09/66	4608	30	FR	205	70	90	20	JOHNSTONE N	PRDG 0069 CLAY STNS HSND 0200 (S 0198
CON	9 23	33341	4925807									BWN	04) GRVL HSND 0205
CON	9 23	57-	619850	820 07/70	4608	30	FR	22	22	2	D9	PHLAJA P	
CON	9 23	7343	4925880									BWN	BEAR CLAY 0020 HSND 0037
CON	9 23	57-	619700	840 07/81	2514	6	FR	15	15	15	3/00 D0	HOORE D	
CON		17379	4925950									BULK TPSL 0001 BWN CLAY STNS 0005 GREY GRVL	
CON	9 24	57-	619719	835 06/60	3414	4	FR	113	23	61	10	PHLAGA T	SAND GRVL CLAY 0040 YLLW HSND 0043 GREY
CON	9 24	33342	4925954									CLAY BLDR 0151 GREY GRVL SAND 0154 (S 0151 03)	
CON	9 24	57-	618662	660 12/63	4102	30	FR	60	60	6	ST DO	PHLAGA T	
CON	9 24	3343	4925489									PRES 0040 EBWN CLAY 0108 HSND 0113 (S 0109 04)	
CON	9 24	57-	620040	790 09/72	2514	6	FR	124	FLW	20	32	BAZZO V	PRDG 0008 GREY HPN BLDR 0039 GREY GRVL
CON	9 24	9379	4925630									CLAY SAND 0071 GREY CLAY 0115 HSND 0129	
CON	9 24	14539	4924200									WALSH DOROTHY DAVID	
CON	9 25	57-	619300	755 08/77	4241	6	FR	29	1	9	20	BLACK FILL 0002 BWN SAND SILT GRVL 0027	
CON	9 25	7006	4924450									BWN SAND CED 0029	
CON	9 25	57-	619750	740 07/70	4713	6	FR	95	FLW	60	20	FOLLEY E	
CON	9 25	7625	4924450									GREY CLAY BLDR 0090 BLUE HSND GRVL 0025	
CON	9 25	57-	620420	725 04/71	4608	30	FR	79	FLW	60	20	GERASUNCHUK U	
CON	9 25	7930	4924800									BLUE CLAY BLD GRVL 0076 BLUE GRVL HSND 0079	
												HORNACO S	
												BRN CLAY STNS 0010 GREY HPN 0055	

CONCESSION UTH WELL EASTING ELEV DIA OF FOUND LVL RATE TEST TEST
ETC LOT NO NORTHING FEET DATE DRILLER INS WATER FEET FEET FEET GPM H2/N USE DEPTHS IN FEET TO WHICH
FORMATIONS EXTEND

MUNICIPALITY CODE 57010 (CONTINUED...)

CON	9	27	57-	619351	720	02/74	1204	5	FR	190	5	170	4	3/00	DJ	CLAY 0194 GREY GRYL CLAY BLDR 0201 GREY GRVL CHTD WB2G 0201
				10735 4923750											DATES R J	
CON	9	27	57-	619300	725	11/74	3203	5	FR	215	16	90	8	10/00	DJ	BROWN CLAY SAND BLDR 0042 BROWN GRYL CLAY SAND 0050 GREY CLAY GRYL BLDR 0060 GREY CLAY GRYL 0190 BROWN SAND GRYL 0195 (S 0191 03)
				12053 4923250											BROWN R	
CON	9	27	57-	619700	745	03/76	3203	5	FR	189	20	80	5	2/30	DJ	BROWN TPSL 0001 BROWN GRYL SAND 0026 BROWN CLAY 0035 GREY CLAY STNS 0130 GREY GRYL SAND CLAY 0178 GREY CLAY 0214 LMSN 0215 STEWARD BILL
				14200 4923610											BLACK TPSL 0001 BROWN CLAY STNS 0009 GREY CLAY STNS 0120 GREY CLAY 0189 GREY GRYL SAND CLAY 0192 (S 0191 03) GREY GRYL SAND 0194 CLAY 0194 EGG ROBERT	
CON	10	1	57-	612620	955	12/69	4713	6	FR	70	42	65	8	2/00	ST DJ	BROWN CLAY STNS 0027 BLUE CLAY STNS 0068 BLUE GRYL 0070
				7195 4938280											SAILA	
CON	10	5	57-	614730	970	11/71	3203	6	FR	24	24	32	6	1/00	DJ	BROWN TPSL 0002 BROWN SAND GRYL 0024 BROWN SAND GRYL 0042 (S 0039 03)
● CON	10	7	57-	614875	1045	05/74	3203	6	FR	156	110	174	3	3/00	DJ	DURCAN B I
				11181 4935300											BLACK TPSL 0002 BROWN SAND LOOS DRY 0018 BROWN GRYL STNS HARD 0065 BROWN SAND CLAY 0130 GREY CLAY 0160 BROWN GRYL CLAY 0165 (S 0183 06) BROWN GRYL SAND KBRG 0138 BARETT D	
CON	10	8	57-	615283	1200	07/73	3203	6	FR	275	140	170	8	4/00	DJ	PREG 0025 BROWN SAND 0052 BROWN CLAY SAND 0095 BROWN GRYL STNS 0140 BROWN SAND CLAY 0180 GREY CLAY 0186 BROWN SAND CLAY 0188 GREY CLAY 0260 GREY CLAY SILT 0275 GREY GRVL 0277
				10342 4934821											GILLESPIE K	
CON	10	9	57-	615532	1155	03/58	4713	6	FR	140	80	120	20	3/00	DJ	TPSL 0002 BROWN CLAY 0020 HSND 0080 QSHD 0130 BLUE CLAY 0135 GRYL MSND 0140
				3334 4934048											HOLDSORTH R	
CON	10	9	57-	615950	1100	11/79	3135	5	FR	217	193	208	9	1/30	DJ	BROWN SAND GRYL STNS 0032 BROWN SAND DRY 0130 BROWN SAND STNS DRY 0138 BROWN SAND DRY 0172 GREY CLAY 0217 BROWN SAND 0225 (S 0222 03) CLAY 0225
				16633 4934100											FEEHAN D	
CON	10	10	57-	616150	1100	08/79	3135	5	FR	151	133	144	8	1/30	DJ	SAND DRY PKXO 0101 CLAY STNS 0108 SAND GRYL HARD 0151 SAND GRYL WERG 0155 (S 0152 03)
● CON	10	11	57-	616285	1060	10/75	4715	6	FR	170	105	108	7	1/00	DJ	TENELTCH ROBERT
				12660 4933215											BROWN TPSL 0003 BROWN SAND 0044 GREY GRYL SAND 0068 GREY CLAY 0030 BROWN SAND 0172 - CONTINUED -	

WATER WELL DATA SYSTEM

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GROUND WATER BULLETIN REPORT

CONCESSION ETC	LOT	WELL NO	EASTING FEET	ELEV FEET	DATE DRILLED	DIA OF WATER FEET	TEST LVL FEET	TEST LVL FEET	WATER USE	OWNER/LOG/SCREEN LOCATIONS EXTEND			
CON	10 11	57-	616750	1075	08/77	2514	6	FR	210	148	190	15 3/30 DO	
CON	10 11	57-	615950	1060	07/78	3135	5	FR	150	FLW	11	6 2/30 DO	
CON	10 12	57-	616900	1025	03/76	3203	5	FR	97	50	90	5 2/00 DO	
CON	10 14	57-	616857	965	06/59	4713		FR	57	6	30	50 2/00 DO	
CON	10 15	3355	4931145	617960	925	08/68	4608	30	FR	18	17	DO	
CON	10 15	57-	617960	931180	617970	955 11/71	3203	5	FR	45	25	34 1/00 DO	
CON	10 16	57-	617920	970	02/73	3203	5	FR	44	30	35	10 1/00 DO	
CON	10 20	57-	619621	890	02/54	1614	6	FR	89	14	25	4 4/00 CO DO	
CON	10 20	57-	619550	930	12/74	4713	6	FR	214	95	150	20 2/30 DO	
CON	10 20	57-	619850	885	04/76	3203	6	FR	178		165	5 48/60 CO DO	
CON	10 20	57-	618850	925	12/78	2653	6	FR	80	35		PRDPET DAVID	
CON	10 21	57-	619203	885	01/62	3715	30	FR	35	5	35	7 2/00 DO	
CON	10 21	57-	619870	875	11/65	2514		FR	237	45	125	14 3/30 ST DO	
CON	10 21	57-	616193	885	01/62	3715	30	FR	40	30	38	PRDG 0035 BRHN CLAY MSND 0044 BLUE CLAY	
CON	10 21	3360	4927041	619630	885	12/77	4241	6	FR	230	78	105	HNSND BLDR 0125 BLUE CLAY 0160 BLUE CLAY
CON	10 21	57-	619630									CLAY 0236 GREY MSND 0242 (S 0238 03)	

MUNICIPALITY CODE 57010 (CONTINUED...)

CONCESSION ETC	UTM LOT	WELL NO	EASTING FEET	ELEV FEET	DATE DRILLED	DIA OF WATER FEET	TEST LVL FEET	TEST LVL FEET	WATER USE	OWNER/LOG/SCREEN LOCATIONS EXTEND			
CON	10 11	57-	616750	1075	08/77	2514	6	FR	210	148	190	15 3/30 DO	
CON	10 11	15703	4932850									(S 0166 04)	
CON	10 12	57-	616900	1025	03/76	3203	5	FR	97	50	90	5 2/00 DO	
CON	10 14	57-	616857	965	06/59	4713		FR	57	6	30	50 2/00 DO	
CON	10 15	3355	4931145	617960	925	08/68	4608	30	FR	18	17	DO	
CON	10 15	57-	617970	955	11/71	3203	5	FR	45	25	34	MSND 0004 BRHN CLAY STNS 0027	
CON	10 16	57-	617920	970	02/73	3203	5	FR	44	30	35	10 1/00 DO	
CON	10 20	57-	619621	890	02/54	1614	6	FR	89	14	25	4 4/00 CO DO BEARDWOOD 6	
CON	10 20	57-	619550	930	12/74	4713	6	FR	214	95	150	20 2/30 DO	
CON	10 20	57-	619850	885	04/76	3203	6	FR	178		165	5 48/60 CO DO	
CON	10 20	57-	618850	925	12/78	2653	6	FR	80	35		PRDG 0015 BRHN CLAY STNS GRYL 0019 GREY	
CON	10 21	57-	619203	885	01/62	3715	30	FR	35	5	35	SAND GRYL 0062 GREY CLAY HARD 0167 GREY	
CON	10 21	57-	619870	875	11/65	2514		FR	237	45	125	SAND GRYL 0176 SAND 0164 (S 0179 05)	
CON	10 21	14175	4928300									HPAN BLDR 0035 SAND 0070 SAND GRYL 0060	
CON	10 20	57-	619850	885	04/76	3203	6	FR	214	95	150	14 3/30 ST DO	
CON	10 20	57-	618850	925	12/78	2653	6	FR	80	35		CLAY 0035 GRYL 0042	
CON	10 21	57-	619203	885	01/62	3715	30	FR	35	5	35	JOHNSTON R	
CON	10 21	57-	619870	875	11/65	2514		FR	237	45	125	CLAY 0035 GRYL 0042	
CON	10 21	3360	4927041	619630	885	12/77	4241	6	FR	40	30	38	PRDG 0035 BRHN CLAY MSND 0044 BLUE CLAY
CON	10 21	57-	619630									HNSND BLDR 0125 BLUE CLAY 0160 BLUE CLAY	
CON	10 21	57-	619630									CLAY 0236 GREY MSND 0242 (S 0238 03)	

- CONTINUED -

WATER WELL DATA SYSTEM

GROUND WATER BULLETIN REPORT

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CONCESSION	WELL	UTM	EASTING	ELEV	DIA OF	WATER	STAT	PUMP	TEST	TEST	DEPTHS IN FEET TO WHICH		
ETC	LOT	NO	NORTHINGS	FEET	DATE DRILLER	INS	WATER	FEET	LYL	RATE	FORMATION EXTEND		
CON	10	25	57-	621300	725	12/77	3135	5	FR	118	FLW	0216	
CON	10	25	57-	15000 4925400	725	12/77	3135	5	FR	123	7	00	DO
CON	10	25	57-	15908 4925300	750	10/73	3135	5	FR	170			
CON	10	25	57-	16632 4925700	750	06/79	3135	5	FR	100	19	30	DO
CON	10	25	57-	16710 4925200	750	04/30	1457	5	FR	136	FLW	3	4/00 DO
CON	10	25	57-	16716 4925400	750	10/79	2514	6	FR	112	14	109	10
CON	10	25	57-	17880 4925650	725	05/81	2514	6	FR	114	7	98	1/00 DO
CON	10	26	57-	14112 4925150	735	04/77	2104	6	FR	134		110	20
CON	11	1	57-	3370 4938779	960	03/66	4606	30	FR	20	16	2	PS
CON	11	3	57-	8536 4937380	975	12/71	4606	30	FR	25	25	30	2/00 DO
CON	11	4	57-	18203 4937550	875	09/32	4816	11		FLW		10	2/00 ST DO
CON	11	5	57-	13565 4936200	950	07/76	3660	6	FR	35	15	23	
CON	11	7	57-	14232 4935950	1000	05/77	3660	5	FR	152	127	5	2/00 DO

MUNICIPALITY CODE 57010 (CONTINUED....)

CONCESSION	WELL	UTM	EASTING	ELEV	DIA OF	WATER	STAT	PUMP	TEST	TEST	DEPTHS IN FEET TO WHICH			
ETC	LOT	NO	NORTHINGS	FEET	DATE DRILLER	INS	WATER	FEET	LYL	RATE	FORMATION EXTEND			
CON	14391	4925500												
CON	10	25	57-	14493 4925700	725	12/77	3135	5	FR	118	FLW			
CON	10	25	57-	15000 4925400	725	12/77	3135	5	FR	123	7	100	4	3/00 DO
CON	10	25	57-	15908 4925300	750	10/73	3135	5	FR	170		144	5	48/00 DO
CON	10	25	57-	16632 4925700	750	06/79	3135	5	FR	100	19	30	10	1/00 DO
CON	10	25	57-	16710 4925200	750	04/30	1457	5	FR	136	FLW	3	4/00 DO	
CON	10	25	57-	16716 4925400	750	10/79	2514	6	FR	112	14	109	10	1/30 DO
CON	10	25	57-	17880 4925650	725	05/81	2514	6	FR	114	7	98	20	1/00 DO
CON	10	26	57-	14112 4925150	735	04/77	2104	6	FR	134		110	20	4/30 DO
CON	11	1	57-	3370 4938779	960	03/66	4606	30	FR	20	16	2		
CON	11	3	57-	8536 4937380	975	12/71	4606	30	FR	25	25	30		
CON	11	4	57-	18203 4937550	875	09/32	4816	11		FLW		10	2/00 ST DO	
CON	11	5	57-	13565 4936200	950	07/76	3660	6	FR	35	15	23		
CON	11	7	57-	14232 4935950	1000	05/77	3660	5	FR	152	127	5	2/00 DO	
CON	14	1	57-	3370 4938779	960	03/66	4606	30	FR	20	16	2		
CON	14	3	57-	8536 4937380	975	12/71	4606	30	FR	25	25	30		
CON	14	4	57-	18203 4937550	875	09/32	4816	11		FLW		10	2/00 ST DO	
CON	14	5	57-	13565 4936200	950	07/76	3660	6	FR	35	15	23		
CON	14	7	57-	14232 4935950	1000	05/77	3660	5	FR	152	127	5	2/00 DO	
CON	15	1	57-	3370 4938779	960	03/66	4606	30	FR	20	16	2		
CON	15	3	57-	8536 4937380	975	12/71	4606	30	FR	25	25	30		
CON	15	4	57-	18203 4937550	875	09/32	4816	11		FLW		10	2/00 ST DO	
CON	15	5	57-	13565 4936200	950	07/76	3660	6	FR	35	15	23		
CON	15	7	57-	14232 4935950	1000	05/77	3660	5	FR	152	127	5	2/00 DO	
CON	16	1	57-	3370 4938779	960	03/66	4606	30	FR	20	16	2		
CON	16	3	57-	8536 4937380	975	12/71	4606	30	FR	25	25	30		
CON	16	4	57-	18203 4937550	875	09/32	4816	11		FLW		10	2/00 ST DO	
CON	16	5	57-	13565 4936200	950	07/76	3660	6	FR	35	15	23		
CON	16	7	57-	14232 4935950	1000	05/77	3660	5	FR	152	127	5	2/00 DO	
CON	17	1	57-	3370 4938779	960	03/66	4606	30	FR	20	16	2		
CON	17	3	57-	8536 4937380	975	12/71	4606	30	FR	25	25	30		
CON	17	4	57-	18203 4937550	875	09/32	4816	11		FLW		10	2/00 ST DO	
CON	17	5	57-	13565 4936200	950	07/76	3660	6	FR	35	15	23		
CON	17	7	57-	14232 4935950	1000	05/77	3660	5	FR	152	127	5	2/00 DO	
CON	18	1	57-	3370 4938779	960	03/66	4606	30	FR	20	16	2		
CON	18	3	57-	8536 4937380	975	12/71	4606	30	FR	25	25	30		
CON	18	4	57-	18203 4937550	875	09/32	4816	11		FLW		10	2/00 ST DO	
CON	18	5	57-	13565 4936200	950	07/76	3660	6	FR	35	15	23		
CON	18	7	57-	14232 4935950	1000	05/77	3660	5	FR	152	127	5	2/00 DO	
CON	19	1	57-	3370 4938779	960	03/66	4606	30	FR	20	16	2		
CON	19	3	57-	8536 4937380	975	12/71	4606	30	FR	25	25	30		
CON	19	4	57-	18203 4937550	875	09/32	4816	11		FLW		10	2/00 ST DO	
CON	19	5	57-	13565 4936200	950	07/76	3660	6	FR	35	15	23		
CON	19	7	57-	14232 4935950	1000	05/77	3660	5	FR	152	127	5	2/00 DO	
CON	20	1	57-	3370 4938779	960	03/66	4606	30	FR	20	16	2		
CON	20	3	57-	8536 4937380	975	12/71	4606	30	FR	25	25	30		
CON	20	4	57-	18203 4937550	875	09/32	4816	11		FLW		10	2/00 ST DO	
CON	20	5	57-	13565 4936200	950	07/76	3660	6	FR	35	15	23		
CON	20	7	57-	14232 4935950	1000	05/77	3660	5	FR	152	127	5	2/00 DO	
CON	21	1	57-	3370 4938779	960	03/66	4606	30	FR	20	16	2		
CON	21	3	57-	8536 4937380	975	12/71	4606	30	FR	25	25	30		
CON	21	4	57-	18203 4937550	875	09/32	4816	11		FLW		10	2/00 ST DO	
CON	21	5	57-	13565 4936200	950	07/76	3660	6	FR	35	15	23		
CON	21	7	57-	14232 4935950	1000	05/77	3660	5	FR	152	127	5	2/00 DO	
CON	22	1	57-	3370 4938779	960	03/66	4606	30	FR	20	16	2		
CON	22	3	57-	8536 4937380	975	12/71	4606	30	FR	25	25	30		
CON	22	4	57-	18203 4937550	875	09/32	4816	11		FLW		10	2/00 ST DO	
CON	22	5	57-	13565 4936200	950	07/76	3660	6	FR	35	15	23		
CON	22	7	57-	14232 4935950	1000	05/77	3660	5	FR	152	127	5	2/00 DO	
CON	23	1	57-	3370 4938779	960	03/66	4606	30	FR	20	16	2		
CON	23	3	57-	8536 4937380	975	12/71	4606	30	FR	25	25	30		
CON	23	4	57-	18203 4937550	875	09/32	4816	11		FLW		10	2/00 ST DO	
CON	23	5	57-	13565 4936200	950	07/76	3660	6	FR	35	15	23		
CON	23	7	57-	14232 4935950	1000	05/77	3660	5	FR	152	127	5	2/00 DO	
CON	24	1	57-	3370 4938779	960	03/66	4606	30	FR	20	16	2		
CON	24	3	57-	8536 4937380	975	12/71	4606	30	FR	25	25	30		
CON	24	4	57-	18203 4937550	875	09/32	4816	11		FLW		10	2/00 ST DO	
CON	24	5	57-	13565 4936200	950	07/76	3660	6	FR	35	15	23		
CON	24	7	57-	14232 4935950	1000	05/77	3660	5	FR	152	127	5	2/00 DO	
CON	25	1	57-	3370 4938779	960	03/66	4606	30	FR	20	16	2		
CON	25	3	57-	8536 4937380	975	12/71	4606	30	FR	25	25	30		
CON	25	4	57-	18203 4937550	875	09/32	4816	11		FLW		10	2/00 ST DO	
CON	25	5	57-	13565 4936200	950	07/76	3660	6	FR	35	15	23		
CON	25	7	57-	14232 4935950	1000	05/77	3660	5	FR	152	127	5	2/00 DO	
CON	26	1	57-	3370 4938779	960	03/66	4606	30	FR	20	16	2		
CON	26	3	57-	8536 4937380	975	12/71	4606	30	FR	25	25	30		
CON	26	4	57-	18203 4937550	875	09/32	4816	11		FLW		10	2/00 ST DO	
CON	26	5	57-	13565 4936200	950	07/76	3660	6	FR	35	15	23		
CON	26	7	57-	14232 4935950	1000	05/77	3660	5	FR	152	127	5	2/00 DO	
CON	27	1	57-	3370 4938779	960	03/66	4606	30	FR	20	16	2		
CON	27	3	57-	8536 493										

WATER WELL DATA SYSTEM

PAGE 666

GROUND WATER BULLETIN REPORT

CONCESSION ETC	LOT	WELL NO	UTM EASTING FEET	ELEV DATE	DIA DRILLER IN FEET	TEST RATE GPM	TEST TIME HR/MIN	WATER USE	OWNER/LOG/SCREEN DEPTHS IN FEET TO WHICH FORMATION EXTEND
● CON	11 9	57- 3371	616110 493463	1185 01/66	5206 4	FR 230	235 4	10/00 ST DO	GOODFELLOW H BRWN MSND COCS BRWN MSND GRVL 0248
● CON	11 9	57- 17924	616250 493400	1150 11/79	2653 6	FR 100	100 10	DO	FLEMING B SAND CLAY 0095 GRVL SAND 0160 CLAY 0240
● CON	11 11	57- 10258	617175 4933850	1050 08/73	4713 6	FR 101	80 80	20 2/30 DO	GRYL 0269 HANZLIK RICHARD BRWN SAND CLAY LYRD 0060 BRWN GRVL STNS
● CON	11 11	57- 10259	617500 4934075	1005 06/73	4713 6	FR 85	60 80	4 2/30 DO	DNSE 0100 BRWN GRVL LOOS 0101 CRESSKELL HARRY BRWN SAND CLAY LYRD 0060 (S 0077 08)
CON	11 11	57- 11483	617320 4933957	1050 07/74	4713 6	FR 92	75 75	25 2/30 DO	BRWN SAND LOOS 0085 ROACH B BRWN FSND CLAY BLDR 0035 BRWN GRVL 0051
CON	11 11	57- 14255	617150 4935600	1075 05/77	2104 6	FR 157	112 150	8 8/30 DO	BRWN GRYL 0092 WAGG WILLIAM BRWN SAND DNSE 0150 (S 0143 04) BRWN
CON	11 11	57- 16988	616800 4933600	1025 11/75	1312 6	FR 104	96 100	6 2/00 DO	SAND LOOS 0157 ABBOTT W
CON	11 12	57- 3572	618171 4933486	1010 07/60	4713 6	FR 406	135 240	10 6/00 ST DO	BRWN HPAN SHND 0050 BRWN SAND GRVL 0104 TDSL OC02 CLAY STNS 0030 HSND GRVL CLAY 0160 BLUE CLAY STNS 0390 GRVL MSND 0406 (S 0399 07)
CON	11 12	57- 3373	617231 4932790	1035 07/66	4713 6	FR 76	40 70	10 2/00 ST DO	HORN A TDSL 0002 CLAY STNS 0035 FSND CLAY 0073 CSND 0081 (S 0074 04)
CON	11 13	57- 3374	618526 4932868	935 04/60	4713 6	FR 380	101 248	10 6/00 ST DO	HORN E TDSL OC02 CLAY STNS 0050 HSND GRVL CLAY CLAY 0378 GRVL 0380
CON	11 13	57- 16124	617500 4932200	1000 07/79	2104 6	FR 61	12 30	20 5/00 ST	BLCK TPSL LCOS 0002 BRWN SAND DNSE 0058 BRWN SAND GRYL LOOS 0061 ERBN SAND GRYL CLAY 0061
CON	11 15	57- 7011	618800 4931360	940 06/69	4713 6	FR 47	10 40	5 2/30 DO	CHASCHUCH W BRWN CLAY MSND STNS 0049 (S 0039 08)
CON	11 15	57- 15627	619100 4931800	981 11/78	4241 6	FR 46	27 39	8 2/30 DO	MOULD W BRWN CLAY TILL 0010 BRWN SAND GRYL CLAY 0041 BRWN FSND MSND 0043 BRWN FSND CLYD GRVL 0046 BRWN MSND GRYL 0049 (S 0046 03)
CON	11 15	57- 15631	619100 4931600	981 11/78	4241 6	FR 41	25 35	10 2/00 DO	BUTLER A PRDG 0026 BRWN SAND GRYL CLAY 0039 BRWN FSND MSND GRVL 0040 BRWN MSND GRYL CLAY 0041 BRWN MSND CSND FGVL 0044 (S 0041 03)
CON	11 16	57- 3375	619501 4931680	931 03/58	4713 FR	46	30 38	10 2/00 DO	HARDING R PRDG 0020 BLUE CLAY 0044 GRVL 0046
CON	11 16	57-	619564	930 10/64	4616 6	FR	80 30	60 2	EAST ORO SCHOOL

- CONTINUED -





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Environment

The Ontario Water Resources Act
WATER WELL RECORD

The Ontario Water Resources Act

5722696

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E CHECK <input checked="" type="checkbox"/> CORRECT BOX WHERE APPLICABLE															
COUNTY OR DISTRICT <i>SACRAMENTO</i>		TOWNSHIP, BOROUGH, CITY, TOWN, VILLAGE <i>ORO</i>		CON., BLOCK, TRACT, SURVEY, LEC <i>6</i>		LOT <i>5</i>									
OWNER'S SURNAME FIRST <i>Fran Booth</i>		ADDRESS <i>88- ORO Station</i>						DATE COMPLETED <i>DAY 23 NO 11 YR 85</i>							
U COUNT <i>21</i>		EXISTING <i>✓</i>		MINIMUS <i>✓</i>		PE <i>✓</i>		ELEVATION <i>✓</i>		AC <i>✓</i>		BASIN CODE <i>✓</i>		II <i>✓</i>	
V <i>✓</i>		I <i>✓</i>		J <i>✓</i>		K <i>✓</i>		L <i>✓</i>		M <i>✓</i>		N <i>✓</i>		III <i>✓</i>	
W <i>✓</i>		X <i>✓</i>		Y <i>✓</i>		Z <i>✓</i>								IV <i>✓</i>	

LOG OF OVERTBURDEN AND BEDROCK MATERIALS (SEE INSTRUCTIONS)

71	PUMPING RATE METHOD 1. ELE PUMP 2. BAUER	PUMPING RATE 12 GPM	140	DURATION OF PUMPING 16 HOURS 30 MIN
	STATIC LEVEL 10-00	WATER LEVEL END OF PUMPING 00-00	WATER LEVELS DURING 15 MINUTES 20 MINUTES 25 MINUTES	1. <input type="checkbox"/> PUMPING 2. <input type="checkbox"/> RECOVERY
221 FEET IF FLOWING, GIVE RATE 00-01	270 FEET 270 FEET	270 270 270	45 MINUTES 50 MINUTES 55 MINUTES	20-27 20-27 20-27
	PUMP INHAKE SET AT GPM		MAIN AT END OF FEET 42	
RECOMMENDED PUMP TYPE <input type="checkbox"/> SHALLOW <input checked="" type="checkbox"/> DEEP	RECOMMENDED PUMP SETTING 270	FEET	1. <input type="checkbox"/> CLEAR 2. <input type="checkbox"/> CLOUDY	
		FEET RATE	RECOMMENDED PUMPING RATE 12 GPM	

FINAL STATUS OF WELL	1 <input type="checkbox"/> WATER SUPPLY	6 <input type="checkbox"/> ABANDONED, INSUFFICIENT SUPPLY
	2 <input type="checkbox"/> OBSERVATION WELL	7 <input type="checkbox"/> ABANDONED POOR QUALITY
	3 <input type="checkbox"/> TEST HOLE	8 <input type="checkbox"/> UNFINISHED
	4 <input type="checkbox"/> RECHARGE WELL	9 <input type="checkbox"/> DEWATERING
10-14		
WATER USE	1 <input type="checkbox"/> DOMESTIC	5 <input type="checkbox"/> COMMERCIAL
	2 <input checked="" type="checkbox"/> AGRIC	6 <input type="checkbox"/> MUNICIPAL
	3 <input type="checkbox"/> IRRIGATION	7 <input type="checkbox"/> PUBLIC SUPPLY
	4 <input type="checkbox"/> INDUSTRIAL	8 <input type="checkbox"/> COOLING OR AIR CONDITIONING
	<input type="checkbox"/> OTHER	9 <input type="checkbox"/> NOT USED
15		
METHOD OF CONSTRUCTION	1 <input type="checkbox"/> CABLE TOOL	6 <input type="checkbox"/> BORING
	2 <input type="checkbox"/> ROTARY (CONVENTIONAL)	7 <input type="checkbox"/> DIAMOND
	3 <input type="checkbox"/> ROTARY (REVERSE)	8 <input type="checkbox"/> JETTING
	4 <input type="checkbox"/> ROTARY (AIR)	9 <input type="checkbox"/> DRIVING
	5 <input type="checkbox"/> AIR PERCUSSION	10 <input type="checkbox"/> DISSOLVING
		11 <input type="checkbox"/> OTHER

CONTRACTOR NAME OF WELL CONTRACTOR ADDRESS	WELL CONTRACTOR LICENCE NUMBER	
	1383	
NAME OF WELL TECHNICIAN ADDRESS	WELL TECHNICIAN LICENCE NUMBER	
	78250	
SIGNATURE OF TECHNICIAN/CONTRACTOR		
SUBMISSION DATE		
DAT	ND	TR

LOCATION OF WELL

IN DIAGRAM BELOW SHOW DISTANCES OF WELL FROM ROAD AND
LOT LINE INDICATE NORTH BY ARROW

NORTH ↑

Detailed description: The diagram shows a vertical line representing a road labeled 'Bass Lake Rd'. A horizontal line to the right represents a 'LOT LINE'. A point on the road is labeled 'X'. A distance of '1.3 KM' is marked between point X and the road. A distance of '6 KM' is marked between point X and the lot line. A vertical line segment connects point X to the road. To the left of this vertical line, the label '5' is written above 'ORO'. To the right, the label '6' is written above 'ORO'. Below the horizontal line, the number '17354' is written.

OFFICE USE ONLY	DATA SOURCE	BB	CONTRACTOR	TR-62	DATE RECEIVED	4-1	83-48	40
	DATE OF INSPECTION /		INSPECTOR	DEC 23 1987				
REMARKS								





Ministry
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Environment

The Ontario Water Resources Act
WATER WELL RECORD

1. PRINT ONLY IN SPACES PROVIDED

2. CHECK [X] CORRECT BOX WHERE APPLICABLE

5725566 57010 COM 106

COUNTY OR DISTRICT		TOWNSHIP, BOROUGH, CITY, TOWN, VILLAGE		CON. BLOCK, SHAPE, SURVEY, LIC.		LOT	
<u>SIMCOE</u>		<u>ORO</u>		<u>6</u>		<u>7</u>	
OWNER (SURNAME FIRST) <u>DOVER GARY</u>		ADDRESS <u>24 Barrett Crs. Barrie Ont.</u>		DATE COMPLETED <u>11-09-89</u>		ACRES <u>.89</u>	
ZONE <u>V</u>		BASING <u>1</u>		ROTATING <u>1</u>		RC <u>1</u>	
ELEVATION <u>1</u>		RC <u>1</u>		BASIN CODE <u>1</u>		II <u>1</u>	
21		1		2		3	

LOG OF OVERTBURDEN AND BEDROCK MATERIALS (SEE INSTRUCTIONS)

A horizontal ruler scale showing markings from 31 to 32 inches. The scale has major tick marks every 1/16 inch. The numbers 31 and 32 are at the far left and right ends respectively. The first tick mark after 31 is labeled '31'. The last tick mark before 32 is labeled '32'.

PUMPING TEST METHOD		PUMPING RATE	10-10	DURATION OF PUMPING
<input type="checkbox"/> PUMP	<input checked="" type="checkbox"/> BAILER	44 GPM	2	15-18 HOURS
STATIC LEVEL	WATER LEVEL END OF PUMPING	28	WATER LEVELS DURING	
135'	200'		<input type="checkbox"/> PUMPING	<input type="checkbox"/> RECOVERY
TEST		15 MINUTES	10 MINUTES	45 MINUTES
		20-25	20-25	20-25
IF FLOWING, GIVE RATE		SP-10	PUMP INTAKE SET AT	WATER AT END OF TEST
		GPM	SP-10	44
RECOMMENDED PUMP TYPE		RECOMMENDED PUMP SETTING	43-45 43-45 300	RECOMMENDED PUMPING RATE
<input type="checkbox"/> SHALLOW		<input checked="" type="checkbox"/> DEEP		44 GPM
0' 1' 2'				

IN DIAGRAM BELOW SHOW DISTANCES OF WELL FROM ROAD AND LOT LINE. INDICATE NORTH BY ARROW.

FINAL STATUS OF WELL	<input checked="" type="checkbox"/> WATER SUPPLY	<input type="checkbox"/> ABANDONED, INSUFFICIENT SUPPLY
	<input type="checkbox"/> CONVENTIONAL WELL	<input type="checkbox"/> ABANDONED, POOR QUALITY
	<input type="checkbox"/> TEST HOLE	<input type="checkbox"/> UNFINISHED
	<input type="checkbox"/> MECHANIC WELL	<input type="checkbox"/> Dewatering
WATER USE	<input checked="" type="checkbox"/> DOMESTIC	<input type="checkbox"/> COMMERCIAL
	<input type="checkbox"/> STOCK	<input type="checkbox"/> MUNICIPAL
	<input type="checkbox"/> IRRIGATION	<input type="checkbox"/> PUBLIC SUPPLY
	<input type="checkbox"/> INDUSTRIAL	<input type="checkbox"/> COOLING OR AIR CONDITIONING
	<input type="checkbox"/> OTHER	<input type="checkbox"/> NOT USED
METHOD OF CONSTRUCTION	<input checked="" type="checkbox"/> CABLE TOOL	<input type="checkbox"/> BORING
	<input type="checkbox"/> ROTARY (CONVENTIONAL)	<input type="checkbox"/> DIAMOND
	<input type="checkbox"/> ROTARY (REVERSE)	<input type="checkbox"/> JEEING
	<input type="checkbox"/> ROTARY (AIR)	<input type="checkbox"/> DRIVING
	<input type="checkbox"/> AIR PERCUSSION	<input type="checkbox"/> DIGGING <input type="checkbox"/> OTHER

DRILLER'S REMARKS

County Rd 11
1/2 mile

58195

CONTRACTOR NAME OF WELL CONTRACTOR ADDRESS	MARCHILDON DRILLING	WELL CONTRACTOR'S LICENCE NUMBER	3660	DATA SOURCE	SA	CONTRACTOR	SD-44	DATE RECEIVED	SD-50	SD
	1PK1 SHANTY BAY ONT	NAME OF WELL TECHNICIAN	WELL TECHNICIAN'S LICENCE NUMBER	3660	DATE OF INSPECTION	36-60	SEP 21 1989			
SIGNATURE OF TECHNICIAN/CONTRACTOR	HETER MARCHILDON		103564	REMARKS						
JULY 15 1989		COMMISSION DATE								



Ministry
of the
Environment

The Ontario Water Resources Act
WATER WELL RECORD

I. PRINT ONLY IN SPACES PROVIDED

11

5720114

MUNICIPAL COMMISSION

1. PRINT ONLY IN SPACES PROVIDED		11	5720114								
2. CHECK <input checked="" type="checkbox"/> CORRECT BOX WHERE APPLICABLE											
COUNTY OR DISTRICT		TOWNSHIP, VILLAGE, CITY, TOWN, VILLAGE		CON., BLOCK, TRACT, SURVEY ETC		LOT, SEC.					
<u>Simsel</u>		<u>Oro</u>		7		6					
OWNER (SURNAME FIRST)		ADDRESS		DATE COMPLETED		RE-101					
<u>Chris Bennett</u>		<u>RR 2 Hawkesbury</u>		DAY	MO	<u>June 8</u>					
21	U	ZONE	EASTING	NORTHING	RC	EL ELEVATION	RC	BASIN CODE	II	III	IV

LOG OF OVERTBURDEN AND BEDROCK MATERIALS (SEE INSTRUCTIONS)

PUMPING TEST METHOD		PUMPING RATE	15-18	DURATION OF PUMPING	
<input type="checkbox"/> PUMP	<input checked="" type="checkbox"/> TRAILER	3	4	15-18 HOURS	12-18 GPM
STATIC LEVEL	WATER LEVEL END III PUMPING	36	WATER LEVELS DURING		
25 FEET	36 FEET	36	15 MINUTES	30 MINUTES	45 MINUTES
		36	15-18 FEET	36	36-45 FEET
IF FLOWING, GIVE RATE	36 GPM	PUMP INJAR SET AT	WATER AT END OF TEST		
		—	—	—	45
RECOMMENDED PUMP TYPE		RECOMMENDED PUMP SETTING	42-45 FEET	RECOMMENDED PUMP SETTING	42-45 GPM
<input type="checkbox"/> SHALLOW		38	FEET	3	GPM

6008 LOCATION OF WELL

IN DIAGRAM BELOW SHOW DISTANCES OF WELL FROM ROAD AND
LOT LINE INDICATE NORTH BY ARROW.

93 HNY CRAIGHURST

HORSE

FINAL STATUS OF WELL	<input checked="" type="checkbox"/> WATER SUPPLY	<input type="checkbox"/> ABANDONED, INSUFFICIENT SUPPLY	
	<input type="checkbox"/> OBSERVATION WELL	<input type="checkbox"/> ABANDONED POOR QUALITY	
	<input type="checkbox"/> TEST HOLE	<input type="checkbox"/> UNFINISHED	
	<input type="checkbox"/> RECHARGE WELL		
WATER USE	<input checked="" type="checkbox"/> DOMESTIC	<input type="checkbox"/> COMMERCIAL	
	<input type="checkbox"/> STOCK	<input type="checkbox"/> MUNICIPAL	
	<input type="checkbox"/> IRRIGATION	<input type="checkbox"/> PUBLIC SUPPLY	
	<input type="checkbox"/> INDUSTRIAL	<input type="checkbox"/> COOLING OR AIR CONDITIONING	
	<input type="checkbox"/> OTHER	<input type="checkbox"/> NOT USED	
	METHOD OF DRILLING	<input type="checkbox"/> CABLE TOOL	<input checked="" type="checkbox"/> BORING
		<input type="checkbox"/> ROTARY (CONVENTIONAL)	<input type="checkbox"/> JETTING
<input type="checkbox"/> ROTARY (REVERSE)		<input type="checkbox"/> DRIVING	
<input type="checkbox"/> ROTARY (AIR)			
<input type="checkbox"/> AIR PERCUSSION			

CON

COLLISION

ESHOE VALLEY RP

DRILLERS REMARKS

NAME OF WELL CONTRACTOR LONE STAR WELL DIGGING		LICENCE NUMBER 3413	DATA SOURCE DATE OF INSPECTION REMARKS	SR	CONTRACTOR SR-62	DATE RECEIVED	REG. NO.
ADDRESS SITE 11, R.R. 6 BARRIE, ONTARIO		LICENCE NUMBER 3744		B. Moore	25 09 85		
NAME OF PERSON SIGNING B. Moore		(705) 436-4359					
SIGNATURE OF CONTRACTOR B. Moore		SUBMISSION DATE DAY 10 MO JUN YR 85					



Ministry
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Environment
Ontario

The Ontario Water Resources Act

WATER WELL RECORD

5718749

MUNICIPALITY
TOWNSHIP, BOROUGH, CITY, TOWN, VILLAGE

CON., BLOCK, THAL., SURVEY ETC.

COUNTY OR DISTRICT		TOWNSHIP, BOROUGH, CITY, TOWN, VILLAGE		CON., BLOCK, THAL., SURVEY ETC.	LOT	RR.
Simcoe	Oro	VII			8	
OWNER, SURNAME FIRST	ADDRESS			DATE COMPLETED	DAY	MO
Garside, Bob	244 St. Vincent St., BARRIE, Ont. L4M 3Z0			31	8	83

21	TIME	EASTING	NUINTING	RC	ELEVATION	RC	RAISEN ELEV.	H	VIS.	EV.
1	10	11	12	13	14	15	16	17	18	19

LOG OF OVERTBURDEN AND BEDROCK MATERIALS (SEE INSTRUCTIONS)											
GENERAL COLOUR	MOST COMMON MATERIAL	OTHER MATERIALS	GENERAL DESCRIPTION								DEPTH - FEET
			FROM	TO							
	top soil										0
	sand										1
	fine gravel & stone, cemented sand										8
	cemented sand, gravel/stones										65
	sand & gravel										95
	fine gravel										123
grey	clay										126
	sand gravel/streaks clay, silt										142
	silt, fine gravel/clay, sand										225
	cemented sand & gravel										260
	med.-coarse sand										299
	cemented sand & clay										312
											320

31	TIME	EASTING	NUINTING	RC	ELEVATION	RC	RAISEN ELEV.	H	VIS.	EV.
32	10	11	12	13	14	15	16	17	18	19

41) WATER RECORD		51) CASING & OPEN HOLE RECORD		61) PLUGGING & SEALING RECORD					
WATER FOUND AT FEET	KIND OF WATER	INSIDE DIAM. INCHES	MATERIAL	WALL THICKNESS INCHES	DEPTH - FEET	SCREEN	SIEVE OR OPENING SIZE INCHES	DIAMETER INCHES	LENGTH FEET
10-11	1 <input type="checkbox"/> FRESH 2 <input type="checkbox"/> SULPHUR 3 <input type="checkbox"/> SALTY 4 <input type="checkbox"/> MINERAL	10-11	1 <input type="checkbox"/> STEEL 2 <input type="checkbox"/> GALVANIZED 3 <input type="checkbox"/> CONCRETE 4 <input type="checkbox"/> OPEN HOLE	10-11	10		10-11	10-11	10-11
18-19	1 <input type="checkbox"/> FRESH 2 <input type="checkbox"/> SULPHUR 3 <input type="checkbox"/> SALTY 4 <input type="checkbox"/> MINERAL	18-19	1 <input type="checkbox"/> STEEL 2 <input type="checkbox"/> GALVANIZED 3 <input type="checkbox"/> CONCRETE 4 <input type="checkbox"/> OPEN HOLE	18-19	10		18-19	18-19	18-19
20-21	1 <input type="checkbox"/> FRESH 2 <input type="checkbox"/> SULPHUR 3 <input type="checkbox"/> SALTY 4 <input type="checkbox"/> MINERAL	20-21	1 <input type="checkbox"/> STEEL 2 <input type="checkbox"/> GALVANIZED 3 <input type="checkbox"/> CONCRETE 4 <input type="checkbox"/> OPEN HOLE	20-21	10		20-21	20-21	20-21
26-27	1 <input type="checkbox"/> FRESH 2 <input type="checkbox"/> SULPHUR 3 <input type="checkbox"/> SALTY 4 <input type="checkbox"/> MINERAL	26-27	1 <input type="checkbox"/> STEEL 2 <input type="checkbox"/> GALVANIZED 3 <input type="checkbox"/> CONCRETE 4 <input type="checkbox"/> OPEN HOLE	26-27	10		26-27	26-27	26-27
30-31	1 <input type="checkbox"/> FRESH 2 <input type="checkbox"/> SULPHUR 3 <input type="checkbox"/> SALTY 4 <input type="checkbox"/> MINERAL	30-31	1 <input type="checkbox"/> STEEL 2 <input type="checkbox"/> GALVANIZED 3 <input type="checkbox"/> CONCRETE 4 <input type="checkbox"/> OPEN HOLE	30-31	10		30-31	30-31	30-31

PUMPING TEST	PUMPING TEST METHOD	PUMPING RATE	DURATION OF PUMPING	
	1 <input type="checkbox"/> PUMP 2 <input type="checkbox"/> HAULER	10 GPM	15-16 HOURS	
STATIC LEVEL	WATER LEVEL END OF PUMPING	WATER LEVELS DURING PUMPING		
18-21	22-24	15 MINUTES	30 MINUTES	45 MINUTES
FEET	FEET	FEET	FEET	FEET
IF FLOWING, GIVE RATE	30-41	PUMP INTAKE SET AT	WATER AT END OF TEST	
RECOMMENDED PUMP TYPE	RECOMMENDED PUMP	RECOMMENDED PUMPING RATE		
	SHALLOW DEEP	FEET		

FINAL STATUS OF WELL	1 <input type="checkbox"/> WATER SUPPLY	6 <input checked="" type="checkbox"/> ABANDONED, INSUFFICIENT SUPPLY
	2 <input type="checkbox"/> OBSERVATION WELL	7 <input type="checkbox"/> ABANDONED POOR QUALITY
WATER USE	3 <input type="checkbox"/> TEST HOLE	8 <input type="checkbox"/> UNFINISHED
	4 <input type="checkbox"/> RECHARGE WELL	
METHOD OF DRILLING	1 <input type="checkbox"/> DOMESTIC	5 <input type="checkbox"/> COMMERCIAL
	2 <input type="checkbox"/> STOCK	6 <input type="checkbox"/> MUNICIPAL
3 <input type="checkbox"/> IRRIGATION	7 <input type="checkbox"/> PUBLIC SUPPLY	
4 <input type="checkbox"/> INDUSTRIAL	8 <input type="checkbox"/> COOLING OR AIR CONDITIONING	
	9 <input type="checkbox"/> OTHER	10 <input type="checkbox"/> NOT USED

CONTRACTOR	NAME OF WELL CONTRACTOR	LICENCE NUMBER
	Snider Drilling and Equipment Ltd.	4816
ADDRESS	R. R. #1, (Craighurst), BARRIE, Ont. L4M 4Y8	LICENCE NUMBER
	Phillip Brown.	
SIGNATURE OF CONTRACTOR	SUBMISSION DATE	
	Snider Drilling and Equipment Ltd.	NO. 100

LOCATION OF WELL										
IN DIAGRAM BELOW SHOW DISTANCES OF WELL FROM ROAD AND LOT LINE. INDICATE NORTH BY ARROW.										
DRILLERS REMARKS										

OFFICE USE ONLY	DATA SOURCE	CONTRACTOR	DATE RECEIVED
			17 11 83
	DATE OF INSPECTION	INSPECTOR	
	REMARKS		



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The Ontario Water Resources Act

WATER WELL RECORD

Ontario

Ministry
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2. CHECK <input checked="" type="checkbox"/> CORRECT BOX WHERE APPLICABLE		1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24
COUNTY OR DISTRICT		TOWNSHIP, BOROUGH, CITY, TOWN, VILLAGE											CON., BLOCK, TRACT, SURVEY ETC.				LOT				SUB-Lot				
Simcoe		ORO											VII				8								
OWNER (SURNAME FIRST)		ADDRESS											DATE COMPLETED				48-52								
Carside, Bob		244 St. Vincent St., BARRIE, Ont. L4M 3Z8											DAY 9				MO 9				YR 83				

LOG OF OVERTBURDEN AND BEDROCK MATERIALS (SEE INSTRUCTIONS)

total depth: 370 feet

PUMPING TEST METHOD		10	PUMPING RATE	10.14	DURATION OF PUMPING	
<input checked="" type="checkbox"/> PUMP	<input checked="" type="checkbox"/> AIR HAULER		4	GPM	15-16	17-18
STATIC LEVEL	WATER LEVEL END OF PUMPING		15	WATER LEVELS DURING		1-3 PUMPING 4 RECOVERY
1025'	1024.5'		15 MINUTES	30 MINUTES	45 MINUTES	60 MINUTES
224 FEET	FEET		2026'	2027'	2028'	2029'
IF FLOWING, GIVE RATE	30-45		FEST	FEST	FEST	FEST
			PUMP INTAKE SET AT	WATER AT END OF TEST		
						45
RECOMMENDED PUMP TYPE		45-45		I <input type="checkbox"/> CLEAR I <input type="checkbox"/> CLOUDY		
<input type="checkbox"/> SHALLOW <input checked="" type="checkbox"/> DEEP		RECOMMENDED PUMP SETTING	300	FEET	RECOMMENDED PUMP RATE	4 GPM
G-53						

FINAL STATUS OF WELL	<input type="checkbox"/> WATER SUPPLY	<input type="checkbox"/> ABANDONED, INSUFFICIENT SUPPLY
	<input type="checkbox"/> OBSERVATION WELL	<input type="checkbox"/> ABANDONED, POOR QUALITY
	<input type="checkbox"/> TEST HOLE	<input type="checkbox"/> UNFINISHED
	<input type="checkbox"/> RECHARGE WELL	
WATER USE	<input type="checkbox"/> DOMESTIC	<input type="checkbox"/> COMMERCIAL
	<input type="checkbox"/> STOCK	<input type="checkbox"/> MUNICIPAL
	<input type="checkbox"/> IRRIGATION	<input type="checkbox"/> PUBLIC SUPPLY
	<input type="checkbox"/> INDUSTRIAL	<input type="checkbox"/> COOLING OR AIR CONDITIONING
	<input type="checkbox"/> OTHER	<input type="checkbox"/> NOT USED
METHOD OF DRILLING	<input type="checkbox"/> CABLE TOOL	<input type="checkbox"/> BORING
	<input checked="" type="checkbox"/> ROTARY (CONVENTIONAL)	<input type="checkbox"/> DIAMOND
	<input type="checkbox"/> ROTARY (REVERSE)	<input type="checkbox"/> JETTING
	<input type="checkbox"/> ROTARY (AIR)	<input type="checkbox"/> DRIVING
	<input type="checkbox"/> AIR PERCUSSION	

LOCATION OF WELL

NAME OF WELL CONTRACTOR		LICENCE NUMBER		
Snider Drilling and Equipment Ltd.		1816		
ADDRESS				
R.R. #1, (Craighurst), BARRIE, Ont.		4M 4Y9		
NAME OF DRILLER OR SUPER		LICENCE NUMBER		
Ronald Jensen & Allan Wright				
SIGNATURE OF CONTRACTOR	SUBMISSION DATE			
Snider Drilling and Equipment Ltd.		NO _____	YR _____	
OFFICE USE ONLY				
DATE SIGNED	BY	CONTRACTOR	SP-RE	INTERVIEWED
				17 11 83
DATE OF INSPECTION		INSPECTOR		
REMARKS				



The logo consists of a stylized tree or leaf design inside a circle, with the word "Ontario" written below it.

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3. CHECK <input checked="" type="checkbox"/> CORRECT BOX WHERE APPLICABLE		1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23
COUNTY OR DISTRICT	TOWNSHIP, BOROUGH, CITY, TOWN, VILLAGE		CLO., BLOCK, TRACE, SURVEY ETC												LOT	LINE								
Simcoe	Oro		VII													8								
OWNER (S) NAME FIRSTS	16-47	ADDRESS	Box 514, BARRIE, Ont. L4M 4T7												DATE COMPLETED	40-82								
Alexander, E. R.															DAY	25	MO	9	YR	84				

31 **32** **33**

41 WATER RECORD		51 CASING & OPEN HOLE RECORD				52 STATUS OF OPENING BELOW MD.			
WATER FOUND AT FEET	KIND OF WATER	INSIDE DIAM. INCHES	MATERIAL	WALL THICKNESS INCHES	DEPTH - FEET	SCREEN	DIAMETER	LENGTH	
30-32	1 <input checked="" type="checkbox"/> FRESH 2 <input type="checkbox"/> SULPHUR 3 <input type="checkbox"/> SALTY 4 <input type="checkbox"/> MINERAL	10-10	1 <input type="checkbox"/> STEEL 2 <input type="checkbox"/> GALVANIZED 3 <input type="checkbox"/> CONCRETE 4 <input type="checkbox"/> OPEN HOLE	.12	10		6 INCHES	3 FEET	
154	18-19	6	1 <input type="checkbox"/> STEEL 2 <input type="checkbox"/> GALVANIZED 3 <input type="checkbox"/> CONCRETE 4 <input type="checkbox"/> OPEN HOLE	.188	+1	148	6 INCHES DEPTH TO SURF. ON SCREEN	148 FEET	
	20-22		1 <input type="checkbox"/> STEEL 2 <input type="checkbox"/> GALVANIZED 3 <input type="checkbox"/> CONCRETE 4 <input type="checkbox"/> OPEN HOLE						
	25-26		1 <input type="checkbox"/> STEEL 2 <input type="checkbox"/> GALVANIZED 3 <input type="checkbox"/> CONCRETE 4 <input type="checkbox"/> OPEN HOLE						
	30-33		1 <input type="checkbox"/> STEEL 2 <input type="checkbox"/> GALVANIZED 3 <input type="checkbox"/> CONCRETE 4 <input type="checkbox"/> OPEN HOLE						

PUMPING TEST METHOD		10	PUMPING RATE	10-14	DURATION OF PUMPING, HRS
<input type="checkbox"/> PUMP	<input checked="" type="checkbox"/> TRAILER		8	GPM	2
STATIC LEVEL	WATER LEVEL END OF PUMPING	75	10-10 HOURS 10-10 MINS		
10-47	24-24				
WATER LEVELS DURING		1) <input type="checkbox"/> PUMPING 2) <input type="checkbox"/> RECOVERY			
15 MINUTES		30 MINUTES	45 MINUTES	60 MINUTES	
20-18		20-31	21-36	21-37	
130 FEET	152 FEET	FEET	FEET	FEET	FEET
IF FLOWING, GIVE RATE		20-47	PUMP INTAKE SITE AT		WATER AT END OF TEST
					42
RECOMMENDED PUMP TYPE		GPM	RECOMMENDED PUMP		RECOMMENDED PUMP
<input type="checkbox"/> SHALLOW		<input checked="" type="checkbox"/> DEEP	42-45	42-45	42-45
SETTING		152 FEET	RATE	5 GPM	
SD-92					

LOCATION OF WELL

IN DIAGRAM BELOW SHOW DISTANCES OF WELL FROM ROAD AND
LOT LINE INDICATE NORTH BY ARROW.

FINAL STATUS OF WELL	<input checked="" type="checkbox"/> WATER SUPPLY	<input type="checkbox"/> ABANDONED, INSUFFICIENT SUPPLY
	<input type="checkbox"/> OBSERVATION WELL	<input type="checkbox"/> ABANDONED, POOR QUALITY
	<input type="checkbox"/> TEST HOLE	<input type="checkbox"/> UNFINISHED
	<input type="checkbox"/> RECHARGE WELL	
WATER USE	<input type="checkbox"/> DOMESTIC	<input type="checkbox"/> COMMERCIAL
	<input type="checkbox"/> STOCK	<input type="checkbox"/> MUNICIPAL
	<input type="checkbox"/> IRRIGATION	<input type="checkbox"/> PUBLIC SUPPLY
	<input type="checkbox"/> INDUSTRIAL	<input type="checkbox"/> COOLING OR AIR CONDITIONING
	<input type="checkbox"/> OTHER	<input type="checkbox"/> NOT USED
METHOD OF DRILLING	<input type="checkbox"/> CABLE TOOL	<input type="checkbox"/> BORING
	<input type="checkbox"/> ROTARY (CONVENTIONAL)	<input type="checkbox"/> DIAMOND
	<input type="checkbox"/> ROTARY (REVERSE)	<input type="checkbox"/> JETTING
	<input type="checkbox"/> ROTARY (AIR)	<input type="checkbox"/> DRIVING
	<input type="checkbox"/> AIR PERCUSSION	
DRILLERS REMARKS		

NAME OF WELL CONTRACTOR		LICENCE NUMBER				
<u>Snider Drilling and Equipment Ltd.</u>		<u>4816</u>				
ADDRESS						
<u>R.R. #1, (Craighurst), BARRIE, Ont.</u>		<u>14M 4YB</u>				
NAME OF DRILLER OR WORKER		LICENCE NUMBER				
<u>Ronald Jensen</u>		<u></u>				
SIGNATURE OF CONTRACTOR		SUBMISSION DATE				
<u>Snider Drilling and Equipment Ltd.</u>		<u>PHO. _____ YR. _____</u>				
CONTRACTOR	OFFICE USE ONLY	DATA SOURCE	SD	CONTRACTOR	SD NO.	LAST RECEIVED
						<u>10 1284</u>
		DATE OF INSPECTION		INSPECTOR		
		REMARKS				



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WATER WELL RECORD

LOG OF OVERRBURDEN AND BEDROCK MATERIALS (SEE INSTRUCTIONS)

31 **32**

71	PUMPING TEST METHOD PUMP A PUMP B TRAILER	10	PUMPING RATE 7 GPM	1-12 2 HOURS	DURATION OF PUMPING 15-16 HOURS	12 GPM
	STATIC LEVEL 154 FEET	WATER LEVEL END OF PUMPING 2.00 FEET	WATER LEVELS DURING 15 MINUTES 20-22 FEET		45 MINUTES 20-31 FEET	45-55 FEET
IF FLOWING, GIVE RATE GPM		PUMP INAKE SET AT GPM		WATER AT END OF TEST CLEAR <input type="checkbox"/> CLOUDY <input checked="" type="checkbox"/>		
RECOMMENDED PUMP TYPE <input type="checkbox"/> SHALLOW <input checked="" type="checkbox"/> DEEP		RECOMMENDED PUMP SETTING 220 FEET	40-45 GPM	RECOMMENDED PUMPING RATE 7 GPM	40-45 GPM	

LOCATION OF WELL

CONTRACTOR	NAME OF WELL CONTRACTOR	LICENCE NUMBER
	MARSHALDON DRILLING LTD	3660
ADDRESS	KRHISHTANTY BAY ONT L0L0C0	
NAME OF DRILLER OR BORER	LICENCE NUMBER	
PETER MARSHALDON	3660	
SIGNATURE OF CONTRACTOR	SUBMISSION DATE	
Ruth Marshalldon	DAY	NO
	7	yr 56

OFFICE USE ONLY	DATE SOURCE	SE	CONTRACTOR	SR-SE	DATE RECEIVED	BB-SE	SD
	DATE OF INSPECTION		INSPECTOR				
	REMARKS						



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COUNTY OR DISTRICT Simcoe	TOWNSHIP, RIVER, CITY, VILLAGE Oro Twp.	CON. BLOCK, TRACT, SURVEY, ETC. Con. 7	LOT 11
OWNER (SUBDIVISION FIRST) Seeley & Arnill Aggreg. Ltd.		ADDRESS c/o Henderson Paddon & Assoc, Ltd. 945 3rd. Ave.E., Owen Sound, ON N4K 2K7	DATE COMPLETED DAY 6 MO Apr. YR 89

LOG OF OVERBURDEN AND BEDROCK MATERIALS (See Instructions)

GENERAL CATEGORIES OF ROCK MATERIALS (SEE INSTRUCTIONS)						
GENERAL COLOUR	MOST COMMON MATERIAL	OTHER MATERIALS	GENERAL DESCRIPTION	DEPTH - FEET		
				END	TOP	10
Brown	Sand		Fine		0	12
	Sand	Gravel	Coarse		12	38
	Cobbles	Stones, Gravel			38	44
	Silty sand	Clay	Fine		44	48
	Silty sand	Gravel	Fine		48	105
	Cemented sand	Gravel, Clay			105	145
Brown	Sand		Fine		145	151
Brown	Sand		Coarse		151	178
	Sand	Gravel	Coarse		178	196
Grey	Clay	Silt till			196	200

CASING & OPEN HOLE RECORD					
INDEX ITEM NUMBER	MATERIAL	OPEN HOLE		Casing	
		TYPE	SIZE	TYPE	SIZE
8	STEEL CALCIUM CONCRETE IRON PLASTIC	PIPE	1.250	-?	180
	STEEL CALCIUM CONCRETE IRON PLASTIC	PIPE			

SCREEN	11" x 16" slot	8	21
	11" x 16" slot	8	21
	11" x 16" slot	8	21
	11" x 16" slot	8	21

PLUGGING & SEALING RECORD

PUMPING TEST METHODS		PUMPING RATE	TESTING BY TYPE OF WATER IN AND OUT	
<input checked="" type="checkbox"/> PUMP	<input type="checkbox"/> DRAILER	300 Imp.	24	40
STATIC LEVEL	WATER LEVEL END OF PUMPING	WATER LEVELS DURING	X	CLOUDY
86.71' feet	136.91' feet	133.73' feet	15 MINUTES	10 MINUTES
		134.25' feet	20 MINUTES	20 MINUTES
PUMPING TIME RATE		PUMP INFUSE RATE AT	WATER IN AND OUT	
		160	<input checked="" type="checkbox"/> CLEAR	<input type="checkbox"/> CLOUDY
RECOMMENDED PUMP TYPE		RECOMMENDED TYPE	RECOMMENDED PUMPING RATE	
<input type="checkbox"/> SHALLOW <input checked="" type="checkbox"/> DEEP		160	300	

LOCATION OF WELL

(IN DIAGRAM BELOW SHOW DISTANCES OF WELL FROM ROAD AND
LOT LINE INDICATE NORTH BY ARROW)

Lot 10

CON. VIII

WELL @ 60'

1200'

6

34609

DRILLER'S RECOMMENDATION

PRODUCTION WELL

CONTRACTOR	NAME OF WELL CONTRACTOR	WELL CONTRACTOR LICENCE NUMBER
	Davidson Well Drilling Limited	1737
ADDRESS	Box 486, Wingham, Ontario. N0G 2W0	
NAME OF WELL TECHNICIAN	WELL TECHNICIAN LICENCE NUMBER	
Clearwater Drilling	507	
SIGNATURE OF THE AGENT/CONTRACTOR	SUBMISSION DATE	
<i>[Signature]</i>	28	Apr. 1989



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MUNICIPALITY, TOWN, CITY, COUNTY, ETC.

OH,

E CHECK <input checked="" type="checkbox"/> CORRECT BOX WHERE APPLICABLE		10	11	12	13	14					
CITY, TOWNSHIP, BOROUGH, CITY, TOWN, VILLAGE		CON., BLOCK, TRACT, SURVEY, ETC.			LOT						
Simpson Leach		RR # 2 Con. St. Ont			6						
OWNER (SURNAME FIRST)		ADDRESS			DATE COMPLETED						
18-67		RR # 2 Con. St. Ont			MAY 12	NO 5					
					YR 84						
V	ZONING	EASTING	NORTHING	PC	ELAETION	RC	BASIN CODE	H	I	J	K
21	1	2	3	4	5	6	7	8	9	10	11
	12	13	14	15	16	17	18	19	20	21	22

LOG OF OVERBURDEN AND BEDROCK MATERIALS (SEE INSTRUCTIONS)

41. WATER RECORD		51. CASING & OPEN HOLE RECORD					61. SCREEN				
WATER FOUND AT - FEET	KIND OF WATER	INSIDE DIAM. INCHES	MATERIAL	WALL THICKNESS INCHES	DEPTH - FEET	FROM	TO	SCREEN	SIZE OF OPENING INCHES NO.	DIA. INCHES	LENGTH FEET
146	<input type="checkbox"/> FRESH <input checked="" type="checkbox"/> SULPHUR <input type="checkbox"/> SALTY <input type="checkbox"/> MINERAL	10-12	1 STEEL 2 GALVANIZED 3 CONCRETE 4 OPEN HOLE	.1968	10	10-16		6	5	37.3	FEET
15-19	<input type="checkbox"/> FRESH <input type="checkbox"/> SULPHUR <input checked="" type="checkbox"/> SALTY <input type="checkbox"/> MINERAL	5			0	11-68					
20-25	<input type="checkbox"/> FRESH <input type="checkbox"/> SULPHUR <input type="checkbox"/> SALTY <input type="checkbox"/> MINERAL	10-12	1 STEEL 2 GALVANIZED 3 CONCRETE 4 OPEN HOLE		10	10-15					
25-34	<input type="checkbox"/> FRESH <input type="checkbox"/> SULPHUR <input type="checkbox"/> SALTY <input type="checkbox"/> MINERAL	24-32	1 STEEL 2 GALVANIZED 3 CONCRETE 4 OPEN HOLE		10	27-30					
30-39	<input type="checkbox"/> FRESH <input type="checkbox"/> SULPHUR <input type="checkbox"/> SALTY <input type="checkbox"/> MINERAL	24-32	1 STEEL 2 GALVANIZED 3 CONCRETE 4 OPEN HOLE		10	30-35					

71	PUMPING TEST NUMBER		10	PUMPING RATE	11-14	DURATION OF PUMPING
	<input type="checkbox"/> PUMP	<input checked="" type="checkbox"/> SPINNEN		5 GPM	2 1/2 HOURS	11-18 MIN
PUMPING TEST	STATIC LEVEL	WATER LEVEL END OF PUMPING	25	WATER LEVELS DURING		
	10.73 FEET	23.24 FEET	15 MINUTES	30 MINUTES	45 MINUTES	60 MINUTES
	23 FEET	125 FEET	10.73 FEET	20.10 FEET	29.33 FEET	38.14 FEET
IF FLOWING, GIVE RATE	10.73	PUMP INTAKE SET AT	WATER AT END OF TEST			4.8
		OPEN	FEET	<input type="checkbox"/> CLEAR	<input type="checkbox"/> CLOUDY	
RECOMMENDED PUMP TYPE:		RECOMMENDED PUMP SIZING	FEET	RECOMMENDED PUMPING RATE		
<input type="checkbox"/> SHALLOW	<input checked="" type="checkbox"/> DEEP	135 FEET	5	GPM		
10-11						

FINAL STATUS OF WELL	14	<input type="checkbox"/> WATER SUPPLY <input type="checkbox"/> ABANDONED, INSUFFICIENT SUPPLY <input type="checkbox"/> OBSERVATION WELL <input type="checkbox"/> ABANDONED POOR QUALITY <input type="checkbox"/> TEST HOLE <input type="checkbox"/> UNFINISHED <input type="checkbox"/> RECHARGE WELL
	15-16	<input type="checkbox"/> DOMESTIC <input type="checkbox"/> COMMERCIAL <input type="checkbox"/> STOCK <input type="checkbox"/> MUNICIPAL <input type="checkbox"/> IRRIGATION <input type="checkbox"/> PUBLIC SUPPLY <input type="checkbox"/> INDUSTRIAL <input type="checkbox"/> COOLING OR AIR CONDITIONING <input type="checkbox"/> OTHER <input type="checkbox"/> NOT USED
	17	<input type="checkbox"/> CHISEL TOOL <input type="checkbox"/> BORING <input checked="" type="checkbox"/> ROTARY (CONVENTIONAL) <input type="checkbox"/> DIAMOND <input type="checkbox"/> ROTARY INVERSE <input type="checkbox"/> JETTING <input type="checkbox"/> ROTARY LADS <input type="checkbox"/> DRIVING <input type="checkbox"/> AIR PERCUSSION
		3135

NAME OF WELL CONTRACTOR		LICENCE NUMBER
<i>Hart Well Drilling</i>		<i>3135</i>
CONTRACTOR	ADDRESS	
<i>P.O. #1, Prairie Dr.</i>		
NAME OF DRILLER OR BOREER		LICENCE NUMBER
<i>Ted Wester</i>		
SIGNATURE OF CONTRACTOR		SUBMISSION DATE
<i>G. Hart</i>		DAY <u> </u> NO. <u> </u> YR. <u> </u>

RECORD		54	55	56	57	58	59	60
DEPTH - FEET		SIZES OF OPENING SLOT NO. 1		14-23	DIA. 14-16	14-16	LENGTH	10-10
10	10	SCREEN		6	5 INCHES	37	3 FEET	
		MATERIAL AND TYPE		OPEN TO TOP OF SCREEN		45-4	10	
D	1148	<i>Jackson SS</i>						
		681		PLUGGING & SEALING RECORD				
		DEPTH SET AT - FEET		MATERIAL AND TYPE		CEMENT GROUT, LEAD PACKER, ETC.		
		CHON		10		1		
		10-12		14-17				
		18-20		21-25				
		26-28		30-33				

638 LOCATION OF WELL

IN DIAGRAM BELOW SHOW DISTANCES OF WELL FROM ROAD AND
LOT LINE INDICATE NORTH BY ARROW.

BASS LK. Rd

150 ft

L-600 ft →

OFFICE USE ONLY	18	CONTRACTOR	33-47	DATE RECEIVED	8:00 AM
SOURCE			SEP 01 1987		PM
DATE OF INSPECTION			INSPECTOR		
REMARKS					



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WATER WELL RECORD

572227

MUNICIPALITY: TOWNSHIP, BOROUGH, CITY, TOWN, VILLAGE

CONTRACTOR: CON. BLOCK, TRACT, SURVEY ETC.

LOT: 25-22

OWNER OR DISTRICT		TOWNSHIP, BOROUGH, CITY, TOWN, VILLAGE		CON. BLOCK, TRACT, SURVEY ETC.	
OWNER'S SURNAME FIRST		ADDRESS		DATE COMPLETED	
Leach H		RD # 2 Ossie Station		DAY 9 NO 5 YR 84	
21	U. TOWN	LAWING	WELLING	AC.	ELEVATION
22	10	11	12	13	14
23	15	16	17	18	19
24	20	21	22	23	24
25	25	26	27	28	29
26	30	31	32	33	34
27	35	36	37	38	39
28	40	41	42	43	44
29	45	46	47	48	49
30	50	51	52	53	54
31	55	56	57	58	59
32	60	61	62	63	64

LOG OF OVERBURDEN AND BEDROCK MATERIALS (SEE INSTRUCTIONS)

GENERAL COLOUR	MOST COMMON MATERIAL	OTHER MATERIALS	GENERAL DESCRIPTION		DEPTH - FEET
			FROM	TO	
	Sand		dry, packed	0	33
	Sand		wet	35	45
31					
32					
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Tables

- B-1 Property Locations, Extraction Areas, and Tonnages
- B-2 Aquifer Characteristics
- B-3 Water Level and Stream Flow Monitoring Data
- B-4 Water Quality Analyses

APPENDIX B

DATA TABLES

ORO SEVENTH LINE AGGREGATE PROPERTIES

TABLE B-1
PROPERTY LOCATIONS, EXTRACTION AREAS AND TONNAGES

OPERATOR	CON	LOT	PROPOSED LICENCE AREA (Ha)	EXTRACTION AREA (APPROX.) (Ha)	ACTIVE AREA (Ha)	ASSUMED MAX. TONNAGE (tonnes/yr)	PROPOSED LICENCE TONNAGE (50 % OF LICENCE) (tonnes/yr)	PROPOSED AVERAGE TONNAGE (50 % OF LICENCE) (tonnes/yr)
SEELEY & ARNILL AGGREGATES LTD.	7	E1/2 10,11	50.8	46	46	23	175000	875000
ORO TOWNSHIP	8	W1/2 11	25	25	25	13		
ALFA AGGREGATES LIMITED	7	PART E1/2 9	32.2	26.65	26.65	14	750000	375000
JAMES DICK CONSTRUCTION LIMITED	7	W1/2 7.9, PT 8 E1/2 8	169.7	149	49	25	1000000	500000
THE SARGEANT CO. LTD.	8	W1/2 8	43.8	39	39	20	750000	375000
ALLAN G. COOK LIMITED	8	W1/2 7	38.8	34.16	34.16	17	900000	450000
TOTAL			360	320	220		5150000	2575000

Notes: Assumed maximum active area is maximum cleared area at any one time

Proposed licence tonnage is the maximum allowable extraction in any one year

Proposed average tonnage is the maximum expected average tonnage in any one year

maximum
probable
50%

ORO SEVENTH LINE AGGREGATE PITS
TABLE B-2
AQUIFER CHARACTERISTICS

PROPERTY	LOCATION	AQUIFER	AQUIFER THICKNESS (m)	HYDRAULIC CONDUCTIVITY (cm/s)	METHOD OF CALC.	TRANSMISSIVITY (m ² /dy)	DHL TBLB2	
							STORATIVITY (m ² /dy)	TESTED RATE (m ³ /dy) (l/s) (GM)
James Dick Construction Limited	M6	A1	5.9	0.01, 0.0001	HZ HZ	51 1		
The Sergeant Co. Ltd.	TW5	A1	8.5	0.0037	HV	27		
	TW2	A2	12	0.02 0.04	HZ HZ	207 415		
TW8		A2	4.9		TS TS	22 92		
	OW191	A1	5.2	0.005 0.007	HV HV	22 31	5.6E-05	
Allan G. Cook Limited								
Seeley & Arnill Aggregates Ltd.	1/89	A2	15.5		TS	500	4.4E-04	1964 22.7 300

Note Method of calculation

HZ HAZEN from grain size analysis
 HV HVORSLEV from response testing of well
 TS THEIS from pump testing of well

ORO SEVENTH LINE AGGREGATE PROPERTIES
TABLE B-3
WATER LEVEL AND STREAM FLOW MONITORING DATA

DHL TBLB3

PROPERTY	LOCATION	AQUIFER	GROUND LEVEL ELEVATION (masl)	WATER LEVEL (masl)				
				3 MAY 91	13 MAY 91	22 JUL 91	31 JUL 91	1 OCT 91
James Dick Construction (Charlesworth, 1991)	M6 DC1 DC2 DC4 DC5 11915	A1 PERCHED A1 A1 A1	352.31 367.25 354.08 354.62 352.03	322.9	322.9	323.3		323.5
				334.6	335.6	335.7		335.4
				320.2	320.3	320.3		320.5
				326.2	326.3	326.5		326.9
				325.9	326.2	327.2		327.2
							321.7	
The Sarjeant Co. Ltd. (Dixon Hydrogeology, 1992)	OW1 TW2 OW3 TH4 TW5 TH6 TW7 TW8	A1 A2 A1 A1 A1 A1 A1 A2	354.73 358.10 356.24 347.67 356.22 362.74 356.22 342.88	JUL 91	AUG 91	SEP 91	10 OCT 91	11 DEC 91
							318.5	318.5
				314.5			314.5	
				317.5				
					317.2		316.1	317.2
						317.5		317.3
							309.3	309.9
Allan G. Cook Limited (Dixon Hydrogeology, 1992)	OW1/91	A1	343.22	14 NOV 91 - 11 DEC 91				
				315.2	315.0			
Alfa Aggregates Limited (Dixon Hydrogeology, 1991)	11628 OW1/91	A1	362.26	323.6				
				DRY				
Seeley & Arnill Aggregates Ltd. (Henderson Paddon, 1990)	1/89	A2		APR 89				
				316.00				
				STREAM FLOW (APPROX.) (m ³ /s)				
Coldwater R. west tributary	culvert beneath Rowanwood Rd.			26 NOV 91				
				0.03				
Coldwater R. east tributary	culvert beneath 8th Line				0.13			
Hawkestone Ck. 15/16 SR	bridge at 15/16th SR				0.23			

ORO SEVENTH LINE AGGREGATE PIT
TABLE B-4
WATER QUALITY ANALYSES

THE SARJEANT CO. LTD.
TEST WELL 8

REPORT OF ANALYSIS
ARECO CANADA INC., 28 CONCOURSE GATE, NEPEAN, ONTARIO, K2B 7T7
TELEPHONE: (613) 228 1145 FAX: (613) 228 1148

LABORATORY I.D.:	161091-4	CLIENTS JOB NUMBER:	Dixon, SO9 Q03
SAMPLE MATRIX:	Well Water	DATE SUBMITTED:	16-10-91
REPORT NUMBER:	3292810	DATE REPORTED:	28-10-91

DRINKING WATER CRITERIA	UNITS	RESULTS			
		Well #8			
PARAMETERS					
Colour	T.C.U.	<1			
Hardness(CaCO ₃)	mg/L	123			
Alkalinity(CaCO ₃)	mg/L	115			
Turbidity	N.T.U.	0.7			
Conductivity	uS/cm	250			
pH		8.07			
Fluoride	mg/L	<0.1			
Chloride	mg/L	<1			
Nitrite (N)	mg/L	<0.1			
Nitrate (N)	mg/L	<0.1			
Sulphate	mg/L	4.3			
Calcium	mg/L	16.1			
Magnesium	mg/L	20.1			
Sodium	mg/L	12.9			
Potassium	mg/L	1.1			
Ammonia (N)	mg/L	0.06			
TKN	mg/L	0.5			
Cyanide	mg/L	<0.02			
Phenols	mg/L	<0.002			
TOC	mg/L	2			
DOC	mg/L	2			

ORO SEVENTH LINE AGGREGATE PIT
TABLE B-4
WATER QUALITY ANALYSES

THE SARJEANT CO. LTD.
TEST WELL 8

REPORT OF ANALYSIS

ARECO CANADA INC., 28 CONCOURSE GATE, NEPEAN, ONTARIO, K2E 7T7
TELEPHONE: (613) 228 1145

FAX: (613) 228 1148

LABORATORY I.D.: 161091-4
SAMPLE MATRIX: Well Water
REPORT NUMBER: 3292810

CLIENTS JOB NUMBER: Dixon, SO9 Q03
DATE SUBMITTED: 16-10-91
DATE REPORTED: 28-10-91

DRINKING WATER CRITERIA	UNITS	RESULTS				
		Well #8				
PARAMETERS						
Barium	mg/L	0.11				
Cadmium	mg/L	<0.004				
Chromium	mg/L	<0.01				
Copper	mg/L	<0.01				
Arsenic	mg/L	<0.01				
Boron	mg/L	0.05				
Iron	mg/L	0.04				
Lead	mg/L	<0.04				
Manganese	mg/L	<0.005				
Selenium	mg/L	<0.01				
Silver	mg/L	<0.01				
Zinc	mg/L	<0.01				
Mercury	mg/L	<0.001				
Uranium	mg/L	<0.0002				

ORO SEVENTH LINE AGGREGATE PIT

TABLE B-4

WATER QUALITY ANALYSES

THE SARJEANT CO. LTD.
TEST WELL 8

REPORT OF ANALYSIS

ARECO CANADA INC., 28 CONCOURSE GATE, NEPEAN, ONTARIO, K2E 7T7
TELEPHONE: (613) 228 1145

FAX: (613) 228 1148

LABORATORY I.D.:
SAMPLE MATRIX:
REPORT NUMBER:161091-4
Well Water
3282810CLIENTS JOB NUMBER: Dixon, SO9 Q03
DATE SUBMITTED: 16-10-91
DATE REPORTED: 28-10-91

DRINKING WATER CRITERIA	UNITS	RESULTS			
		Well #8			
Total Coliform	/100ml	0			
Fecal Coliform	/100ml	0			
Fecal Strep.	/100ml	0			
Background	/100ml	TNTC			
Anion Sum	meq/L	2.81			
Cation Sum	meq/L	3.05			
% Difference	%	4.06			
Ion Ratio	AS/CS	0.92			
Conductivity (calc.)	uS/cm	255			
TDS (ion sum calc.)	mg/L	159			
SAR		0.51			
Langelier Index	S.I.	-0.16			

Certified by
Greg Clarkin, B.Sc., GeoChem
Lab Manager

ORO SEVENTH LINE AGGREGATE PIT

TABLE B-4
WATER QUALITY ANALYSESSEELEY & ARNILL AGGREGATES LTD.
WELL 1/89HENDERSON, PADDON & ASSOCIATES
MR. IAN FLEMING

LAB NUMBER..... 128
 SAMPLE NAME..... HENDERSON, PADDON
 SAMPLE LOCATION..... EDGAR GRAVEL PIT

CLIENT'S JOB NUMBER..... 88300
 COLLECTION DATE..... April 21, 1989

<u>DETERMINATION</u>	<u>DETECTION</u>	<u>RESULT</u>
Sodium	0.5 mg/L	2.0
Potassium	0.1 mg/L	1.0
Calcium	0.005 mg/L	62.0
Magnesium	0.001 mg/L	14.0
Hardness	0.05 mg/L	213
Alkalinity (CaCO ₃)	1.0 mg/L	199
Carbonate	1.0 mg/L	1.7
Bicarbonate	1.0 mg/L	197
Sulphate	1.0 mg/L	12.6
Chloride	1.0 mg/L	1.4
Silica (SiO ₂)	0.5 mg/L	10.8
Ortho Phosphate (P)	0.01 mg/L	0.01
Nitrate+Nitrite (N)	0.05 mg/L	0.95
Ammonia (N)	0.05 mg/L	<0.05
Colour (true)	3.0 TCU	<3.0
Turbidity	0.1 NTU	<0.1
Conductivity (25°C)	0.1 umho/cm	423
pH	0.0 units	7.9
Total Organic Carbon	0.5 mg/L	1.0
Iron (total)	0.02 mg/L	0.02
Copper (total)	0.01 mg/L	<0.01
Manganese (total)	0.01 mg/L	<0.01
Zinc (total)	0.01 mg/L	0.02
Cation Sum.....	meq/L...	4.38
Anion Sum.....	meq/L...	4.35
%Difference.....	%.....	0.34
Ion Ratio.....		1.01
TDS (ion sum, calculated)....	mg/L...	349
Conductivity (calc.).....	umho/cm	422
Saturation pH 5 C.....		7.77
Langelier Index 5 C.....		0.18

Tables

- Table C-1 Water Budget Values - Thornthwaite Model
- Table C-2 Water Budget Values - Hargreaves Model
- Table C-3 Summary of Changes to the Local Water Budget
- Table C-4 Estimated Exported Water
- Table C-5 Groundwater Mounding - Theis Equation
a) Minimum Recharge Increase ($1.4 \text{ m}^3/\text{day/Ha}$)
b) Average Recharge Increase ($4.1 \text{ m}^3/\text{day/Ha}$)
c) Maximum Recharge Increase ($6.8 \text{ m}^3/\text{day/Ha}$)
- Table C-6 Groundwater Mounding - Darcy Formula
- Table C-7 Potential Well Interference
- Table C-8 Potential Impact of Washing Operation Wells on Private Wells
- Table C-9 Estimate of Maximum Annual Calcium Chloride Application

ORO SEVENTH LINE AGGREGATE PROPERTIES
TABLE C-1
WATER BUDGET VALUES THORNTHWAITE MODEL

DHL TBLC1

15099 MIDHURST ONT. WATER BUDGET VALUES FOR THE PERIOD 1953 TO 1989

LAT.... 44.27 WATER HOLDING CAPACITY...100 MM HEAT INDEX... 33.90

LONG... 79.46 LOWER ZONE..... 60 MM A..... 1.038

SUMMARY FROM 1953 to 1989

MONTH	TEMP (C)	PCPN	RAIN	MELT	PE	PCPN-PE	AE	DEF	SURP	SNOW	SOIL
1	-8.6	69	7	17	1	68	1	0	23	296	100
2	-7.8	61	14	24	1	60	1	0	37	422	100
3	-3.6	51	21	66	4	47	4	0	84	379	100
4	4.3	64	58	80	31	33	31	0	112	96	98
5	11.1	66	66	0	62	3	61	-1	18	0	86
6	16.2	73	73	0	96	-24	89	-7	5	0	63
7	19.5	89	89	0	144	-55	106	-38	5	0	44
8	19.0	75	75	0	104	-30	74	-31	0	0	34
9	15.0	113	113	0	92	21	75	-17	13	0	51
10	8.8	75	75	0	37	38	35	-2	19	0	76
11	3.1	73	62	6	14	59	14	-0	39	8	92
12	-4.5	94	33	21	3	92	3	0	49	135	99
SUM		901	684	214	589	312	492	-96	407		

15810 ORILLIA ONT. WATER BUDGET VALUES FOR THE PERIOD 1929 TO 1961

LAT...44.37 WATER HOLDING CAPACITY... 100 MM HEAT INDEX... 33.92

LONG...79.24 LOWER ZONE.....60 MM A..... 1.038

15820 ORILLIA TS ONT. WATER BUDGET VALUES FOR THE PERIOD 1980 TO 1989

LAT...44.37 WATER HOLDING CAPACITY... 100 MM HEAT INDEX... 36.34

LONG...79.25 LOWER ZONE.....60 MM A..... 1.074

SUMMARY FROM 1929 to 1961 and 1980 to 1989

MONTH	TEMP (C)	PCPN	RAIN	MELT	PE	PCPN-PE	AE	DEF	SURP	SNOW	SOIL
1	-8.6	66	11	16	1	65	1	0	25	297	97
2	-8.4	56	10	20	1	55	1	0	29	445	98
3	-3.4	76	35	84	5	71	5	0	112	539	99
4	4.8	65	60	76	26	39	26	0	114	87	98
5	11.1	73	73	1	62	11	62	-0	28	0	86
6	16.4	66	66	0	97	-31	87	-10	3	0	64
7	19.8	89	89	0	146	-57	110	-36	2	0	41
8	19.4	70	70	0	106	-36	72	-34	1	0	30
9	16.1	97	97	0	80	17	64	-16	7	0	46
10	9.5	93	93	0	51	42	46	-5	24	0	70
11	2.9	74	62	6	14	60	14	-0	37	11	91
12	-5.0	97	30	21	2	95	2	0	46	164	98
SUM		922	695	223	590	331	489	-101	427		
MEAN VALUES		911	690	219	589	322	491	-99	417		

Note: All measurements are in mm except where noted.

Source: CANADIAN CLIMATE CENTRE

PCPN :Precipitation

AE :Actual Evapotranspiration

MELT :Snow Melt

DEF :AE-PE Difference between available and required water

PE :Potential Evapotranspiration

SURP :SURPLUS Precipitation in excess of Evapotranspiration

PCPN-PE :SURP/DEF (Water Body)

SNOW :Water equivalent of accumulated snow

SOIL :Water storage in Soil

ORO SEVENTH LINE AGGREGATE PITS
TABLE C-2
WATER BUDGET VALUES HARGREAVES MODEL
POTENTIAL EVAPOTRANSPIRATION

$$PE = kd(.38 - .0038h)(t-32)$$

PE Potential Evapotranspiration (in)

k vegetation coefficient

d monthly daytime coefficient

h mean monthly relative humidity (Muskoka Climate Station)

t mean monthly temperature (F)

DHL TBLC2

Deciduous Orchard						Pasture					
15099 MIDHURST ONT.											
MONTH	k	d	h	t	PE	MONTH	k	d	h	t	PE
1						1					
2						2					
3	0.14	0.99	65	25		3	0.11	0.99	65	25	
4	0.45	1.09	55	40	0.65	4	0.25	1.09	55	40	0.36
5	0.49	1.24	52	52	2.22	5	0.29	1.24	52	52	1.31
6	0.74	1.26	58	61	4.34	6	0.33	1.26	58	61	1.93
7	0.71	1.27	58	67	5.06	7	0.31	1.27	58	67	2.21
8	0.55	1.17	60	66	3.35	8	0.32	1.17	60	66	1.95
9	0.43	1.01	63	59	1.65	9	0.32	1.01	63	59	1.23
10	0.36	0.91	64	48	0.71	10	0.22	0.91	64	48	0.43
11						11	0.14	0.77	74	38	0.06
12						12					

ORO SEVENTH LINE AGGREGATE PITS
TABLE C-3
SUMMARY OF CHANGES TO THE LOCAL WATER BUDGET

Increased Recharge Clearing the Excavation		Maximum Extraction Area (Table B-1)	Water Budget Gains Ha	Evaporation Wash Pond m3/dy	Stockpiles m3/dy	Exported Water (Table C-4) m3/dy	Water Budget Losses m3/dy	Net Change m3/dy
m3/dy/Ha (mm/annum)			equivalent avg. Ha	m3/dy				
Avg.			Low	High				
Low	High							
1.4 (50)	4.1 (150)	6.8 (250)	220	308	902	1496	64	32 Max. Avg. Extraction 12.8 Max. Licensed Extraction
							140	234 +572 +1166 -100
							280	467 376 563 -68 +339 +933

Note: Evaporation loss from wash pond is equivalent to 12.8 m3/dy/Ha for June through Aug. assuming a 1 Ha pond per operation.

Evaporation loss from stockpiles is equivalent to a surface area of 1 Ha per operation and an evaporation rate of 12.8 m3/dy/Ha for half of the time during the period of June through Aug.

Dust & Scrap

DHL TBL C3

ORO 7th LINE AGGREGATE PITS

TABLE C-4
ESTIMATED EXPORTED WATER

DHL TBL C4

OPERATOR	PROPOSED LICENCE TONNAGE tonnes/yr	ESTIMATED TONNAGE OF WASHED PRODUCT tonnes/yr	RETAINED WATER m3/yr	EXPORTED WATER (OPERATING 190 DAYS) m3/day	5 %
SEELEY & ARNILL AGGREGATES LTD.	1750000	25 % 437500	13125	21875	69 115
ALFA AGGREGATES LIMITED	750000	25 % 187500	5625	9375	30 49
JAMES DICK CONSTRUCTION LIMITED	1000000	25 % 250000	7500	12500	39 66
THE SARJEANT CO. LTD.	750000	90 % 675000	20250	33750	107 178
ALLAN G. COOK LIMITED	900000	25 % 225000	6750	11250	36 59
TOTAL		1775000	53250	88750	280 467
	PROPOSED AVERAGE TONNAGE (50 % OF LICENCE)				
SEELEY & ARNILL AGGREGATES LTD.	875000	25 % 218750	6563	10938	35 58
ALFA AGGREGATES LIMITED	375000	25 % 93750	2812.5	4637.5	15 25
JAMES DICK CONSTRUCTION LIMITED	500000	25 % 125000	3750	6250	20 33
THE SARJEANT CO. LTD.	375000	90 % 337500	10125	16875	53 89
ALLAN G. COOK LIMITED	450000	25 % 112500	3375	5625	18 30
TOTAL		887500	26525	44375	140 234

ORO SEVENTH LINE AGGREGATE PITS
TABLE C-5a
GROUNDWATER MOUNDING THEIS EQUATION
MINIMUM RECHARGE INCREASE (1.4 M3/DY/HA)

DHL TBLC5A

Q m3/day	Increased recharge							
k m/day	Aquifer hydraulic conductivity							
T m2/day	Aquifer transmissivity							
S	Aquifer storativity							
s m	Mounding at distance r							
t days	Time since mounding started							
r m	Distance from centre of property							
THEIS								
s = $(Q/(4\pi T t)) \ln((2.25 T t)/(r^2 S))$								
Q	T	S	t	MOUNDING (m)				
				<i>r</i> =	200	300	400	500
SEELEY & ARNILL AGGREGATES LTD. 46 Ha								
64	100	0.1	365		0.2	0.1	0.1	0.1
64	200	0.1	365		0.1	0.1	0.1	0.0
64	500	0.1	365		0.0	0.0	0.0	0.0
TOWNSHIP OF ORO 25 Ha								
35	100	0.1	365		0.1	0.1	0.0	0.0
35	200	0.1	365		0.1	0.0	0.0	0.0
37	400	0.1	365		0.0	0.0	0.0	0.0
ALFA AGGREGATES LIMITED 26.65 Ha								
37	50	0.1	365		0.1	0.1	0.1	0.0
37	100	0.1	365		0.1	0.1	0.0	0.0
37	200	0.1	365		0.1	0.0	0.0	0.0
JAMES DICK CONSTRUCTION LIMITED 50 Ha								
70	30	0.1	365		0.3	0.2	0.1	0.0
70	50	0.1	365		0.3	0.2	0.1	0.1
70	100	0.1	365		0.2	0.1	0.1	0.1
THE SARJEANT CO. LTD. 39 Ha								
55	30	0.1	365		0.3	0.1	0.1	0.0
55	50	0.1	365		0.2	0.1	0.1	0.0
55	100	0.1	365		0.1	0.1	0.1	0.1
ALLAN G. COOK LIMITED 34.16 Ha								
48	30	0.1	365		0.2	0.1	0.1	0.0
48	50	0.1	365		0.2	0.1	0.1	0.0
48	100	0.1	365		0.1	0.1	0.1	0.0

ORO SEVENTH LINE AGGREGATE PITS
TABLE C-5b
GROUNDWATER MOUNDING THEIS EQUATION
AVERAGE RECHARGE INCREASE (4.1 M³/DY/HA)

DHL TBLC5B

Q m ³ /day	Increased recharge							
k m/day	Aquifer hydraulic conductivity							
T m ² /day	Aquifer transmissivity							
S	Aquifer storativity							
s m	Mounding at distance r							
t days	Time since mounding started							
r m	Distance from centre of property							
THEIS								
$s = (Q/(4\pi T t)) \ln((2.25 T t)/(r^2 S))$								
Q	T	S	t	MOUNDING (m)				
				r =	200	300	400	500
SEELEY & ARNILL AGGREGATES LTD. 46 Ha								
189	100	0.1	365		0.5	0.3	0.2	0.2
189	200	0.1	365		0.3	0.2	0.2	0.1
189	500	0.1	365		0.1	0.1	0.1	0.1
TOWNSHIP OF ORO 25 Ha								
102	100	0.1	365		0.2	0.2	0.1	0.1
102	200	0.1	365		0.2	0.1	0.1	0.1
102	400	0.1	365		0.1	0.1	0.1	0.1
ALFA AGGREGATES LIMITED 26.65 Ha								
109	50	0.1	365		0.4	0.3	0.2	0.1
109	100	0.1	365		0.3	0.2	0.1	0.1
109	200	0.1	365		0.2	0.1	0.1	0.1
JAMES DICK CONSTRUCTION LIMITED 50 Ha								
205	30	0.1	365		1.0	0.5	0.2	-0.0
205	50	0.1	365		0.8	0.5	0.3	0.2
205	100	0.1	365		0.5	0.4	0.3	0.2
THE SARJEANT CO. LTD. 39 Ha								
160	30	0.1	365		0.8	0.4	0.2	-0.0
160	50	0.1	365		0.6	0.4	0.2	0.1
160	100	0.1	365		0.4	0.3	0.2	0.2
ALLAN G. COOK LIMITED 34.16 Ha								
140	30	0.1	365		0.7	0.4	0.2	-0.0
140	50	0.1	365		0.5	0.3	0.2	0.1
140	100	0.1	365		0.3	0.2	0.2	0.1

DIXON HYDROGEOLOGY LIMITED
 TABLE C-5c
 GROUNDWATER MOUNDING THEIS EQUATION
 MAXIMUM RECHARGE INCREASE (6.8 M3/DY/HA)

DHL TBLC5C

Q m ³ /day	Increased recharge							
k m/day	Aquifer hydraulic conductivity							
T m ² /day	Aquifer transmissivity							
S	Aquifer storativity							
s m	Mounding at distance r							
t days	Time since mounding started							
r m	Distance from centre of property							
THEIS	$s = (Q/(4\pi T t)) \ln((2.25 T t)/(r^2 S))$							
Q	T	S	t	MOUNDING (m)				
				r =	200	300	400	500
SEELEY & ARNILL AGGREGATES LTD. 46 Ha								
313	100	0.1	365		0.8	0.6	0.4	0.3
313	200	0.1	365		0.5	0.4	0.3	0.2
313	500	0.1	365		0.2	0.2	0.2	0.1
TOWNSHIP OF ORO 25 Ha								
170	100	0.1	365		0.4	0.3	0.2	0.2
170	200	0.1	365		0.3	0.2	0.2	0.1
170	400	0.1	365		0.1	0.1	0.1	0.1
ALFA AGGREGATES LIMITED 26.65 Ha								
181	50	0.1	365		0.7	0.4	0.3	0.1
181	100	0.1	365		0.4	0.3	0.2	0.2
181	200	0.1	365		0.3	0.2	0.2	0.1
JAMES DICK CONSTRUCTION LIMITED 50 Ha								
340	30	0.1	365		1.6	0.9	0.4	-0.0
340	50	0.1	365		1.3	0.8	0.5	0.3
340	100	0.1	365		0.8	0.6	0.4	0.3
THE SARJEANT CO. LTD. 39 Ha								
265	30	0.1	365		1.3	0.7	0.3	-0.0
265	50	0.1	365		1.0	0.6	0.4	0.2
265	100	0.1	365		0.6	0.5	0.3	0.3
ALLAN G. COOK LIMITED 34.16 Ha								
232	30	0.1	365		1.1	0.6	0.3	-0.0
232	50	0.1	365		0.9	0.6	0.3	0.2
232	100	0.1	365		0.6	0.4	0.3	0.2

ORO SEVENTH LINE AGGREGATE PITS
TABLE C-6
GROUNDWATER MOUNDING DARCY FORMULA

Hydrogeological Variables							Mounding Variables										
Q (m ³ /day)	Rate of Increased Recharge	Hm (m)	Maximum water table rise at equilibrium														
k (m/day)	Hydraulic Conductivity	Xm (m)	Maximum mound radius on X axis														
b (m)	Aquifer Thickness	Ym (m)	Maximum mound radius on Y axis														
T (m ² /day)	Aquifer Transmissivity	Tm (days)	Time to reach equilibrium														
S	Storage (Specific Yield)	rx (m)	Distance From Center of Pit in X Direction														
Excavation Variables																	
L (m)	Length of Excavation	ry (m)	Distance From Center of Pit in Y Direction														
W (m)	Width of Excavation	hm (m)	Height of Mound (Water Table Rise)														
Mounding Calculations							Mounding hm										
Maximum Mound			1. Hm =	$\frac{Q/(2 \cdot P \cdot I \cdot k \cdot b)}{Hm - Q/(P \cdot I \cdot k \cdot b) \cdot \ln(rx/(L^2))}$			rx= 325			Mounding hm			y= 225				
Mound at Distance			2. hm =	$\frac{Hm - Q/(P \cdot I \cdot k \cdot b) \cdot \ln(rx/(L^2))}{Q/(2 \cdot P \cdot I \cdot k \cdot b) \cdot (1 - \exp((-(2 \cdot P \cdot I \cdot k \cdot b) \cdot (L^2 \cdot W \cdot S)) / (L^2 \cdot W \cdot S)))}$			rx= 330			Mounding hm			300				
Mounding at Time			3. hm =				rx= 340			Mounding hm			310				
MINIMUM RECHARGE INCREASE (1.4 m³/dy/Ha)							Mounding hm										
Seeley & Arnill Aggregates	64	32.3	15.5	501	0.1	700	600	0.0	361	310	0	0.1	0.0	-0.0	-0.0	-0.1	
Alta Aggregates	37	10.0	10.0	100	0.1	650	450	0.1	358	237	100	0.1	0.0	-0.1	-0.2	-0.4	
Township of Oro	35	30.0	15.0	450	0.1	650	450	0.0	352	233	0	0.0	-0.0	-0.1	-0.1	-0.2	
James Dick Construction	70	8.7	5.9	51	0.1	650	600	0.2	381	327	150	0.2	0.1	-0.1	-0.3	-0.5	
The Sarjeant Co.	55	2.6	13.5	35	0.1	650	600	0.2	353	311	175	0.2	-0.1	-0.2	-0.1	-0.2	
Allan G. Cook	48	5.9	5.2	31	0.1	650	600	0.2	365	330	203	0.2	0.1	-0.1	-0.2	-0.3	
AVERAGE RECHARGE INCREASE (4.1 m³/dy/Ha)							Mounding hm										
Seeley & Arnill Aggregates	189	32.3	15.5	501	0.1	700	600	0.1	361	310	25	0.2	0.1	-0.1	-0.2	-0.1	
Alta Aggregates	109	10.0	10.0	100	0.1	650	450	0.2	358	237	100	0.2	0.1	-0.1	-0.2	-0.4	
Township of Oro	102	30.0	15.0	450	0.1	650	450	0.0	352	233	0	0.0	-0.0	-0.1	-0.3	-0.4	
James Dick Construction	205	8.7	5.9	51	0.1	650	600	0.6	381	327	250	0.6	0.5	-0.4	-0.9	-2.8	
The Sarjeant Co.	160	2.6	13.5	35	0.1	650	600	0.7	353	311	425	0.7	0.4	-0.2	-3.3	-6.4	
Allan G. Cook	140	5.9	5.2	31	0.1	650	600	0.7	365	330	475	0.7	0.6	-0.4	-0.8	-2.9	
MAXIMUM RECHARGE INCREASE (6.8 m³/dy/Ha)							Mounding hm										
Seeley & Arnill Aggregates	313	32.3	15.5	501	0.1	700	600	0.1	361	310	10	0.3	0.2	-0.1	-0.3	-1.0	
Alta Aggregates	181	10.0	10.0	100	0.1	650	450	0.3	358	237	100	0.3	0.2	-0.1	-0.5	-3.0	
Township of Oro	170	30.0	15.0	450	0.1	650	450	0.1	352	233	20	0.1	0.0	-0.1	-0.3	-0.1	
James Dick Construction	340	8.7	5.9	51	0.1	650	600	1.1	381	327	675	1.1	0.9	0.1	-1.5	-4.6	
The Sarjeant Co.	265	2.6	13.5	35	0.1	650	600	1.2	353	311	575	1.2	0.7	-0.3	-3.4	-5.5	
Allan G. Cook	232	5.9	5.2	31	0.1	650	600	1.2	365	330	650	1.2	1.0	0.6	-0.6	-1.4	

ORO SEVENTH LINE AGGREGATE PITS
TABLE C-7
POTENTIAL WELL INTERFERENCE

DHL TBLC7

Q m ³ /day	Pumping rate			328 and 657 m ³ /dy assumed pumping rates based on testing at Sarjeant						1964 m ³ /dy tested rate at Seeley & Arnill																	
T m ² /day	Aquifer transmissivity			Range of 50 to 100 m ² /dy from testing at Sarjeant Co.						500 m ² /dy from testing at Seeley & Arnill																	
S	Aquifer storativity			6E-05 from testing at Sarjeant Co.						4E-04 from testing at Seeley & Arnill																	
s m	Drawdown at distance r			Range of 1 to 6 days for storage refilling						Range of 0.1 to 0.5 days for make up water																	
t days	Time since pumping started																										
r m	Distance from well																										
THEIS																											
s = (Q/(4PI T)) LN ((2.25Tt)/(r^2S))																											
Q	T	S	t	INTERFERENCE (m)				r =																			
				0.0762	100	200	300	400	500	600	700	800	900	1000	1200	1500											
328	50	6E-05	0.1	9.1	1.6	0.8	0.4	0.1	0.0	0.0	0.0	0.0	0.0	0.0													
328	50	6E-05	0.5	9.9	2.4	1.7	1.3	1.0	0.7	0.5	0.4	0.2	0.1	0.0													
328	50	6E-05	1	10.3	2.8	2.0	1.6	1.3	1.1	0.9	0.7	0.6	0.5	0.4													
328	50	6E-05	3	10.8	3.3	2.6	2.2	1.9	1.7	1.5	1.3	1.2	1.0	0.9													
328	50	6E-05	6	11.2	3.7	3.0	2.6	2.3	2.0	1.8	1.7	1.5	1.4	1.3													
657	50	6E-05	0.1	18.2	3.1	1.7	0.8	0.2	0.0	0.0	0.0	0.0	0.0	0.0													
657	50	6E-05	0.5	19.8	4.8	3.4	2.5	1.9	1.5	1.1	0.8	0.5	0.2	0.0													
657	50	6E-05	1	20.6	5.5	4.1	3.2	2.6	2.2	1.8	1.5	1.2	0.9	0.7													
657	50	6E-05	3	21.7	6.7	5.2	4.4	3.8	3.3	2.9	2.6	2.3	2.1	1.9													
328	100	6E-05	0.1	4.7	1.0	0.6	0.4	0.2	0.1	0.0	0.0	0.0	0.0	0.0													
328	100	6E-05	0.5	5.1	1.4	1.0	0.8	0.7	0.5	0.4	0.4	0.3	0.2	0.1													
328	100	6E-05	1	5.3	1.6	1.2	1.0	0.8	0.7	0.6	0.5	0.5	0.4	0.3													
328	100	6E-05	3	5.6	1.9	1.5	1.3	1.1	1.0	0.9	0.8	0.8	0.7	0.6													
328	100	6E-05	6	5.8	2.0	1.7	1.5	1.3	1.2	1.1	1.0	0.9	0.8	0.7													
657	100	6E-05	0.1	9.4	1.9	1.2	0.8	0.5	0.2	0.1	0.0	0.0	0.0	0.0													
657	100	6E-05	0.5	10.3	2.8	2.0	1.6	1.3	1.1	0.9	0.7	0.6	0.5	0.4													
657	100	6E-05	1	10.6	3.1	2.4	2.0	1.7	1.5	1.3	1.1	1.0	0.8	0.7													
657	100	6E-05	3	11.2	3.7	3.0	2.6	2.3	2.0	1.8	1.7	1.5	1.4	1.3													
1313	200	6E-05	0.1	9.8	2.3	1.6	1.1	0.8	0.6	0.4	0.3	0.1	0.0	0.0													
1313	200	6E-05	0.5	10.6	3.1	2.4	2.0	1.7	1.5	1.3	1.1	1.0	0.8	0.7													
1313	200	6E-05	1	11.0	3.5	2.8	2.3	2.0	1.8	1.6	1.5	1.3	1.2	1.1													
1313	200	6E-05	1.5	11.2	3.7	3.0	2.6	2.3	2.0	1.8	1.7	1.5	1.4	1.3													
1964	500	4E-04	0.1	5.5	1.0	0.6	0.3	0.1	0.0	0.0	0.0	0.0	0.0	0.0													
1964	500	4E-04	0.5	6.0	1.5	1.1	0.8	0.6	0.5	0.4	0.3	0.2	0.1	0.0													
1964	500	4E-04	1	6.2	1.7	1.3	1.0	0.9	0.7	0.6	0.5	0.4	0.3	0.2													
1964	500	4E-04	1.5	6.3	1.9	1.4	1.2	1.0	0.9	0.7	0.6	0.6	0.5	0.4													

ORO SEVENTH LINE AGGREGATE PITS
TABLE C-8
POTENTIAL CUMULATIVE IMPACT OF WASHING OPERATION WELLS ON PRIVATE WELLS

r (m)	Distance from private well to existing/assumed washing well								
s (m)	Potential water level drawdown								
PROPERTY	Assumed Well Location	Assumed Rate (m³/dy)	Assumed Transmissivity (m²/dy)	Pumping Period (days)	WELL	20959	7681	Murray	7010
Storage filling at start of season									
Seeley & Arnill Aggregate	1/89	1964	500	1	r=	1500	>1500	>1500	1050
					s=	0	0	0	0.3
Alla Aggregates Limited	11628	657	100	3	r=	400	1500	>1500	600
					s=	2.3	0.9	0.9	1.8
James Dick Construction	11519	657	100	3	r=	400	700	1050	1150
					s=	2.3	1.7	1.4	1.1
The Sarjeant Co.	TW8	328	100	6	r=	700	1200	1500	700
					s=	1.0	0.7	0.6	1
Allan G. Cook	OW1/91	657	100	3	r=	1200	500	600	>1500
					s=	1.1	2.0	1.8	0.9
			Total		s=	6.7	5.3	4.7	5.1
Makeup water requirements									
Seeley & Arnill Aggregate	1/89	1964	500	0.1	r=	1500	>1500	>1500	1050
					s=	0	0	0	0
Alla Aggregates Limited	11628	657	100	0.1	r=	400	1500	>1500	600
					s=	0.5	0	0	0.1
James Dick Construction	11519	657	100	0.1	r=	400	700	1050	1150
					s=	0.5	0	0	0
The Sarjeant Co.	TW8	328	100	0.5	r=	700	1200	1500	700
					s=	0.4	0.1	0	0.1
Allan G. Cook	OW1/91	657	100	0.1	r=	1200	500	600	>1500
			Total		s=	0	0.2	0.1	0
					s=	1.4	0.3	0.1	0.2

Calculations are in Table C-7.

ORO SEVENTH LINE AGGREGATE PITS
TABLE C-9
ESTIMATE OF MAXIMUM ANNUAL CALCIUM CHLORIDE APPLICATION

DHL TBLC9

Groundwater recharge	Qgw	= $(Ap \cdot Ip \cdot 365)$					
Reasonable Use concentration	CRU	= $Cgw + (0.5 \cdot (CDW - Cgw))$					
Permitted CaCl ₂ application equivalent to Cin	CaCl _{2A}	$\frac{m^3}{l} \cdot \frac{l}{m^3} \cdot \frac{kg}{l} \cdot \frac{kg}{1000,000}$ = $(Cin \cdot 1.56 \cdot Qgw \cdot 1000) / 1000,000$					
Ap (ha) = Property recharge area Ip (m ³ /day/Ha) = Annual precipitation <i>infiltration</i> Qgw (m ³) = Annual groundwater recharge Cgw (mg/L) = Background chloride in groundwater CDW (mg/L) = Maximum desirable chloride concentration in drinking water CRU (mg/L) = Reasonable Use Policy allowable concentration Cin (mg/L) = Permitted chloride increase from Cgw to CRU CaCl ₂ (kg) = Permitted annual calcium chloride application equivalent to Cin							
Ap	Ip	Qgw	Cgw	CDW	CRU	Cin	CaCl ₂
1	11.4	4161	1.0	250	126	125	808
 Note:							
1 Groundwater recharge is calculated for an infiltration rate equivalent to the water budget surplus of 417 mm/yr (Table C-1)							
2 Background Cl concentration from Table B-4							

Ca 40

Cl 35.5

$$\frac{2 \times Cl + Ca}{2 \times Cl} = \frac{2 \times 35.5 + 40}{2 \times 35.5}$$

ORO SEVENTH LINE AGGREGATE PITS
TABLE C-2
WATER BUDGET VALUES HARGREAVES MODEL
POTENTIAL EVAPOTRANSPIRATION

$$PE = kd(.38 - .0038h)(t-32)$$

PE Potential Evapotranspiration (in)

k vegetation coefficient

d monthly daytime coefficient

h mean monthly relative humidity (Muskoka Climate Station)

t mean monthly temperature (F)

DHL TBLC2

Deciduous Orchard						Pasture					
15099 MIDHURST ONT.											
MONTH	k	d	h	t	PE	MONTH	k	d	h	t	PE
1						1					
2						2					
3	0.14	0.99	65	25		3	0.11	0.99	65	25	
4	0.45	1.09	55	40	0.65	4	0.25	1.09	55	40	0.36
5	0.49	1.24	52	52	2.22	5	0.29	1.24	52	52	1.31
6	0.74	1.26	58	61	4.34	6	0.33	1.26	58	61	1.93
7	0.71	1.27	58	67	5.06	7	0.31	1.27	58	67	2.21
8	0.55	1.17	60	66	3.35	8	0.32	1.17	60	66	1.95
9	0.43	1.01	63	59	1.65	9	0.32	1.01	63	59	1.23
10	0.36	0.91	64	48	0.71	10	0.22	0.91	64	48	0.43
11						11	0.14	0.77	74	38	0.06
12						12					

ORO SEVENTH LINE AGGREGATE PITS
TABLE C-3
SUMMARY OF CHANGES TO THE LOCAL WATER BUDGET

Increased Recharge Clearing the Excavation		Maximum Extraction Area (Table B-1)	Water Budget Gains Ha	Evaporation Wash Pond m3/dy	Stockpiles m3/dy	Exported Water (Table C-4) m3/dy	Water Budget Losses m3/dy	Net Change m3/dy
m3/dy/Ha (mm/annum)			equivalent avg. Ha	m3/dy				
Avg.			Low	High				
Low	High							
1.4 (50)	4.1 (150)	6.8 (250)	220	308	902	1496	64	32 Max. Avg. Extraction 12.8 Max. Licensed Extraction
							140	234 +572 +1166 -100
							280	467 376 563 -68 +339 +933

Note: Evaporation loss from wash pond is equivalent to 12.8 m3/dy/Ha for June through Aug. assuming a 1 Ha pond per operation.

Evaporation loss from stockpiles is equivalent to a surface area of 1 Ha per operation and an evaporation rate of 12.8 m3/dy/Ha for half of the time during the period of June through Aug.

Dust & Scrap

DHL TBL C3

ORO 7th LINE AGGREGATE PITS

TABLE C-4
ESTIMATED EXPORTED WATER

DHL TBL C4

OPERATOR	PROPOSED LICENCE TONNAGE tonnes/yr	ESTIMATED TONNAGE OF WASHED PRODUCT tonnes/yr	RETAINED WATER		EXPORTED WATER (OPERATING 190 DAYS)	
			m3/yr 3 %	m3/yr 5 %	m3/day 3 %	m3/day 5 %
SEELEY & ARNILL AGGREGATES LTD.	1750000	25 % 437500	13125	21875	69	115
ALFA AGGREGATES LIMITED	750000	25 % 187500	5625	9375	30	49
JAMES DICK CONSTRUCTION LIMITED	1000000	25 % 250000	7500	12500	39	66
THE SARJEANT CO. LTD.	750000	90 % 675000	20250	33750	107	178
ALLAN G. COOK LIMITED	900000	25 % 225000	6750	11250	36	59
TOTAL			1775000	53250	280	467
PROPOSED AVERAGE TONNAGE (50 % OF LICENCE)						
SEELEY & ARNILL AGGREGATES LTD.	875000	25 % 218750	6563	10938	35	58
ALFA AGGREGATES LIMITED	375000	25 % 93750	2812.5	4687.5	15	25
JAMES DICK CONSTRUCTION LIMITED	500000	25 % 125000	3750	6250	20	33
THE SARJEANT CO. LTD.	375000	90 % 337500	10125	16875	53	89
ALLAN G. COOK LIMITED	450000	25 % 112500	3375	5625	18	30
TOTAL			887500	26525	44375	140

ORO SEVENTH LINE AGGREGATE PITS
TABLE C-5a
GROUNDWATER MOUNDING THEIS EQUATION
MINIMUM RECHARGE INCREASE (1.4 M3/DY/HA)

DHL TBLC5A

Q m3/day	Increased recharge							
k m/day	Aquifer hydraulic conductivity							
T m2/day	Aquifer transmissivity							
S	Aquifer storativity							
s m	Mounding at distance r							
t days	Time since mounding started							
r m	Distance from centre of property							
THEIS								
s = $(Q/(4\pi T t)) \ln((2.25 T t)/(r^2 S))$								
Q	T	S	t	MOUNDING (m)				
				<i>r</i> =	200	300	400	500
SEELEY & ARNILL AGGREGATES LTD. 46 Ha								
64	100	0.1	365		0.2	0.1	0.1	0.1
64	200	0.1	365		0.1	0.1	0.1	0.0
64	500	0.1	365		0.0	0.0	0.0	0.0
TOWNSHIP OF ORO 25 Ha								
35	100	0.1	365		0.1	0.1	0.0	0.0
35	200	0.1	365		0.1	0.0	0.0	0.0
37	400	0.1	365		0.0	0.0	0.0	0.0
ALFA AGGREGATES LIMITED 26.65 Ha								
37	50	0.1	365		0.1	0.1	0.1	0.0
37	100	0.1	365		0.1	0.1	0.0	0.0
37	200	0.1	365		0.1	0.0	0.0	0.0
JAMES DICK CONSTRUCTION LIMITED 50 Ha								
70	30	0.1	365		0.3	0.2	0.1	0.0
70	50	0.1	365		0.3	0.2	0.1	0.1
70	100	0.1	365		0.2	0.1	0.1	0.1
THE SARJEANT CO. LTD. 39 Ha								
55	30	0.1	365		0.3	0.1	0.1	0.0
55	50	0.1	365		0.2	0.1	0.1	0.0
55	100	0.1	365		0.1	0.1	0.1	0.1
ALLAN G. COOK LIMITED 34.16 Ha								
48	30	0.1	365		0.2	0.1	0.1	0.0
48	50	0.1	365		0.2	0.1	0.1	0.0
48	100	0.1	365		0.1	0.1	0.1	0.0

ORO SEVENTH LINE AGGREGATE PITS
TABLE C-5b
GROUNDWATER MOUNDING THEIS EQUATION
AVERAGE RECHARGE INCREASE (4.1 M³/DY/HA)

DHL TBLC5B

Q m ³ /day	Increased recharge							
k m/day	Aquifer hydraulic conductivity							
T m ² /day	Aquifer transmissivity							
S	Aquifer storativity							
s m	Mounding at distance r							
t days	Time since mounding started							
r m	Distance from centre of property							
THEIS								
$s = (Q/(4\pi T t)) \ln((2.25 T t)/(r^2 S))$								
Q	T	S	t	MOUNDING (m)				
				r =	200	300	400	500
SEELEY & ARNILL AGGREGATES LTD. 46 Ha								
189	100	0.1	365		0.5	0.3	0.2	0.2
189	200	0.1	365		0.3	0.2	0.2	0.1
189	500	0.1	365		0.1	0.1	0.1	0.1
TOWNSHIP OF ORO 25 Ha								
102	100	0.1	365		0.2	0.2	0.1	0.1
102	200	0.1	365		0.2	0.1	0.1	0.1
102	400	0.1	365		0.1	0.1	0.1	0.1
ALFA AGGREGATES LIMITED 26.65 Ha								
109	50	0.1	365		0.4	0.3	0.2	0.1
109	100	0.1	365		0.3	0.2	0.1	0.1
109	200	0.1	365		0.2	0.1	0.1	0.1
JAMES DICK CONSTRUCTION LIMITED 50 Ha								
205	30	0.1	365		1.0	0.5	0.2	-0.0
205	50	0.1	365		0.8	0.5	0.3	0.2
205	100	0.1	365		0.5	0.4	0.3	0.2
THE SARJEANT CO. LTD. 39 Ha								
160	30	0.1	365		0.8	0.4	0.2	-0.0
160	50	0.1	365		0.6	0.4	0.2	0.1
160	100	0.1	365		0.4	0.3	0.2	0.2
ALLAN G. COOK LIMITED 34.16 Ha								
140	30	0.1	365		0.7	0.4	0.2	-0.0
140	50	0.1	365		0.5	0.3	0.2	0.1
140	100	0.1	365		0.3	0.2	0.2	0.1

DIXON HYDROGEOLOGY LIMITED
 TABLE C-5c
 GROUNDWATER MOUNDING THEIS EQUATION
 MAXIMUM RECHARGE INCREASE (6.8 M3/DY/HA)

DHL TBLC5C

Q m ³ /day	Increased recharge							
k m/day	Aquifer hydraulic conductivity							
T m ² /day	Aquifer transmissivity							
S	Aquifer storativity							
s m	Mounding at distance r							
t days	Time since mounding started							
r m	Distance from centre of property							
THEIS	$s = (Q/(4\pi T t)) \ln((2.25 T t)/(r^2 S))$							
Q	T	S	t	MOUNDING (m)				
				r =	200	300	400	500
SEELEY & ARNILL AGGREGATES LTD. 46 Ha								
313	100	0.1	365		0.8	0.6	0.4	0.3
313	200	0.1	365		0.5	0.4	0.3	0.2
313	500	0.1	365		0.2	0.2	0.2	0.1
TOWNSHIP OF ORO 25 Ha								
170	100	0.1	365		0.4	0.3	0.2	0.2
170	200	0.1	365		0.3	0.2	0.2	0.1
170	400	0.1	365		0.1	0.1	0.1	0.1
ALFA AGGREGATES LIMITED 26.65 Ha								
181	50	0.1	365		0.7	0.4	0.3	0.1
181	100	0.1	365		0.4	0.3	0.2	0.2
181	200	0.1	365		0.3	0.2	0.2	0.1
JAMES DICK CONSTRUCTION LIMITED 50 Ha								
340	30	0.1	365		1.6	0.9	0.4	-0.0
340	50	0.1	365		1.3	0.8	0.5	0.3
340	100	0.1	365		0.8	0.6	0.4	0.3
THE SARJEANT CO. LTD. 39 Ha								
265	30	0.1	365		1.3	0.7	0.3	-0.0
265	50	0.1	365		1.0	0.6	0.4	0.2
265	100	0.1	365		0.6	0.5	0.3	0.3
ALLAN G. COOK LIMITED 34.16 Ha								
232	30	0.1	365		1.1	0.6	0.3	-0.0
232	50	0.1	365		0.9	0.6	0.3	0.2
232	100	0.1	365		0.6	0.4	0.3	0.2

ORO SEVENTH LINE AGGREGATE PITS
TABLE C-6
GROUNDWATER MOUNDING DARCY FORMULA

Hydrogeological Variables							Mounding Variables										
Q (m ³ /day)	Rate of Increased Recharge	Hm (m)	Maximum water table rise at equilibrium														
k (m/day)	Hydraulic Conductivity	Xm (m)	Maximum mound radius on X axis														
b (m)	Aquifer Thickness	Ym (m)	Maximum mound radius on Y axis														
T (m ² /day)	Aquifer Transmissivity	Tm (days)	Time to reach equilibrium														
S	Storage (Specific Yield)	rx (m)	Distance From Center of Pit in X Direction														
Excavation Variables																	
L (m)	Length of Excavation	ry (m)	Distance From Center of Pit in Y Direction														
W (m)	Width of Excavation	hm (m)	Height of Mound (Water Table Rise)														
Mounding Calculations							Mounding hm										
Maximum Mound			1. Hm =	$\frac{Q/(2 \cdot P \cdot I \cdot k \cdot b)}{Hm - Q/(P \cdot I \cdot k \cdot b) \cdot \ln(rx/(L^2))}$			325			Mounding hm			225				
Mound at Distance			2. hm =	$\frac{Hm - Q/(P \cdot I \cdot k \cdot b) \cdot \ln(rx/(L^2))}{Q/(2 \cdot P \cdot I \cdot k \cdot b) \cdot (1 - \exp((-(2 \cdot P \cdot I \cdot k \cdot b) \cdot (L^2 \cdot W \cdot S)) / (L^2 \cdot W \cdot S)))}$			330			Mounding hm			300				
Mounding at Time			3. hm =				340			Mounding hm			310				
MINIMUM RECHARGE INCREASE (1.4 m³/dy/Ha)							Mounding hm										
Seeley & Arnill Aggregates	64	32.3	15.5	501	0.1	700	600	0.0	361	310	0	0.1	0.0	-0.0	-0.0	-0.1	
Alta Aggregates	37	10.0	10.0	100	0.1	650	450	0.1	358	237	100	0.1	0.0	-0.1	-0.2	-0.4	
Township of Oro	35	30.0	15.0	450	0.1	650	450	0.0	352	233	0	0.0	-0.0	-0.1	-0.1	-0.2	
James Dick Construction	70	8.7	5.9	51	0.1	650	600	0.2	381	327	150	0.2	0.1	-0.3	1.0	0.2	
The Sarjeant Co.	55	2.6	13.5	35	0.1	650	600	0.2	353	311	175	0.2	-0.1	-0.2	2.2	0.0	
Allan G. Cook	48	5.9	5.2	31	0.1	650	600	0.2	365	330	203	0.2	0.1	-0.3	1.0	0.2	
AVERAGE RECHARGE INCREASE (4.1 m³/dy/Ha)							Mounding hm										
Seeley & Arnill Aggregates	189	32.3	15.5	501	0.1	700	600	0.1	361	310	25	0.2	0.2	-0.1	-0.1	-0.2	
Alta Aggregates	109	10.0	10.0	100	0.1	650	450	0.2	358	237	100	0.2	0.1	-0.3	0.2	-0.8	
Township of Oro	102	30.0	15.0	450	0.1	650	450	0.0	352	233	0	0.0	-0.0	-0.1	0.0	-0.3	
James Dick Construction	205	8.7	5.9	51	0.1	650	600	0.6	381	327	250	0.6	0.5	-0.4	2.8	0.6	
The Sarjeant Co.	160	2.6	13.5	35	0.1	650	600	0.7	353	311	425	0.7	0.4	-0.2	-3.3	6.4	
Allan G. Cook	140	5.9	5.2	31	0.1	650	600	0.7	365	330	475	0.7	0.6	-0.4	-0.8	2.9	
MAXIMUM RECHARGE INCREASE (6.8 m³/dy/Ha)							Mounding hm										
Seeley & Arnill Aggregates	313	32.3	15.5	501	0.1	700	600	0.1	361	310	10	0.3	0.3	-0.1	-0.1	-0.4	
Alta Aggregates	181	10.0	10.0	100	0.1	650	450	0.3	358	237	100	0.3	0.2	-0.1	-0.2	-2.3	
Township of Oro	170	30.0	15.0	450	0.1	650	450	0.1	352	233	20	0.1	0.0	-0.1	-0.5	-0.6	
James Dick Construction	340	8.7	5.9	51	0.1	650	600	1.1	381	327	675	1.1	0.9	0.1	-1.5	-4.6	
The Sarjeant Co.	265	2.6	13.5	35	0.1	650	600	1.2	353	311	575	1.2	0.7	-0.3	-3.4	-5.5	
Allan G. Cook	232	5.9	5.2	31	0.1	650	600	1.2	365	330	650	1.2	1.0	0.6	-0.6	-1.4	

ORO SEVENTH LINE AGGREGATE PITS
TABLE C-7
POTENTIAL WELL INTERFERENCE

DHL TBLC7

Q m ³ /day	Pumping rate			328 and 657 m ³ /dy assumed pumping rates based on testing at Sarjeant						1964 m ³ /dy tested rate at Seeley & Arnill																	
T m ² /day	Aquifer transmissivity			Range of 50 to 100 m ² /dy from testing at Sarjeant Co.						500 m ² /dy from testing at Seeley & Arnill																	
S	Aquifer storativity			6E-05 from testing at Sarjeant Co.						4E-04 from testing at Seeley & Arnill																	
s m	Drawdown at distance r			Range of 1 to 6 days for storage refilling						Range of 0.1 to 0.5 days for make up water																	
t days	Time since pumping started																										
r m	Distance from well																										
THEIS																											
s = (Q/(4PIT) LN ((2.25Tt)/(r²S)))																											
Q	T	S	t	INTERFERENCE (m)				r =																			
				0.0762	100	200	300	400	500	600	700	800	900	1000	1200	1500											
328	50	6E-05	0.1	9.1	1.6	0.8	0.4	0.1	0.0	0.0	0.0	0.0	0.0	0.0													
328	50	6E-05	0.5	9.9	2.4	1.7	1.3	1.0	0.7	0.5	0.4	0.2	0.1	0.0													
328	50	6E-05	1	10.3	2.8	2.0	1.6	1.3	1.1	0.9	0.7	0.6	0.5	0.4													
328	50	6E-05	3	10.8	3.3	2.6	2.2	1.9	1.7	1.5	1.3	1.2	1.0	0.9													
328	50	6E-05	6	11.2	3.7	3.0	2.6	2.3	2.0	1.8	1.7	1.5	1.4	1.3													
657	50	6E-05	0.1	18.2	3.1	1.7	0.8	0.2	0.0	0.0	0.0	0.0	0.0	0.0													
657	50	6E-05	0.5	19.8	4.8	3.4	2.5	1.9	1.5	1.1	0.8	0.5	0.2	0.0													
657	50	6E-05	1	20.6	5.5	4.1	3.2	2.6	2.2	1.8	1.5	1.2	0.9	0.7													
657	50	6E-05	3	21.7	6.7	5.2	4.4	3.8	3.3	2.9	2.6	2.3	2.1	1.9													
328	100	6E-05	0.1	4.7	1.0	0.6	0.4	0.2	0.1	0.0	0.0	0.0	0.0	0.0													
328	100	6E-05	0.5	5.1	1.4	1.0	0.8	0.7	0.5	0.4	0.4	0.3	0.2	0.1													
328	100	6E-05	1	5.3	1.6	1.2	1.0	0.8	0.7	0.6	0.5	0.5	0.4	0.3													
328	100	6E-05	3	5.6	1.9	1.5	1.3	1.1	1.0	0.9	0.8	0.8	0.7	0.6													
328	100	6E-05	6	5.8	2.0	1.7	1.5	1.3	1.2	1.1	1.0	0.9	0.8	0.7													
657	100	6E-05	0.1	9.4	1.9	1.2	0.8	0.5	0.2	0.1	0.0	0.0	0.0	0.0													
657	100	6E-05	0.5	10.3	2.8	2.0	1.6	1.3	1.1	0.9	0.7	0.6	0.5	0.4													
657	100	6E-05	1	10.6	3.1	2.4	2.0	1.7	1.5	1.3	1.1	1.0	0.8	0.7													
657	100	6E-05	3	11.2	3.7	3.0	2.6	2.3	2.0	1.8	1.7	1.5	1.4	1.3													
1313	200	6E-05	0.1	9.8	2.3	1.6	1.1	0.8	0.6	0.4	0.3	0.1	0.0	0.0													
1313	200	6E-05	0.5	10.6	3.1	2.4	2.0	1.7	1.5	1.3	1.1	1.0	0.8	0.7													
1313	200	6E-05	1	11.0	3.5	2.8	2.3	2.0	1.8	1.6	1.5	1.3	1.2	1.1													
1313	200	6E-05	1.5	11.2	3.7	3.0	2.6	2.3	2.0	1.8	1.7	1.5	1.4	1.3													
1964	500	4E-04	0.1	5.5	1.0	0.6	0.3	0.1	0.0	0.0	0.0	0.0	0.0	0.0													
1964	500	4E-04	0.5	6.0	1.5	1.1	0.8	0.6	0.5	0.4	0.3	0.2	0.1	0.0													
1964	500	4E-04	1	6.2	1.7	1.3	1.0	0.9	0.7	0.6	0.5	0.4	0.3	0.2													
1964	500	4E-04	1.5	6.3	1.9	1.4	1.2	1.0	0.9	0.7	0.6	0.6	0.5	0.4													

ORO SEVENTH LINE AGGREGATE PITS
TABLE C-8
POTENTIAL CUMULATIVE IMPACT OF WASHING OPERATION WELLS ON PRIVATE WELLS

r (m)	Distance from private well to existing/assumed washing well								
s (m)	Potential water level drawdown								
PROPERTY	Assumed Well Location	Assumed Rate (m³/dy)	Assumed Transmissivity (m²/dy)	Pumping Period (days)	WELL	20959	7681	Murray	7010
Storage filling at start of season									
Seeley & Arnill Aggregate	1/89	1964	500	1	r=	1500	>1500	>1500	1050
					s=	0	0	0	0.3
Alla Aggregates Limited	11628	657	100	3	r=	400	1500	>1500	600
					s=	2.3	0.9	0.9	1.8
James Dick Construction	11519	657	100	3	r=	400	700	1050	1150
					s=	2.3	1.7	1.4	1.1
The Sarjeant Co.	TW8	328	100	6	r=	700	1200	1500	700
					s=	1.0	0.7	0.6	1
Allan G. Cook	OW1/91	657	100	3	r=	1200	500	600	>1500
					s=	1.1	2.0	1.8	0.9
			Total		s=	6.7	5.3	4.7	5.1
Makeup water requirements									
Seeley & Arnill Aggregate	1/89	1964	500	0.1	r=	1500	>1500	>1500	1050
					s=	0	0	0	0
Alla Aggregates Limited	11628	657	100	0.1	r=	400	1500	>1500	600
					s=	0.5	0	0	0.1
James Dick Construction	11519	657	100	0.1	r=	400	700	1050	1150
					s=	0.5	0	0	0
The Sarjeant Co.	TW8	328	100	0.5	r=	700	1200	1500	700
					s=	0.4	0.1	0	0.1
Allan G. Cook	OW1/91	657	100	0.1	r=	1200	500	600	>1500
			Total		s=	0	0.2	0.1	0
					s=	1.4	0.3	0.1	0.2

Calculations are in Table C-7.

ORO SEVENTH LINE AGGREGATE PITS
TABLE C-9
ESTIMATE OF MAXIMUM ANNUAL CALCIUM CHLORIDE APPLICATION

DHL TBLC9

Groundwater recharge	Qgw	= $(Ap \cdot Ip \cdot 365)$					
Reasonable Use concentration	CRU	= $Cgw + (0.5 \cdot (CDW - Cgw))$					
Permitted CaCl ₂ application equivalent to Cin	CaCl _{2A}	$\frac{m^3}{l} \cdot \frac{l}{m^3} \cdot \frac{kg}{l} \cdot \frac{kg}{1000,000}$ = $(Cin \cdot 1.56 \cdot Qgw \cdot 1000) / 1000,000$					
Ap (ha) = Property recharge area Ip (m ³ /day/Ha) = Annual precipitation <i>infiltration</i> Qgw (m ³) = Annual groundwater recharge Cgw (mg/L) = Background chloride in groundwater CDW (mg/L) = Maximum desirable chloride concentration in drinking water CRU (mg/L) = Reasonable Use Policy allowable concentration Cin (mg/L) = Permitted chloride increase from Cgw to CRU CaCl ₂ (kg) = Permitted annual calcium chloride application equivalent to Cin							
Ap	Ip	Qgw	Cgw	CDW	CRU	Cin	CaCl ₂
1	11.4	4161	1.0	250	126	125	808
 Note:							
1 Groundwater recharge is calculated for an infiltration rate equivalent to the water budget surplus of 417 mm/yr (Table C-1)							
2 Background Cl concentration from Table B-4							

Ca 40

Cl 35.5

$$\frac{2 \times Cl + Ca}{2 \times Cl} = \frac{2 \times 35.5 + 40}{2 \times 35.5}$$